

THE EFFECT OF ISOLATED DAMPING LAYER SYSTEM ON EARTH DAM  
UNDER EARTHQUAKE LOADING

BEHROUZ GORDAN

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Universiti Teknologi Malaysia

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## **DEDICATION**

To my respectful parents and beloved wife Tayebah Alipour as well as my son Arian Gordan

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## ABSTRACT

The structural behavior during an earthquake is one of the major concerns for earth dam of a medium size about 30 meter height and 90 meter width. The body crack is created by relative vertical displacement at both edges of the crest. The failure is recorded with the crack development in dam body by interaction between dam and reservoir. To reinforce dams, some methods were used with respect to literature such as perpendicular drain, prefabricated vertical drain, geotextile layers, pile group, micro pile injection and cutoff wall system. This research included three objectives; (i) Identifying damage location in earth dam with respect to case study (Bakun dam), (ii) Studying the effect of Isolated Damping Layer (IDL) system in blanket layer using physical modeling on top of the vibrator table, and (iii) Evaluating slope stability based on seismic motion. In terms of methodology, Finite-Element method using ANSYS13 program and equilibrium method using Geostudio 2007 (Slope/W) were used. Series of soil mechanic test to design IDL and small-scale model (1/100) using IDL were carried out. Displacements, shear stresses and shear strains of dam were evaluated using nonlinear analysis under strong earthquake intensity of 0.6g. The major effect on the displacement of dam was due to different foundation properties (soft, medium and stiff soil) in comparison to different core configuration in terms of geometry. The best elastic modulus ratio between unsaturated part of dam and foundation,  $\beta$  was 0.66 for saturated part and foundation,  $\lambda$  was 0.13 in order to reduce response of the earth dam. Time-history and response spectra analysis of Bakun dam showed the minimum relative vertical displacement between both edges of crest by peak ground acceleration less than 0.24g. For all site classes, the displacement ratio ( $\Delta=2$ ) for return earthquake period from 2500 to 500 years was recorded. Based on modal analysis, the rigid behavior of foundation was achieved by modulus ratio more than three. Effect of modulus ratio on dominant frequency was greater than depth ratio. The minimum relative vertical displacement was attained when modulus elasticity ratio between shell and core clay was less than five. The optimal behavior was obtained by using clay in blanket layer when a modulus elasticity ratio was equal to 2.50, between this layer and weak foundation. The blanket layer was designed based on mixed product of laterite soil with shredded tire and micro silica. The main role of silica was to control seepage. The qualified combination by comparison of thirteen samples was distinguished. Subsequently, nine physical models were vibrated using dominant frequency. Most of damage occurred at upstream of one third near to the crest. The best absorption of energy without any destruction was observed when the layer thickness of reinforced blanket was one fourth of dam height. The safety factor was increased using blanket reinforced layer. Finally, IDL system showed the best performance in order to reinforce dam under resonance seismic motion.

