

FOREPOLING PRE-SUPPORT APPLICATION IN THE SHALLOW
TUNNELING THROUGH WEATHERED GRANITE

SARDER MOHAMMAD YAHYA

A thesis submitted in fulfilment of the
requirements for the award of the degree of
Master of Engineering (Geotechnics)

Faculty of Civil Engineering
Universiti Teknologi Malaysia

JULY 2016

Dedicated to my beloved family.
And those craving for unveiling this mysterious universe.

ACKNOWLEDGEMENT

Firstly, I convey my sincere thanks to the Almighty Creator, our Lord Allah (SWT) for making everything possible as there is no power nor movement without His permission. He was my last resort to turn when I felt empty inside and every hope shattered around me. He was my sole guardian in this lonely expat life who continuously took care of me. He always pulled me through every hardship and challenge I encountered. Remembrance of Him kept me leaping forward when I was about to get lost in the abyss. Peace be upon Prophet Muhammad (SAW), the last messenger whose beautiful teachings of peace and prosperity shaped my psychology and life's philosophy.

I am greatly indebted to my beloved family members back home for their unconditional love, support and prayer. The sacrifices of my loving and caring parents are undeniable as those are the source of all my accomplishments. May Allah grant them with the best of rewards here and hereafter.

I would like to express my heartfelt gratitude for my supervisor Dr. Rini Asnida Binti Abdullah for her constant support and guidance throughout my study with patience and enthusiasm. Her suggestions and comments up until the end of writing thesis was invaluable. I am greatly thankful to my co-supervisor Dr. Mohd Ashraf Mohamad Ismail for his tremendous support in my research. His guidelines shaped my entire research design, enhanced my understanding of the software and his help regarding the necessary data collection was greatly appreciable. I am also very grateful to my co-supervisor Assoc. Prof. Dr. Hisham Bin Mohamad for his priceless suggestions regarding model geometry setup, setting up boundary condition, model verification approach and finite element analysis theory. May Allah bless them and their family.

I want to specially mention my favorite course teacher Assoc. Prof. Dr. Mohd For Mohd Amin for his generosity, friendly & positive behavior. His delivery of awesome lectures in classes developed my further passion in rock mechanics and tunnel engineering.

I want to specially thank my brother and research mentor Md. Habibur Rahman (Titas) for his suggestions, guidance and constant motivation to keep me going when I wanted to give up. His contributions in my post graduate student life cannot be described in words. I really appreciate the kindness of his nice wife Nusrat Jahan towards me.

I am indebted to friends Md. Tareq Rahman and Md. Sabbir Ahmed for their unimaginable support in every moment. My appreciation goes to Sakif Saad, Sumaiya Rahman, Mhm Mubassir, Faisal Ahmed.

Special thanks to my lovely juniors Raian Zafar Khan, Sharful Hossain Rafee, Saidus Salehin and Shamur Rahman Akash. Their company made my life colorful and very enjoyable here. I will never forget their awesome support despite me not being able to give them any service.

The suggestions of my seniors Mir Hossain Sohel and Mohammad Asif Ul Haq regarding research were precious. I am also thankful to Sohel Rana, Sadia Afrin, Ananya Raka Chakrabarty, Himadry Shekhor Das, Raha Siddiqui, Jubaer Ahmed, Ahmad Al Razi for their encouragement and concerns in my research.

So many people were involved in my journey here, I cannot mention all the names in this limited space and therefore, I sincerely apologize in this regard. I will cherish the sweet memories of UTM rest of my life. Thanks to all my treasured friends and relatives back home who missed me, yet prayed and wished for me.

Johor Bahru, July, 2016
Sarder Mohammad Yahya

ABSTRACT

Tunnel face stability and ground movements induced by excavation are of significant concern in shallow tunneling through weak ground. In conjunction with other support systems, various pre-support measures are applied to reinforce the ground prior to excavation for ensuring tunnel crown stability and construction safety in general. Among them, forepoling is the simplest form of umbrella arch pre-support method which is widely used in tunneling application. Until now, very few studies addressed the effect of forepoling design parameters on crown stability of shallow tunnel constructed in weathered rock and no study is available for weathered granite. This leads to conservative and uneconomical designs of basic forepoling design parameters. This study aims at quantitative understanding through numerical parametric analysis regarding the behavior of forepoling pre-support design parameters that affect the crown stability of a shallow tunnel constructed in weathered granite. The effect of three design parameters - forepole length, installation angle and spacing on crown stability were investigated by using 3D finite element analysis program RS3 by Rocscience; using Pahang-Selangor raw water transfer tunnel project as a reference case. These parameters were then optimized considering total displacement and induced major principal stress at tunnel crown as evaluation indicator of crown stability. The results show that, the total displacement at tunnel crown increases by 0.64% when the forepole length was increased from 5 m to 9 m. The induced major principal stress at tunnel crown decreases by 1.02% when the length is varied from 3 m to 8 m. The total displacement at tunnel crown decreases by 0.23% when the installation angle of forepole is varied from 5° to 9°. From this study, it was found that length of 3 and 4 m; with angle of 5° to 7°, and spacing of 300 mm are optimum forepoling design parameters under given tunneling condition.