## ABSTRAK

Pelakuan struktur semasa gempa bumi adalah salah satu daripada kebimbangan utama bagi empangan bumi saiz sederhana untuk ketinggian kira-kira 30 meter dan 90 meter lebar. Keretakan pada empangan tanah disebabkan oleh anjakan tegak relatif pada penjuru struktur tersebut. Kegagalan struktur direkodkan bersama dengan retak dalam badan empangan oleh interaksi antara empangan dan takungan. Untuk mengukuhkan empangan, beberapa kaedah telah digunakan oleh penyelidik yang lepas seperti longkang serenjang, pasang siap longkang menegak, lapisan geotekstil, kumpulan cerucuk, suntikan cerucuk mikro dan sistem dinding potong. Terdapat tiga objektif kajian; (i) mengenalpasti lokasi kerosakan dalam empangan bumi seperti dalam kajian kes (Empangan Bakun), (ii) mengkaji kesan Isolated Damping Layer IDL sistem dalam lapisan selimut menggunakan model fizikal di atas meja penggeggar dan (iii) menilai kestabilan cerun berdasarkan gerakan seismik. Untuk menjalankan kajian ini, kaedah Unsur-Terhingga oleh program ANSYS13 dan kaedah keseimbangan dalam Geostudio 2007 (Slope/W) telah digunakan. Beberapa siri ujian mekanik tanah dibuat untuk mereka bentuk (IDL) dan skala kecil model (1/100) menggunakan IDL juga dibuat. Anjakan, tegasan ricih dan tekanan ricih empangan telah dinilai daripada analisis tidak linear di bawah keamatan gempa bumi 0.6g. Kesan yang besar ke atas anjakan empangan adalah kerana sifat-sifat asas yang berbeza (tanah lembut, sederhana dan keras) berbanding dengan konfigurasi teras yang berbeza dari segi geometri. Nisbah modulus elastik antara bahagian tepu empangan dan asas,  $\beta$  adalah 0.66 dan antara bahagian tepu dan asas,  $\lambda$  adalah 0.13 untuk mengurangkan tindak balas empangan bumi. Masa sejarah dan analisis spektrum gerak balas empangan Bakun menunjukkan anjakan tegak relatif minimum antara kedua-dua tepi puncak oleh pecutan bumi puncak kurang daripada 0.24g. Selain itu, nisbah anjakan ( $\Delta=2$ ) untuk kembali pada tempoh gempa bumi dari 2500 ke arah 500 tahun untuk semua kelas tapak direkodkan. Sehubungan dengan analisis modal, tingkah laku tegar asas dicapai oleh nisbah modulus lebih daripada tiga. Anjakan minimum menegak relatif dicapai apabila nisbah modulus keanjalan antara cengkering dan teras tanah liat adalah kurang daripada lima. Tingkah laku yang optimum ditunjukkan dengan menggunakan tanah liat pada lapisan selimut apabila nisbah modulus keanjalan adalah sama dengan 2.50, antara lapisan ini dan asas tapak yang lemah. Lapisan selimut telah dibuat berdasarkan campuran produk daripada tanah laterit bersama hirisan tayar dan mikro silika. Peranan utama silika adalah untuk mengawal resapan. Kombinasi terbaik diperolehi daripada perbandingan lima belas sampel. Selepas itu, sembilan model fizikal telah digetarkan. Kebanyakan kerosakan berlaku di bahagian satu pertiga puncak. Penyerapan terbaik oleh tenaga tanpa apa-apa kemusnahan diperhatikan apabila ketebalan lapisan penutup bertetulang adalah satu perempat daripada ketinggian empangan. Selain itu, faktor keselamatan telah meningkat dengan lapisan selimut bertetulang. Akhirnya, system IDL menunjukkan kelakunan terbaik bagi memperkuat empangan di bawah gegaran sismik resonan.

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**LIST OF SYMBOLS**

$\delta$	- Displacement
$\dot{\delta}$	-Velocity
$\ddot{\delta}$	-Acceleration
T	-Priod
f	-Frequency
$\sigma$	-Stress
$\tau$	-Shear stress
$\epsilon$	-Strain
$\theta$	-Slope
$\gamma$	-Density
$\phi$	-Angle of friction
C	-Cohesion
SF	-Safety factor
PGA	-Peak ground acceleration
°	-Degree
$\zeta$	-Damping ratio
$\nu$	-Poisson's ratio
E	-Modulus elasticity
$\lambda$	-Gradient
2D	-Two dimensional analysis
3D	-Three dimensional analysis
FEM	-Finite Element Method
FDM	-Finite Difference Method
$\omega$	-Natural frequency of the system
BS	-British Standard

H	-Height
W	-Width
Hz	-Hertz
$m_v$	-Coefficient of Volume Compressibility
$c_v$	-Coefficient of Consolidation
g	-Gravity

**LIST OF APPENDIX**

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## **CHAPTER 1**

### **INTRODUCTION**

#### **1.1 Introduction**

Nowadays, dam construction is critical trend in the world in order to access some requirements. The major purposes defined by water supply and electricity. In this context, some important aspects investigated by initial phase of design. One of the main problems is structural behavior during an earthquake. Besides, there are some case studies of damages. The earth dam damaged while some type of body cracks made in dam or foundation. Some phenomena occurred such as overflow, piping and structural failure in parallel to development of cracks. In brief, evaluation of earth dam in order to earthquake effects is one of the major purposes of design.

Dynamic analysis of earth dam is one of the main purposes through design process. According to the literature, there are some studies in this area like reinforcement techniques, shaking table test, data monitoring and numerical analysis. In terms of the numerical method, two famous methods such as Finite-element or Finite-difference are used. Besides, not only the effect of material properties on dynamic trend but also comparison of the two and three-dimensional analysis were reviewed. In addition, the distribution of frequency and acceleration evaluated for

structure. In terms of earth dams under seismic load or earthquake, the integrated response is the increase of acceleration and displacement at the crest based on nonlinear aspect. The main role of this process appeared by interaction between dam and reservoir. It seems to be that, improvement of earth dam behavior with reinforced techniques is required.

Seismic-resistant capacity of the earth dams is a great issue within earthquake active zone. However, dynamic behavior is one of the main concerns. For a realistic evaluation of the seismic risk, one must consider some uncertainties. They are some major aspects such as site geology, material stiffness, and analysis method. In addition, depth investigation in this domain indicated that numerical methods applicably used to assess dynamic behavior during the earthquake. Moreover, the numerical results verified by some experimental tests like shaking table and centrifuge.

## **1.2 Problem Statement**

In terms of problem statement, failure mechanism is the main problem in earth dams. Before failure, some significant factor like freeboard, overflow and piping that should critically controlled by design approach. Overflow is very huge danger for dam, and should avoid. In fact, most of the reports in order to damage in dam related to the overflow. Therefore, freeboard design is very important to control overflow. It is important to note that the wave height in reservoir increased during earthquake. Moreover, interaction between dam and reservoir is very effective to dam behavior under seismic load. It is worth noting that piping is other problem in this category. This phenomenon related to the body cracks. In addition, body cracks are directly corresponded to deformation during an earthquake. In this case, the plastic deformation created by relative displacement. After all, the main goal in order to control dynamic behavior is the relative displacement during earthquake with respect to damage.

Lower San Fernando dam suffered an underwater slide during the San Fernando earthquake, 1971. Fortunately, the dam barely avoided collapse, thereby

preventing a potential disaster of flooding of the heavily populated areas below the dam[Karl V, 1971].

Two decades later, the 1994 Northridge earthquake put the Los Angeles Dam with concrete face rock-fill dam (CFRD) to the test [Robert et al, 1994]. The Northridge earthquake was almost equal in magnitude to the previous San Fernando earthquake. Ground shaking was very strong, with amplitudes among the highest ever recorded but consistent with the USGS estimates. Yet the dam showed only minor deformation and superficial cracking. Despite the intense shaking, the crest of the dam moved only 1 inch sideways and settled only 3.5 inches. Moreover, longitudinal crack reported in Fengshou reservoir dam. Dam was 200 meters (656 feet) long, 0.6 meters (2 feet) wide and 3 meters (10 feet) deep at its largest degree.

Furthermore, On May 12, 2008, a strong earthquake with 8.0 Richter scale jolted Wenchuan County in Sichuan province of China [Xu Zeping, 2008]. Zipingpu concrete faced rock-fill dam, which is only 17 km away from the epicenter, survived from the earthquake. However, it is also suffered severe damages during the strong earthquake.

### **1.3 Aim of Research**

The main aim of this research work is improvement of earth dam behavior under earthquake by new reinforcement technique. This study tried to introduce Isolator Damper Layer (IDL) system in order to reinforce dam with respect to increase resistance under the strong earthquake.