ABSTRAK

Kestabilan muka terowong dan pergerakan tanah yang disebabkan oleh penggalian harus diambil berat terutamanya apabila melibatkan terowong cetek di dalam tanah yang lemah. Dengan kaedah sistem sokongan, pelbagai langkah pra-sokongan digunakan untuk mengukuhkan tanah sebelum penggalian bagi memastikan kestabilan bumbung terowong dan keselamatan pembinaan secara amnya. Antaranya, kaedah *forepoling*, di mana merupakan kaedah pra-sokongan yang paling mudah di dalam sistem kaedah pra-sokongan payung gerbang dan digunakan secara meluas di dalam aplikasi terowong. Sehingga kini, sangat sedikit kajian mengambilkira kesan parameter reka bentuk *forepoling* terhadap kestabilan bumbung terowong cetek yang dibina di dalam batu terluluhawa, dan tiada kajian dilakukan untuk granit terluluhawa. Ini seterusnya, membawa kepada reka bentuk konservatif dan tidak ekonomi dalam reka bentuk asas *forepoling*. Kajian ini bertujuan untuk memahami dari sudut kuantitatif melalui kaedah analisis parametrik berangka mengenai tingkah laku parameter reka bentuk *forepoling* yang mempengaruhi kestabilan bumbung terowong cetek di dalam granit terluluhawa. Kesan daripada tiga parameter reka bentuk, iaitu panjang, sudut pemasangan dan jarak, kestabilan bumbung terowong telah dikaji dengan menggunakan program RS3, iaitu kaedah unsur terhingga 3D oleh Rocscience; dengan mengambil kira projek Terowong Pemindahan Air Mentah Pahang-Selangor yang telah siap sebagai kes rujukan. Parameter-parameter ini kemudian dioptimumkan dengan mempertimbangkan jumlah anjakan dan tegasan prinsipal utama di bumbung terowong sebagai petunjuk penilaian kestabilan bumbung. Keputusan menunjukkan bahawa, jumlah anjakan pada bumbung terowong meningkat sebanyak 0.64% apabila panjang *forepole* telah ditingkatkan daripada 5m hingga 9m. Tegasan utama pada bumbung terowong berkurangan sebanyak 1.02% apabila panjang ditambah dari 3m kepada 8m. Manakala, jumlah anjakan pada bumbung terowong berkurangan sebanyak 0.23% apabila sudut pemasangan daripada *forepole* diubah dari 5° kepada 9°. Daripada kajian ini, didapati bahawa panjang 3m dan 4 m; dengan sudut sudut 5° kepada 7°, dan jarak 300mm adalah reka bentuk *forepoling* yang paling optimum merujuk kepada keadaan terowong tersebut.

TABLE OF CONTENTS

CHAPTER	TITLE	PAGE
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDGEMENT	iv
	ABSTRACT	vi
	ABSTRAK	vii
	TABLE OF CONTENTS	viii
	LIST OF TABLES	xii
	LIST OF FIGURES	xiv
	LIST OF ABBREVIATIONS	xvi
	LIST OF SYMBOLS	xvii
	LIST OF APPENDICES	xviii
1	INTRODUCTION	1
	1.1 Background of the Study	1
	1.2 Problem Statement	3
	1.3 Significance of the Study	3
	1.4 Research Aim and Objectives	4
	1.5 Scope and Limitations of the Study	4
	1.6 Organization of the Thesis	5
2	LITERATURE REVIEW	7
	2.1 Introduction	7

2.2	In Situ and Induced Stresses around Underground Excavation in Rock	7
2.3	New Austrian Tunneling Method (NATM)	9
2.4	Tunnel Support Systems	10
2.4.1	Initial Tunnel Support Systems	11
2.4.2	Pre-Support Systems	12
2.4.3	Umbrella Arch Pre-support Mechanism and Design Parameters	13
2.4.4	Umbrella Arch Pre-support in Contrast to Rock Reinforcements	17
2.4.5	Previous Studies Related to Umbrella Arch Design Parameters	18
2.5	Selection of Practical Forepoling Design Parameters	19
2.6	Rock Mass Classification	20
2.6.1	Rock Quality Designation (RQD)	21
2.6.2	Geological Strength Index (GSI)	22
2.6.3	Japan Highway (JH) System	23
2.7	Rock Weathering Classification	24
2.8	Characteristics of Weathered Granite Materials	25
2.9	Subsurface Modelling	28
2.9.1	Programs Used for Subsurface Modelling	28
2.10	Numerical Analysis Methods	29
2.10.1	Benefits of Numerical Methods	30
2.10.2	3D Numerical Simulation in Tunnelling	31
2.10.3	Modelling Methods in Rock Mechanics	31
2.10.4	A Brief Overview of Finite Element Method (FEM)	34
2.10.5	Solution Scheme and Convergence Criteria	34

2.11	Failure Criteria Used in Numerical Model	36
2.11.1	Mohr-Coulomb Failure Criteria	36
2.11.2	Generalized Hoek-Brown Failure Criteria	37
2.12	Program Used for Numerical Modelling	39
2.12.1	The RS ³ Program	40
2.12.2	Working Principles of Piles and Forepoles in RS3	41
2.13	Model Verification Approaches in Tunnelling	42
2.14	Summary	43
3	RESEARCH METHODOLOGY	44
3.1	Introduction	44
3.2	Research Procedures	44
3.3	Description of the Case Study	48
3.3.1	Geology of the Area	49
3.4	Subsurface Modelling in RockWorks 15 and EnviroInsite	50
3.5	Modelling Procedures in RS3	55
3.6	Constructing Simplified Conceptual Model	55
3.6.1	Geometry Setup	56
3.6.2	Modelling Parameters	57
3.6.3	Boundary Condition	59
3.6.4	Multi-stage Excavation and Support Installation	60
3.6.5	Mesh and Discretization	62
3.7	Summary	63
4	RESULTS AND DISCUSSION	64
4.1	Introduction	64

4.2	Limitations and Pitfalls in Using Finite Element Software	64
4.3	Considerations of Output Results from Numerical Analysis	65
4.4	Analysis of Case 1 - Without Forepole	66
4.4.1	Verification of the model	67
4.5	Numerical Parametric Analyses of Forepoling Design Parameters	67
4.5.1	Analysis of Forepole Length	68
4.5.2	Analysis of Installation Angle	70
4.5.3	Analysis of Spacing	72
4.6	Significant Findings from the Results of Numerical Analysis	75
4.6.1	Effect of Forepoling Design Parameters on Crown Stability	75
4.6.2	Optimization of Forepoling Design Parameters	76
4.6.3	Comparison of Obtained Results with Previous Relevant Studies	77
4.7	Summary	78
5	CONCLUSIONS AND RECOMMENDATIONS	79
5.1	Conclusions	79
5.2	Recommendations for Future Works	81
	REFERENCES	83
	Appendix A	94
	Appendix B	96
	Appendix C	98
	Appendix D	103
	Appendix E	105

LIST OF TABLES

TABLE NO.	TITLE	PAGE
2.1	Previous studies related to umbrella arch design parameters	18
2.2	Forepoling application parameters for design in previous literatures	19
2.3	Selected values of forepole design parameters for this study	20
2.4	Rock mass quality classification according to RQD (Deere <i>et al.</i> 1967)	21
2.5	JH classification class and rock description	23
2.6	ISRM suggested classification of weathered rock (after Santi, 2006)	24
2.7	Weathering degree characteristics of Granite (after Santi, 2006)	25
2.8	Difference between lithology and stratigraphy	28
2.9	Relative advantages and disadvantages of different numerical methods (after Cai, 2008; Coggan, 2012)	33
2.10	Guidelines for estimating disturbance factor D (from Hoek <i>et al.</i> , 2002)	39
3.1	Location of boreholes and tunnel alignment relative to chaingae	51
3.2	Location, depth, elevation and inclination of boreholes	51
3.3	Lithological sequence of BH34	52
3.4	Lithological sequence of BH35	52
3.5	Lithological sequence of BH15	52
3.6	Lithological sequence of TDH7D	52
3.7	Lithological sequence of TDH7L	52

3.8	Material properties used for soil strata	58
3.9	Material properties used for rock strata	58
4.1	Total displacement values at tunnel crown for different forepole lengths	68
4.2	Induced major principal stress values at tunnel crown for different forepole lengths	68
4.3	Total displacement values at tunnel crown for different installation angles	70
4.4	Induced major principal stress values at tunnel crown for different installation angles	71
4.5	Total displacement values at tunnel crown for different spacing	73
4.6	Induced major principal stress values at tunnel crown for different spacing	73
4.7	Optimized values of different forepoling design parameters	77

LIST OF FIGURES

FIGURE NO.	TITLE	PAGE
2.1	Typical illustration of in-situ and induced major principal stresses around circular tunnel (after Hoek <i>et al.</i> , 2000)	9
2.2	Different tunnel support system categories (Modified after FHA, 2009)	10
2.3	Application of application as initial support system in NATM tunneling	11
2.4	Action mechanism of umbrella arch pre-support	14
2.5	Action mechanism of fiberglass nails pre-support	15
2.6	Cross-sectional profile of pipe roof umbrella pre-support	16
2.7	3D view of pipe roof umbrella parameters (modified after Tuncdemir, 2012)	16
2.8	Layout of different design parameters of forepoles	17
2.9	Changes in a granite block in different weathering grades (after Thuro and Scholz, 2003)	27
2.10	The non-linear response of a spring to applied loads (after Potts and Ganendra, 1994)	35
2.11	The Mohr-Coulomb failure criterion with a tension cutoff (after Goodman, 1989)	37
2.12	Geometry of an embedded beam element (after Sadek and Shahrour, 2004)	42
3.1	Flow of the research activities	45

3.2	Outline of PSRWT tunnel showing studied segment (after Ismail <i>et al.</i> , 2013)	49
3.3	Geological condition surrounding the tunnel (after Nordin, 2010)	50
3.4	Location of boreholes and tunnel alignment from Google™ Earth	51
3.5	Borehole strip plot obtained from RockWorks 15	53
3.6	Lithology model constructed in RockWorks 15	54
3.7	Simplified stratigraphy profile generated in EnviroInsite	54
3.8	Workflow of RS ³ modelling steps	55
3.9	Integrated GSI calculator	56
3.10	Constructed geometry in RS ³	59
3.11	Boundary condition adopted in the model in 3D	60
3.12	Multi-stage excavation sequence of Case 1- without forepole models	61
3.13	Multi-stage excavation sequence of Case 2- with forepole models	62
3.14	Finite element model after meshing in 3D	63
4.1	Illustration showing the distance along excavation advance length	66
4.2	Total displacement at tunnel crown without forepole	66
4.3	Plot of total displacement against forepole length curve	69
4.4	Plot of induced major principal stress against forepole length curve	70
4.5	Plot of total displacement against installation angle curve	71
4.6	Plot of induced major principal stress against installation angle curve	72
4.7	Plot of total displacement against spacing curve	74
4.8	Plot of induced major principal stress against spacing curve	74

LIST OF ABBREVIATIONS

BEM	-	Boundary Element Method
CAD	-	Computer-Aided Design
DEM	-	Distinct Element Method
DFN	-	Discrete Fracture Network
FDM	-	Finite Difference Method
FEA	-	Finite Element Analysis
FEM	-	Finite Element Method
GSI	-	Geological Strength Index
JH	-	Japan Highway
ISRM	-	International Society for Rock Mechanics
MLD	-	Million Liter Per Day
NATM	-	New Austrian Tunneling Method
NGI	-	Norwegian Geotechnical Institute
PSRWT	-	Pahang-Selangor Raw Water Transfer
RMi	-	Rock Mass Index
RMR	-	Rock Mass Rating
RQD	-	Rock Quality Designation
TBM	-	Tunnel Boring Machine
UA	-	Umbrella Arch
UCS	-	Uniaxial Compressive Strength
UDEC	-	Universal Distinct Element Code
UTM	-	Universal Transverse Mercator