### **1.4 Objectives of Research**

To achieve such aim the following objectives are considered for the research work:

- i. To identify the location of damage by evaluating the effect of material properties in dam body and performing vibration analysis, time history and response spectrum analysis.
- ii. To study the effect of blanket layer using Isolated Damping Layer (IDL) system between dam and foundation to control dynamic behavior by investigating material properties, layer thickness and reinforcement arrangement.
- iii. To evaluate slope stability in earth dam by evaluating the safety factor under static and dynamic load conditions.

### **1.5 Scope of Research**

This scope covers all objectives as mentioned in last section. Finite –Element method (FEM) is performed using Ansys13 program for numerical analysis such as time-history and response spectra in order to consider the location of damage. In parallel, evaluation of slope stability by Geostudio 2007 (Slope/W) program was performed utilizing equilibrium method to compute safety factor in both static and dynamic load conditions. Moreover, the British standard is applied for IDL geotechnical tests. Furthermore, for small-scale physical modeling, the short homogenized dam of 16.5 meter with scale ratio (1/100) is tested on top of the vibrator table. In addition, in terms of critical situation for earthquake effect, the resonance condition is evaluated according to dominant frequency. The vibrator table functions in one dimension (vertical motion) only and the duration is two minutes for all samples. The reason for two minutes is about background in Sumatra fault that is near to Malaysia. In terms of limitation for this research, Local soil (Laterite) is used from campus of the University Teknologi Malaysia. Vibrator table is used with capacity equal 250 kg. Data logger is used to record data in each two seconds during the vibration.

### **1.6 Significance of the Research**

This research covers earthquake effect on some structures such as earth dam; homogenize embankment, concrete face rock-fill dam and embankment-bridge. This research also includes a case study for Bakun dam in the east of Malaysia. In addition, this research is the pioneer study to introduce new material (IDL), as can be used to increase structural resistance under seismic load. According to the use of blanket layer reinforced with new material (IDL), dam performance during the strong intensity of ground motion like resonance condition is very good. It is worth noting that, this material can use for different structures based on next study.

### **1.7 Organization of thesis**

The organization of the thesis can described briefly as follows:

- Chapter 1 is the Introduction, which explains on the background, the aim, objectives, and scope.
- Chapter 2 is on Literature review that explains analysis of earth dam during an earthquake. This chapter included comprehensive review in some methods such as Finite-Element method (FEM), Finite-Difference method (FEM), Plane stress (3D), Plan strain (2D), Free vibration analysis, Time-history analysis, Response spectra analysis, Reinforcement techniques, Data monitoring and Shaking table test.
- Chapter 3 is Research Methodology that explains the methodology to complete the research besides the data collection and analysis technique used in this study.
- Chapter 4 is Analytical and experimental tests that includes dominant frequency and slope stability in dam, experimental test to design Isolated Damping Layer (IDL) and small-scale physical modeling.
- Chapter 5 is Time-history analysis with case study (Bakun dam).
- Chapter 6 is conclusion and recommendation that explains the significance of the research finding including recommendation or suggestion and benefit of the research for future study.



## **1.8 Summery**

Introduction of thesis presented in this chapter. In addition, problem statement and aim of the research are described. Moreover, objectives of study based on scope with are explained. Finally, the significant points of present study and thesis organization are presented in this chapter.