LIST OF SYMBOLS

F	-	Internal Forces
P	-	Applied Loads
J_a	-	Joint Alteration Number
J_n	-	Joint Set number
J_r	-	Joint Roughness Number
J_w	-	Joint Water Reduction Factor
m_b	-	Material Constant
ΔU	-	Nodal Displacements
σ_1	-	Major Principal Stress
σ_3	-	Minor Principal Stress
σ_{ci}	-	Intact Rock Uniaxial Compressive Strength
σ_n	-	Normal Stress
τ_f	-	Shear Stress
φ	-	Friction Angle

LIST OF APPENDICES

APPENDIX	TITLE	PAGE
A	NATM section design sheet of different rock class	94
B	Tunnel face mapping at Chainage 5920	96
C	Convergence monitoring data at Chainage 5921	98
D	Borehole log data of BH34	103
E	RS ³ software generated output	105

CHAPTER 1

INTRODUCTION

1.1 Background of the Study

With the advent of urbanization and innovative technologies for infrastructure development, tunneling is becoming a sustainable practice recently. In various cities around the world, the continuously growing development projects are utilizing the underground space due to having limited available surface space resulting from pre-existing structures. These tunnels are commonly situated in populated urban areas and are shallow in nature. Generally, the urban ground condition is not always favorable, and therefore is susceptible to major displacement due to tunneling process especially excavated in the soft soil and weak weathered rock formation (Watika *et al.*, 2004; Volkmann and Schubert, 2007; Latif *et al.*, 2012). Rock masses situated at shallow depths exhibit weak mechanical behavior than similar rock masses at greater depth under confined stress. This is due to the fact that rock masses near the surface are prone to weathering and stress relief which disrupts the interlocking between rock particles, and therefore tend to exhibit more mobility (Hoek, 2004).

For the abovementioned issues, to prevent any undesirable scenario and to ensure excavation safety by reinforcing the ground different pre-supports are employed in compliance with design requirements, such as ground freezing, inserting jet grouted columns, incorporating umbrella arch and so on. Among them the umbrella arch pre-support systems have widely been used to ensure stability during tunnel excavation by preventing tunnel face from collapsing especially in unfavorable subsurface conditions such as poor self-standing and/or weathered ground formation

with a shallow overburden which are vulnerable to excessive displacement (Park *et al.*, 2012; Latif *et al.*, 2012).

The umbrella arch pre-support system installation procedure is less time intensive and more cost efficient compared to the abovementioned pre-support system. For these reasons, the use of umbrella arch pre-support systems have been increased over the last few years. However, there is still lack of comprehensive understanding and knowledge regarding the interaction between ground and pre-support systems related to umbrella arch system (Volkman *et al.*, 2006). The absence of widely accepted rules, formulas and clear guidelines for determining basic design parameters led the tunnel designers rely to on real life experience and common practices which may account for uneconomical and conservative design (Volkman and Schubert, 2006, 2007).

Various previous literature emphasized on the significance of understanding the umbrella arch system behavior. In order to understand its behavior it is necessary to determine the influence of each umbrella arch design parameter on stability of tunnel face. Very few studies addressed the issue of design parameters (Song *et al.*, 2006; Wang, 2012; Oke, 2014). However, no previous studies covered the parametric study of forepoling design parameters in weathered rock.

Generally numerical modeling illustrates a real life physical phenomena in an idealized and simplified conceptual model. This method is proven to be an effective tool for anticipating possible scenarios arising in underground excavation. Also, numerical modeling provides a simulation environment of probable uncertainties that might occur during tunnel excavation. Although results produced by numerical modeling are not entirely accurate, the results are still effective for aiding in understanding the existing underlying mechanisms and thus providing various options for ground improvement (Latif *et al.*, 2012). Contrary to the construction experience obtained during real life tunnel excavation, the same tunnel can be excavated repeatedly by varying different parameters in numerical modeling.

1.2 Problem Statement

Over the past few years, the application of umbrella arch pre-support systems have significantly increased in shallow tunneling in weathered rock. Often, determining the basic design parameters of these pre-support systems are based solely on the experience of tunnel engineers as objective design guidelines are still not available. A standardized approach is required for determining these design parameters during tunnel designing process. Also, any civil engineering design needs optimization to ensure both safety and economic efficiency at the same time. However, there is a significant lack of understanding regarding the geotechnical behavior of umbrella arch pre-support system which may lead to the conservative and inefficient designs in terms of economy. Therefore, comprehensive fundamental knowledge regarding the interaction between weathered rock and umbrella arch pre-support is essential in order to optimize the design parameters. Hence, it is necessary to understand the influences of each design parameter on ground response with a view to providing an insight to the tunnel designers for making sound engineering judgment.

1.3 Significance of the study

The study of umbrella arch pre-support is limited in rock engineering literatures, compared to other conventional support systems, such as shotcrete, rock bolt, and lattice girder. This study attempted to contribute to the knowledge of geotechnical behavior of pre-support in rock addressing the issues of dimensioning forepole design parameters. The result of numerical analysis of forepoling design parameters will provide a useful insight to the tunnel engineers and related policy makers for practical design optimization as well as decision making. The understanding of the influence of these design parameters to ground response will aid the designers to adopt a standard design guidelines avoiding overestimation as well as maintaining safety at the same time. The knowledge of rock and pre-support interaction will benefit policy makers and contactors related to tunnel construction for preparing contract documents and avoiding dispute. The outcome and recommendation of this study will provide future direction for further comprehensive

and detailed study incorporating jointed rock model along with high quality field and laboratory test data.

1.4 Research Aim and Objectives

The research is aimed at quantitative fundamental understanding of the behavior of forepole pre-support design parameters that affect the crown stability of a shallow tunnel constructed in weathered granite by using numerical analysis. Hence, two main objectives to achieve the aim are as follows:

1. To identify the effect of three forepoling design parameters - forepole length, installation angle and spacing on crown stability of shallow tunnel in weathered granite.
2. To determine the optimized forepoling design parameters with a view to providing an insight for safe and economic design under given tunneling condition.

1.5 Scope and Limitations of the Study

The study takes Pahang-Selangor Raw Water Transfer (PSRWT) tunnel project as reference case. A 20 m long tunnel section belong to New Austrian Tunneling Method (NATM)-3 segment of PSRWT tunnel is considered for this study which was located within Chainage 5910 to 5930. The studied section is situated just before the Kerau River crossing area having a shallow overburden of 20 m. The tunnel section encountered weathered granite which had undergone full face excavation by controlled drill and blast technique. Among different categories of umbrella arch pre-support systems, forepoling was considered for this study focusing on mainly three design parameters: forepole length, installation angle, and spacing. Their effects on tunnel crown stability were investigated in terms of two indicators - total displacement and induced major principal stress. RockWorks 15 and EnvironInsite software were used for subsurface modelling. Parameters for subsurface modelling programs were

adopted from the geological data of five boreholes. The numerical parametric studies are carried out using three dimensional (3D) finite element software RS³ incorporating multi-stage excavation.

Required data were collected from various secondary sources such as Malaysian Ministry of Energy, Green Technology and Water (Kementerian Tenaga, Teknologi Hijau dan Air in Malay language, abbreviated as KeTTHA), project contractors, literature review and personal contact with tunnel related professionals. Unavailability of some data as well as unwillingness to share confidential data were significant constraints faced during this research. In these cases, derived data were used where some of the input data of the numerical analysis software were devised from correlations, engineering judgment, and typical values from previous literatures.

1.6 Organization of the Thesis

This thesis has been organized into five chapters. Chapter 1 discusses the background of the study and identifies the problem statement. It also highlights the main aim and objectives of the research as well as its scope and limitations. A brief overview of the forthcoming chapters is also provided here.

Chapter 2 presents comprehensive literature review of general concepts, theories, previous findings and overview of software relevant to this study. This chapter focuses on various NATM support systems including initial support systems and umbrella arch pre-support systems long with its design parameters. Value of three forepoling design parameters from previous projects and studies are summarized. The chapter also provides a general overview of stresses around excavation opening in rock, various rock mass classification systems, rock weathering classification, weathered granite material characteristics, different numerical analysis methods, Mohr-Coulomb and Generalized Hoek-Brown failure criteria.

Chapter 3 provides a flowchart and a brief overview of the research activities. A short description of the PSRWT tunnel project is provided which is used as a

reference case for this study. This chapter also illustrates the technical descriptions, procedures and steps adopted for both subsurface modelling and numerical modelling. Firstly, it puts forward the input requirements of the modelling software extracted from collected data. After that it shows the step-by-step development of numerical model with two cases; Case 1- with forepole and Case 2 - without forepole.

Chapter 4 discusses the limitations and pitfalls in using the finite element software. It also illustrates the model verification by comparing simulation results based on analysis of Case 1 with field instrumentation data. After verification the analyzed results of numerical parametric study of Case 2 models are discussed to investigate the influence of three aforementioned forepoling design parameters on crown stability. Optimized values based on two crown stability indicators are illustrated. The findings of this study were compared with the relevant previous studies conducted by other researchers.

Finally, Chapter 5 presents the conclusive remarks on the study referring to the objectives which including summarized research findings. It highlights the main contributions made by this research, its practical applicability, and also sheds light into some areas where attention can be given in future for further advancement of this research.

REFERENCES

- Aksoy, C. O., Onargan, T., Kucuk, K., Genis, M. and Guney, A. (2009) Numerical Modeling For the Umbrella Arch Application at the Shaft and Tunnel Intersection Paper No. 104. Proceedings of the ISRM-Sponsored International Symposium on Rock Mechanics:" Rock Characterisation, Modelling and Engineering Design Methods"(SINOROCK2009), 2009.
- Anagnostou, G., Iakovidis, G. and Vasilakopoulou, G. (1997). Athens Metro–Design and Construction of Shallow Tunnels to Control Settlements of Surface Structures. Proc. Int. Symposium Tunnelling'97, 1997 London. The Institution of Mining and Metallurgy, 709-720.
- Aşçıoğlu, G. (2007). *Analysis of Support Design Practice at Elmalik Portals of Bolu Tunnel*. M.Sc. Thesis, Middle East Technical University.
- Barisone, G., Pigorini, B. and Pelizza, S. (1982). Umbrella Arch Method for Tunnelling in Difficult Conditions-Analysis of Italian Cases.. Proceedings of the Fourth Congress International Association of Engineering Geology. New Delhi, 1982. Iv 15- Iv 27.
- Barton, N., Lien, R. and Lunde, J. (1974). Engineering Classification of Rock Masses for the Design of Tunnel Support. *Rock Mechanics*, 6, 189-236.
- Basnet, C. B. (2013). *Evaluation on the Squeezing Phenomenon at the Headrace Tunnel of Chameliya Hydroelectric Project, Nepal*. M.Sc. Thesis, NTNU Norwegian University of Science and Technology.