## References

- Abusharar, S. W., Zheng, J. J., Chen, B. G., & Yin, J. H. (2009). A simplified method for analysis of a piled embankment reinforced with geosynthetics. *Geotextiles and Geomembranes*, 27(1), 39-52.
- Adnan, A., Hendriyawan., Sunaryati, J., Suhana, S., Sophia, A. and Norsurian, BA, R. (2005). Siesmic Hazard Assessmenet for Bakun Hydroelectriv Project. *Structural Earthquake Engineering Research University teknologi Malaysia*
- Ahmad, M. H., Noorzaie, J., & Al Qbadi, F. (2008) Principal Stresses in non-linear analaysis of Bakun concrete faced rockfill dam. *AJSTD Vol. 25 Issue 2 pp. 469-479*
- ASCE 2007, seismic design chapter 11.
- Baker. R. (2006). A relation between safety factors with respect to strength and height of slopes. *Computers and Geotechnics*; 33: 275–277, 2006.
- Bayraktar, A., Kartal, M. E., & Adanur, S. (2011). The effect of concrete slab–rockfill interface behavior on the earthquake performance of a CFR dam. *International Journal of Non-Linear Mechanics*, 46(1), 35-46.
- Berhe, T. G., Wang, X. T., & Wu, W. (2010, June). Numerical Investigation into the arrangement of Clay Core on the Seismic Performance of Earth Dams. *In Soil Dynamics and Earthquake Engineering* (pp. 131-138). ASCE.
- Bishop. A.W. (1955). The use of slip circle in the stability analysis of slopes. *Geotechnique*; 5(1): 7–17, 1955. Board on Hydraulic structure department, Modern methods concerning the dam static and dynamic analysis (1977). *Civil engineering institute*. Bucharest, Romania.
- Borges, J. L. (2004). Three-dimensional analysis of embankments on soft soils incorporating vertical drains by finite element method. *Computers and Geotechnics*, 31(8), 665-676.

- Chakraborty, D., & Choudhury, D. (2009). Investigation of the behavior of tailings earthen dam under seismic conditions. *American Journal of Engineering and Applied Sciences*, 2(3), 559.
- Chin, L. C. (2004). A Study on Concrete Faced Rockfill Dams. University of Southern Queensland Faculty of Engineering and Surveying
- Cho. S. E. (2009). Probabilistic stability analyses of slopes using the ANN-based response surface. *Computers and Geotechnics*; 36: 787–797, 2009.
- Chopra, A. k. and Perumalswami, P. R.(1971). Dynamic of rock and earthfill dams with foundation interaction. *Journal of the Eng Mech. Div*, April.
- Chowdhury, I., & Dasgupta, S. P. (2003). Computation of Rayleigh damping coefficients for large systems. *The Electronic Journal of Geotechnical Engineering*, 8(0).
- Clough. R.W. and Chopra. A.K. (1966). Earthquake stress analysis in earth dams. *J. Eng. Mech., ASCE.*, 92: 197-211. <http://nisee.berkeley.edu/elibrary/Text/300451>.
- Das, B. M. (2008). *Advanced soil mechanics*. Psychology Press.
- Das, B. M., & Ramana, G. V. (2010). Principles of soil dynamics. *Cengage Learning, USA*, 128-129.
- Duncan, J. M., & Chang, C. Y. (1970). Nonlinear analysis of stress and strain in soils. *Journal of the Soil Mechanics and Foundations Division*, 96(5), 1629-1653.
- El-Ramly. H., Morgenstern. N.R. and Cruden. D.M. (2002). Probabilistic slope stability analysis for practice. *Canadian Geotechnical Journal*; 39 (3): 665–683.
- Elgamal, A. W. (1992). Three-dimensional seismic analysis of La Villita dam. *Journal of geotechnical engineering*, 118(12),1937-1958.
- Fellenius. W. (1936). Calculations of the stability of earth dams, in: *Transactions of the 2<sup>nd</sup> Congress on Large Dams*. Washington, DC, vol. 4, p. 445, 1936.
- Finn, W. D. L., Yogendra kumar, M., Yoshida, N. and Yoshida, H. (1986). TARA-3: A Program for Nonlinear Static and Dynamic Effective Stress Analysis. *Department of Civil Engineering, University of British Columbia, Vancouver, British Columbia, Canada*.
- Garnier, J.; Gaudin, C.; Springman, S.M.; Culligan, P.J.; Goodings, D.J.; Konig, D.; Kutter, B.L.; Phillips, R.; Randolph, M.F. and Thorel, L. (2007), Catalogue of scaling laws and similitude questions in geotechnical centrifuge modelling 7 (3), *International Journal of Physical Modelling in Geotechnics*, pp. 1–23, ISSN: 1346-213X, E-ISSN: 2042-6550