- Beiniawski, Z. T. (1974). Geomechanics Classification of Rock Masses and Its Application in Tunnelling. 3rd International Congress on Rock Mechanics, 1974 Denver. 27-32.
- Bhasin, R., Magnussen, A. W. and Grimstad, E. (2006). The Effect of Tunnel Size on Stability Problems in Rock Masses. *Tunnelling and Underground Space Technology*, 21, 405-406.
- Bieniawski, Z. T. (1989). *Engineering Rock Mass Classifications: A Complete Manual for Engineers and Geologists in Mining, Civil, and Petroleum Engineering*, John Wiley and Sons.
- Bobet, A. (2010). Numerical Methods in Geomechanics. *The Arabian Journal for Science and Engineering*, 35, 27-48.
- Bowles, J. E. (1996). *Foundation Analysis and Design*, England, McGraw-Hill Book Company Limited.
- Cai, M. (2008). Influence of Stress Path on Tunnel Excavation Response—Numerical Tool Selection and Modeling Strategy. *Tunnelling and Underground Space Technology*, 23, 618-628.
- Calvello, M. and Taylor, R. N. (1999). Centrifuge Modelling of a Pile-Reinforced Tunnel Heading. *Proc. Of Geotechnical Aspect of Underground Construction in Soft Rock, Tokyo*, 345–350.
- Carter, M. and Bentley, S. P. (1991). *Correlations of Soil Properties*, London, Pentech Press Publishers.
- Coduto, D. P. (2001). *Foundation Engineering: Principles and Practices*, New Jersey, Prentice-Hall.

- Coggan, J., Gao, F., Stead, D. and Elmo, D. (2012). Numerical Modelling of the Effects of Weak Immediate Roof Lithology on Coal Mine Roadway Stability. *International Journal of Coal Geology*, 90, 100-109.
- Das, B. M. (1994). *Principles of Geotechnical Engineering*, U.S.A, CI Engineering.
- Dias, D. and Oreste, P. (2013). Key Factors in the Face Stability Analysis of Shallow Tunnels. *American Journal of Applied Sciences*, 10, 1025-1038.
- Edelbro, C. (2004). *Evaluation of Rock Mass Strength Criteria*. Ph.D. Thesis. Luleå University of Technology.
- Envirosite. (2014). www.envirosite.com
- Ersoy, İ. (2011). *Analysis of Fore Poling Method Used in S14c Shaft of Kadıkoy Kartal Metro*. B.Sc. Thesis, Istanbul Technical University.
- Gibbs, P. W., Lowrie, J., Keiffer, S. and Mcqueen, L. (2002) M5 East-Design of a Shallow Soft Ground Shotcrete Motorway Tunnel. Proceeding of the 28th ITA-AITES World Tunnelling Congress, Sydney, Australia, March, 2002.
- Goodman, R. E. (1989). *Introduction to Rock Mechanics*, New York, Wiley.
- Gümüş, T. (2008). *Work Time Analysis of Umbrella Arch Support Used in the 4. Levent Ayazaga Section of Istanbul Metro*. B.Sc. Thesis., Istanbul Technical University.
- Federal Highway Administration (FHA). 2009. Technical manual for design and construction of road tunnelcivil elements.
- Hajiabdolmajid, V. and Kaiser, P. (2003). Brittleness of Rock and Stability Assessment in Hard Rock Tunneling. *Tunnelling and Underground Space Technology*, 18, 35-48.

- Hefny, A., Tan, W., Pathegama, R., Sharma, J. and Zhao, J. (2004). Numerical Analysis for Umbrella Arch Method in Shallow Large Scale Excavation in Weak Rocks. Proceedings of 30th ITA-AITES World Tunneling Congress, Singapore, 22 - 27 May 2004, 1-7.
- Hoek, E. (1994). Strength of Rock and Rock Masses. *ISRM News Journal*, 2, 4-16.
- Hoek, E. (2004). Numerical Modelling for Shallow Tunnels in Weak Rock. Unpublished Notes. Retrieved from <https://www.rocscience.com/documents/pdfs/rocnews/Spring2003/ShallowTunnels.pdf>
- Hoek, E. and Brown, E. T. (1980). *Underground Excavations in Rock*, London, Table 4-2: Classification Of RMR Values.
- Hoek, E. and Brown, E. T. (1997). Practical Estimates of Rock Mass Strength. *International Journal of Rock Mechanics and Mining Sciences*, 34, 1165-1186.
- Hoek, E., Carranza-Torres, C. and Corkum, B. (2002). Hoek-Brown Failure Criterion-2002 Edition. *Proceedings of NARMS-TAC*, 1, 267-273.
- Hoek, E. and Diederichs, M. S. (2006). Empirical Estimation of Rock Mass Modulus. *International Journal of Rock Mechanics and Mining Sciences*, 43, 203-215.
- Hoek, E., Kaiser, P. K. and Bawden, W. F. (1995). Support of Underground Excavations in Hard Rock, 1995. *Balkema, Rotterdam*, 215.
- Hung, C. J., Monsees, J., Munfah, N. and Wisniewski, J. (2009). Technical Manual for Design and Construction of Road Tunnels - Civil Elements. *Report To Us Department Of Transportation Prepared By Parsons Brinckerhoff, Inc. New York, U.S.A, FHWA-NHI-09-010*.
- Ismail, M. A. M., Azit, R., Ng, S. M., Zabidi, H., Bakhudin, N. and Nordin, Z. (2013). Evaluation of Geological Conditions Ahead of a Tunnel Face Using the Tunnel Seismic Prediction Method (TSP) Lesson Learned From the Pahang-Selangor

Raw Water Transfer Tunnel, Malaysia. *Rock Mechanics for Resources, Energy and Environment*. CRC Press. 901-907

Jing, L. (2003). A Review of Techniques, Advances and Outstanding Issues in Numerical Modelling for Rock Mechanics and Rock Engineering. *International Journal of Rock Mechanics and Mining Sciences*, 40, 283-353.

Jing, L. and Hudson, J. A. (2002). Numerical Methods in Rock Mechanics. *International Journal of Rock Mechanics and Mining Sciences*, 39, 409-427.

Juneja, A., Hegde, A., Lee, F. H. and Yeo, C. H. (2010). Centrifuge Modelling of Tunnel Face Reinforcement Using Forepoling. *Tunnelling and Underground Space Technology*, 25, 377-381.

Kavvadas, M. (2005). Numerical Analysis in the Design of Urban Tunnels. Proceedings of the 11th International Conference of IACMAG, 19-24 June 2005 Torino.

Kontogianni, V. A. and Stiros, S. C. (2002). Predictions and Observations of Convergence in Shallow Tunnels: Case Histories in Greece. *Engineering Geology*, 63, 333-345.

Latif, M. F. A., Ismail, M. A. M., Selamat, M. R. and Ng, S. M. (2012). Effects of Pipe Roof Supports and the Tunnels Excavation on the Ground Settlement. *Electronic Journal of Geotechnical Engineering*, 18 (Bundle E), 1045-1056.

Lee, S. C., Ismail, M. A. M. and Ng, S. M. (2012). The Evaluation of Tunnel Behaviors under High Rock Stress Using Numerical Analysis Method. *Electronic Journal of Geotechnical Engineering*, Vol. 17, Bund. T. 3605-3626.

Likar, J., Volkmann, G. and Button, E. (2004). New Evaluation Methods in Pipe Roof Supported Tunnels and Its Influence on Design during Construction. EUROCK 2004 and 53rd Geomechanics Colloquium, October 6-8, 2004 2004 Salzburg, Austria, 277-282.

- Look, B. G. (2007). *Handbook of Geotechnical Investigation and Design Tables*, London, Taylor and Francis.
- Martins, J. B. (2001). A Short Survey on Construction Problems and Numerical Modelling of Shallow Tunnels. *Engenharia Civil-University of Minho*, 7-20.
- Miura, K. (2003). Design and Construction of Mountain Tunnels in Japan. *Tunnelling and Underground Space Technology*, 18, 115-126.
- NAVFAC (1986). NAVFAC Design Manual 7.2 - Foundations and Earth Structures. In: Command, N. F. E. (Ed.). Virginia: Naval Facilities Engineering Command.
- Nilsen, B. and Palmström, A. (2000). *Engineering Geology and Rock Engineering: Handbook*, Norwegian Group for Rock Mechanics.
- Nordin, Z. (2010). Pahang – Selangor Raw Water Transfer Project. *The Ingenieur*, 47, 42-53.
- Obrzud, R. and Truty, A. (2012). The Hardening Soil Model-A Practical Guidebook Z Soil. Pc 100701 Report.
- Ocak, I. (2008). Control of Surface Settlements with Umbrella Arch Method in Second Stage Excavations of Istanbul Metro. *Tunnelling and Underground Space Technology*, 23, 674-681.
- Oke, J., Vlachopoulos, N. and Marinos, V. (2014). Umbrella Arch Nomenclature and Selection Methodology for Temporary Support Systems for the Design and Construction of Tunnels. *Geotechnical and Geological Engineering*, 32, 97-130.
- Oreste, P. P. and Peila, D. (1998). A New Theory for Steel Pipe Umbrella Design in Tunneling. Proc. Of The 24th ITA-AITES World Tunnelling Congress,

Tunnels And Metropolises. Eds. A. Negro Jr and Aa Ferreira, Sao Paulo, Brazil, 1998. 1033-1039.

Özer, S. C. (2011). *Determination of Optimum Umbrella Arch Size by Numerical Modelling in Tunnel Excavations*. M.Sc. Thesis, Dokuz Eylül University.

Palmström, A. (1995). *RMi – A Rock Mass Characterisation System for Rock Engineering Purposes*. Ph.D. thesis, University Of Oslo.

Park, J.-J., Cho, I.-S., Lee, I.-M., and Lee, S.-W. (2012). Tunnel Reinforcement by Using Pressure-Induced Inflatable Pipes Method. *Journal of Geotechnical and Geoenvironmental Engineering*, 138, 1483-1491.

Prountzopoulos, G. (2011). Tunnel Face Reinforcement and Protection–Optimization Using 3D Finite Element Analyses. Proceedings Of The 21st European Young Geotechnical Engineers Conference, The Netherlands, Rotterdam, 4-7 September, 2011, 33-39.

Rabcewicz, L. V. (1964). The New Austrian Tunnelling Method. *Water Power*, 16, 453-457.

Ravandi, E. G. and Rahmanned, R. (2013). Wall Displacement Prediction of Circular, D Shaped and Modified Horseshoe Tunnels in Non-Hydrostatic Stress Fields. *Tunnelling and Underground Space Technology*, 34, 54-60.

Rezaei, A. H., Katebi, H., Hajjalilue-Bonab, M. and Hosseini, B. (2013). The Influence of Buildings and Ground Stratification on Tunnel Lining Loads Using Finite Element Method. 18th International Conference on Soil Mechanics and Geotechnical Engineering, 2013 Paris. 789-792.