- GEO-SLOPE International Ltd, Calgary, Alberta, Canada [www.geos-slope.com](http://www.geos-slope.com). WABA Dam Permanent Deformation due to an earthquake.
- Geostudio. (2007). Manuel. [www. Geo-slope.com](http://www.Geo-slope.com)
- Gikas, V., & Sakellariou, M. (2008). Settlement analysis of the Mornos earth dam (Greece): evidence from numerical modeling and geodetic monitoring. *Engineering Structures*, 30(11), 3074-3081.
- Griffiths. D.V., and Fenton. G.A. (2004). Probabilistic slope stability analysis by finite elements. *Journal of Geotechnical and Geoenvironmental Engineering*; 130 (5): 507- 518.
- Hayashi, M., Fuziwara, Y and Komada, H. (1975). Dynamic viscosity and dynamic deformability of rock-fill material in laboratory, application to response analysis and comparison with observed damping during earthquakes. *Proc. of symp. On criteria and assumptions for numerical analysis of dams*. Swansea.
- Hwang, J. H., Wu, C. P., & Chou, J. T. (2008, May). Motion Characteristics of Compacted Earth Dams under Small Earthquake Excitations in Taiwan. In *Geotechnical Earthquake Engineering and Soil Dynamics IV* (pp. 1-12). ASCE.
- IITK-GSDMA Guidelines for seismic design of earth dams and Provisions with Commentary and Explanatory Examples(August 2005; Revised May 2007). *Indian institute of technology Kanpur*. India.
- Ionescu, S. (1977). Contributions to the analysis of rockfill dams sealed with non-earthly materials (in Romanian). *Doctoral Paper, ICB*. Romania.
- Jahed. H., Noban. M. R., and Eshraghi. M. A. (2009). ANSYS Finite Element. *Tehran University*
- Janbu. N. (1973). Slope stability computations, *in: R.C. Hirshfield, S.J. Poulos (Eds.), Embankment Dam Engineering*. Cassagrande Volume, John Wiley and Sons; 47–86, 1973.
- Karl. V., Steinbrugge Collection, Earthquake Engineering Research Center, University of California, Berkeley.
- Kong, X. J., Zhou, Y., Xu, B., & Zou, D. G. (2010). Analysis on seismic failure mechanism of zipingpu dam and several reflections of aseismic design for high Rock-fill dam. *Earth and Space*, 3177-3189.
- Le Hello, B., & Villard, P. (2009). Embankments reinforced by piles and geosynthetics— Numerical and experimental studies dealing with the transfer of load on the soil embankment. *Engineering Geology*, 106(1), 78-91.

- Li, K.S., and Lumb, P. (1987). Probabilistic design of slopes. *Canadian Geotechnical Journal*; 24: 520–535, 1987.
- Low, B., Tang, S., and Choa, V. (1994). Arching in Piled Embankments. *J. Geotech. Engrg.*, 120(11), 1917–1938.
- Matsumaru, T., Watanabe, K., Isono, J., Tateyama, M. and Uchimura, T. (2008) Application of cement-mixed gravel reinforced by ground for soft ground improvement. *Proceedings of the 4th Asian Regional Conference on Geosynthetics June 17 - 20, 2008*. Shanghai, China.
- Maosong, H. and Cang-Qin, J. (2009). Strength reduction FEM in stability analysis of soil slopes subjected to transient unsaturated seepage. *Computers and Geotechnics*; 36: 93–101.
- Marto, A., Latifi, N., & Sohaei, H. (2013). Stabilization of laterite soil using GKS soil stabilizer. *Electron J Geotech Eng (EJGE)*, 18, 521-532.
- Mejia, L. H., & Seed, H. B. (1983). Comparison of 2-D and 3-D dynamic analyses of earth dams. *Journal of Geotechnical Engineering*, 109(11), 1383-1398.
- Mizokoshi, T and Minura, S. (1975). Studies on the earthquake forces in the design of Takase dam. *Proc. of Symp. on Criteria and assumption for numerical analysis of dams*. Swansea.
- Morgenstern, N. R. and V.E. Price. (1965). The analysis of the stability of general slip surface. *Geotechnique*; 15 (4): 289–290, 1965.
- Namdar, A. and Pelko, A. K. (2010). Seismic Evaluation of Embankment Shaking Table Test and Finite Element Method. *The Pacific Journal of Science and Technology - volume11, Number2, November*.
- Newmark, M. N. and Rosenblueth, E. (1971). Fundamental of earthquake engineering. *Mc Graw-Hill*, New Jersey.
- Noorzad, R., & Omidvar, M. (2010). Seismic displacement analysis of embankment dams with reinforced cohesive shell. *Soil Dynamics and Earthquake Engineering*, 30(11), 1149-1157.
- Nose, M., Takahashi, T and Kunii, K. (1976). Results of earthquake observation and dynamic tests on rockfill dams and their consideration. *12-th ICOLD Congress*, Mexico.
- Okamoto, S. (1973). Introduction to earthquake engineering. *Tokyo University of Tokyo Press*.

- Okamoto, S., Tamura, C., Kato, K. and Ohmachi, T.(1974). A study on the dynamic stability of rockfill dams during earthquakes based on vibration failure tests of models. *Bull of ERS*, University of Tokyo.
- Özkan, M. Y., Erdik, M., Tuncer, M. A., & Yilmaz, C. (1996). An evaluation of Sürgü dam response during 5 May 1986 earthquake. *Soil Dynamics and Earthquake Engineering*, 15(1), 1-10.
- Palmeria, E. M., Pereira, J. H.F and da Silva, A. R.L. (1998). Back analyses of geosynthetic reinforced embankments on soft soils. *Geotextiles and Geomembrances* 16 (1998) 273-292.
- Papalou, A., & Bielak, J. (2001). Seismic elastic response of earth dams with canyon interaction. *Journal of geotechnical and geoenvironmental engineering*,127(5), 446-453.
- Parish, Y., & Abadi, F. N. (2009). Dynamic Behaviour of Earth Dams for Variation of Earth Material Stiffness. *World Academy of science Engineering and Technology*, 50, 2009.
- Papalou, A., & Bielak, J. (2001). Seismic elastic response of earth dams with canyon interaction. *Journal of geotechnical and geoenvironmental engineering*,127(5), 446-453.
- Popovici, A. (1978). Hydrotechnical structure analysis of large structure (dams, dikes, etc.). *doctoral paper, ICB*. Romania.
- Priscu, R. (1974). Hydrotechnical construction (in Romanian). *Editura didactica si pedagogica, Bucharest*.
- Priscu, R. (1985). Earthquake engineering for large dams.
- Priscu, R., Lonescu, S and Stematiu. (1978). A new model for movement analysis of rockfill dams “*L’Energia Elettrica*”, Milan.
- Robert. A., Page, David M., Boore , Robert F, Yerkes (1994). The Los Angeles Dam Story. *USGS*. <http://earthquake.usgs.gov>
- Sarma. S. K. (1973). Stability analysis of embankments and slopes. *Geotechnique*; 23 (3): 423–433.
- Sarma. S. K. (1979). Stability analysis of embankments and slopes. *Journal of the Geotechnical Engineering Division*. ASCE; 105 (12): 1511–1524, 1979.
- Schanz, T., Vermeer, P. A., & Bonnier, P. G. (1999). The hardening soil model: formulation and verification. *Beyond 2000 in computational geotechnics*, 281-296.