Rocscience. (2014). www.rocscience.com

Rockware. 2014. www.rockware.com

- Sadek, M. and Shahrour, I. (2004). A Three Dimensional Embedded Beam Element for Reinforced Geomaterials. *International Journal for Numerical and Analytical Methods in Geomechanics*, 28, 931-946.
- Santi, P. M. (2006). Field Methods for Characterizing Weak Rock for Engineering. *Environmental and Engineering Geoscience*, 12(1), 1-11.
- Sarathchandran, A. (2014). *Three Dimensional Numerical modelling Of Coal Mine Roadways Under High Horizontal Stress Fields*. M.Sc. Thesis, University of Exeter.
- Shin, C. B., Chun, B. S., Park, D. and Jung, J. J. (2006). Construction of Double Track Subway Tunnel in the Alluvial Soils with Shallow Overburden. *Tunnelling and Underground Space Technology*, 21, 388-388.
- Shin, H. S., Han, K. C., Sunwoo, C., Choi, S. O. and Choi, Y. K. (1999) Collapse Of a Tunnel in Weak Rock and the Optimal Design of the Support System. 9th ISRM Congress, 1999. International Society for Rock Mechanics.
- Shin, J.-H., Choi, Y.-K., Kwon, O.-Y., and Lee, S.-D. (2008). Model Testing for Pipe-Reinforced Tunnel Heading in a Granular Soil. *Tunnelling and Underground Space Technology*, 23, 241-250.
- Shinji, M., Akagi, W., Shiroma, H., Yamada, A. and Nakagawa, K. (2002). JH Method of Rock Mass Classification for Tunnelling. International Society for Rock Mechanics.
- Simanjuntak, T. D. Y. F., Marence, M., Schleiss, A. J. and Mynett, A. E. (2012). Design of Pressure Tunnels Using a Finite Element Model. *Hydropower and Dams*, 19, 98-105.
- Song, K. I., Cho, G. C., Sim, Y. J. and Lee, I. M. (2006). Optimization of a Pre-Improvement Support System for Large Underground Excavation. *Tunnelling and Underground Space Technology*, 21, 374.

- Süzen, E. (2009). *Application of Excavation and Reinforcement in the 4. Levent – Ayazaga (Istanbul) Tunnel*. M.Sc. Thesis, University of Cukurova.
- Truzman, M., Corley, D. and Lipka, D. (2011). Determination of Unit Tip Resistance for Drilled Shafts in Fractured Rocks Using the Global Rock Mass Strength. *2011 Pan-Am CGS Geotechnical Conference*. Toronto.
- Tuncdemir, H., Aksoy, C., Güçlü, E., & Özer, S. (2012). *Umbrella arch and forepoling support methods: a comparison*. Paper presented at the ISRM International Symposium-EUROCK 2012, 28-30 May 2012, Stockholm, Sweden, 1-13
- Usta, E. (2005). *The Engineering Geology in Between Yenikapi and Unkapani in Istanbul Subway*. M.Sc. Thesis, Istanbul Technical University.
- Vardakos, S. S., Gutierrez, M. S. and Barton, N. R. (2007). Back-Analysis of Shimizu Tunnel No. 3 by Distinct Element Modeling. *Tunnelling and Underground Space Technology*, 22, 401-413.
- Vestad, M. L. (2014). *Analysis of the Deformation Behavior at the Underground Caverns of Neelum Jhelum HPP, Pakistan*. M.Sc. Thesis, NTNU Norwegian University of Science and Technology.
- Volkman, G. M. and Schubert, W. (2006). Contribution to the Design of Tunnels with Pipe Roof Support. 4th Asian Rock Mechanics Symposium, ISRM International Symposium. Eds. Leung Cf and Zhou Yx, 2006. 981-270.
- Volkman, G. M. and Schubert, W. (2007). Geotechnical Model for Pipe Roof Supports in Tunneling. Proc. Of The 33rd ITA-AITES World Tunneling Congress, Underground Space–The 4th Dimension Of Metropolises, 2007. 755-760.
- Volkman, G. M., Schubert, W. and Button, E. A. (2006) A Contribution to the Design of Tunnels Supported by a Pipe Roof. Golden Rocks 2006, The 41st US Symposium On Rock Mechanics (USRMS), 2006. American Rock Mechanics Association.

- Wakita, S., Date, K., Yamamoto, T., Yanagisawa, H. and Uesugi, N. (2004). Effective Grouting Materials for Tunneling through Unconsolidated Ground. Proceedings Of The 30th ITA-AITES World Tunnel Congress Singapore, 22-27 May 2004, 2004.
- Wang, H. (2012). Optimization of Pipe Roof Reinforcement Applied in Tunnel Construction Under Complex Conditions. *Electronic Journal of Geotechnical Engineering*, 17 (Bundle C), 301-310.
- Wang, H. and Jia, J. (2009). Face Stability Analysis of Tunnel With Pipe Roof Reinforcement Based On Limit Analysis. *Electronic Journal of Geotechnical Engineering*, 14 (Bundle G), 1-15.
- Wittke, W. (2001). *Osterfeld Tunnel, Advancing Vault Excavation with Closed Invert in a Rock Mass with High Horizontal Stresses*, A.A. Balkema Publishers.
- Yahya, S. M. and Abdullah, R. A. (2014). A Review on Methods of Predicting Tunneling Induced Ground Settlements. *Electronic Journal of Geotechnical Engineering*, Vol. 19, Bund. T, 5813-5826.
- Yeo, C. H., Lee, F. H., Tan, S. C., Hasegawa, O., Suzuki, H. and Shinji, M. (2009). Three Dimensional Numerical Modelling of a NATM Tunnel. *International Journal of the JCRM*, 5, 33-38.
- Yoo, C. and Shin, H.-K. (2003). Deformation Behaviour of Tunnel Face Reinforced With Longitudinal Pipes—Laboratory and Numerical Investigation. *Tunnelling and Underground Space Technology*, 18, 303-319.
- Zeeshan, B. A., De Stefano, A. and Invernizzi, S. (2013). Effects Of Using Innovative Seismic Isolation Technique On Masonry: Tunnelling Work Required. *Earthquake Resistant Engineering Structures Ix*, 132, 369.

Zeidler, K. and Hun, Y. C. (2007). FE Modelling For the Shallow Fort Canning Tunnel. ECCOMAS Thematic Conference On Computational Methods In Tunneling, Euro: Tun 2007, August 27-29, 2007 2007 Vienna, Austria. 1-19.