- Seed, B. H. (1973). Stability of earth and rock-fill dams during earthquakes. *Embankment dam engineering*, Prentice-Hall. New York.
- Seed, H. B. (1979). Considerations in the Earthquake Resistant Design of Earth and Rockfill Dams. *Geotechnique*, 29(3), pp. 215-263.
- Seed, H. B., Wong, R. T., Idriss, I. M., & Tokimatsu, K. (1986). Moduli and damping factors for dynamic analyses of cohesionless soils. *Journal of Geotechnical Engineering*, 112(11), 1016-1032.
- Sivakumar. B. G. L., Srivastava. A., and Sahana. V. (2007). Analysis of stability of earthen dams in kachchh region, Gujarat, India. *Engineering Geology* 94(2007)123-136.
- Siyahi, B., & Arslan, H. (2008). Earthquake induced deformation of earth dams. *Bulletin of Engineering Geology and the Environment*, 67(3), 397-403.
- Spencer. E. (1967). A method of analysis of the stability of embankments assuming parallel inter-slice forces. *Geotechnique*; 17 (1): 11–26, 1967.
- Toritsu, S. S., Sato, J., Towhata, I., & Honda, T. (2010). 1-G model tests and hollow cylindrical torsional shear experiments on seismic residual displacements of fill dams from the viewpoint of seismic performance-based design. *Soil Dynamics and Earthquake Engineering*, 30(6), 423-437.
- Tsai, P. H., Hsu, S. C., & Lai, J. (2009, August). Effects of Core on Dynamic Responses of Earth Dam. In *Slope Stability, Retaining Walls, and Foundations@ Selected Papers from the 2009 GeoHunan International Conference* (pp. 8-13). ASCE.
- Wang, L., Zhang, G., & Zhang, J. M. (2011). Centrifuge model tests of geotextile-reinforced soil embankments during an earthquake. *Geotextiles and Geomembranes*, 29(3), 222-232.
- Watanabe, H. (1975). A numerical method of seismic analysis for rock and earthfill dams and verification of its reliability through both model test and observation of earthquake on an actual dam. *Proc. of Symp. on Criteria and assumptions for numerical analysis of dam*. Swansea.
- Wieland. M. (2008). Analysis aspects of dams subjected to strong ground shaking. *International water power & Dam construction*, pp:28-31
- Xie. G., Zhang. J., and Li. J.(2008). Adapted genetic algorithm applied to slope reliability analysis, in: *4th International Conference on Natural Computation*. vol. 1, pp. 520-524.

- Xia, Z. F., Ye, G. L., Wang, J. H., Ye, B., & Zhang, F. (2010). Fully coupled numerical analysis of repeated shake-consolidation process of earth embankment on liquefiable foundation. *Soil Dynamics and Earthquake Engineering*, 30(11), 1309-1318.
- Yang, C.X., Tham, L. G., Feng, X. T., Wang, Y. J., and Lee, P.K.K. (2004). Two-stepped Evolutionary algorithm and its application to stability analysis of slopes. *Journal of Computing in Civil Engineering*; 18 (2): 145–153.
- Yildiz, A. (2009). Numerical analyses of embankments on PVD improved soft clays. *Advances in Engineering Software*, 40(10), 1047-1055.
- Yu, Y., Xie, L., & Zhang, B. (2005). Stability of earth–rockfill dams: influence of geometry on the three-dimensional effect. *Computers and Geotechnics*, 32(5), 326-339.
- Zeghal, M. and Abdel-Ghaffar, A. (1992). Analysis of Behavior of Earth Dam Using Strong- Motion Earthquake Records. *J. Geotech. Engrg.*, 118(2), 266–277.
- Zeping, X. (2008). Performance of Zipingpu CFRD during the strong earthquake. *China Institute of Water Resources and Hydropower Research*.