STRESS-STRAIN BEHAVIOUR OF HIGH-STRENGTH CONCRETE WITH LATERAL PRE-TENSIONING CONFINEMENT

ABDULLAH ZAWAWI BIN AWANG

A thesis submitted in fulfilment of the requirements for the award of the degree of Doctor of Philosophy (Civil Engineering)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > MAY 2013

Date: 10 MAY 2013

Librarian Perpustakaan Sultanah Zanariah UTM, Skudai Johor

Sir,

CLASSIFICATION OF PROJECT REPORT AS RESTRICTED "STRESS-STRAIN BEHAVIOUR OF HIGH-STRENGTH CONCRETE WITH LATERAL PRE-TENSIONING CONFINEMENT" BY ABDULLAH ZAWAWI BIN AWANG

Please be informed that the above mentioned project report entitled "STRESS-STRAIN BEHAVIOUR OF HIGH-STRENGTH CONCRETE WITH LATERAL PRE-TENSIONING CONFINEMENT" be classified as RESTRICTED for a period of three (3) years from the date of this letter. The reasons for this classification is

 This report contains confidential material that is in the process of filing for pattern before considering open access.

Thank you.

Yours sincerely,

PROFESSOR Ir. Dr. WAHID BIN OMAR Faculti of Civil Engineering, Universiti Teknologi Malaysia Tel: 019-7533000

ACKNOWLEDGEMENTS

This study would not have been possible without help of many people whom I would like to thank from the bottom of my heart. First and foremost, I would like to express my deepest gratitude to my supervisor, Professor Ir Dr. Wahid bin Omar for his guidance, encouragement and support throughout the course of my study. I am particularly grateful to his patience and guidance that has walked me through many difficulties in this study.

I am also indebted to Universiti Teknologi Malaysia (UTM) for funding my Ph.D. study. Professor Dr. Kypros Pilakoatas of University of Sheffield (UK) also deserves special thanks for his co-operation during my study attachment at the university. His assistance and guidance is much indeed appreciated.

This study would be imperfect without the support and help from my friend and laboratory staff. Therefore, I would like to give my heartily and thousand thank to Mr. Lee Hoong Pin, Mr. Ma Chau Khun and laboratory technical staffs, Mr. Razale Mohammad, Mr. Zulkifly Abdul Wahid and Mr. Raja Ezar Ishamuddin Raja Abdul Latiff for their help and support along the period.

Besides, I would also like to extend my sincere appreciation to all my dearest family members especially to my beloved wife, Mrs. Hadizah Ab.Hamid for being caring and supportive at all time. Last but not least, I would like to thank all my friends for their support and encouragement.

ABSTRACT

High-strength concrete is currently being used in columns of multi-storey building all over the world. Although, it offers superior properties, it is relatively a brittle material. This study intends to improve the ductility of the concrete using a pre-tensioning lateral confinement technique. It investigates the effects of the technique on strength and ductility of the concrete and develops model equations to predict the stress-strain behaviour of confined concrete. The basis of the technique is to apply a pre-tensioning force to relatively low cost steel straps wrapped around the cylindrical concrete specimens. The experimental work was carried out using cylindrical specimens with a dimension of 100mm in diameter and 200mm in height and concrete compressive strength were fixed at 50, 60 and 80 MPa. The parameters studied including different properties of steel straps, various spacing between straps, number of layers of straps and different levels of pre-tensioning stress in the steel straps. The confined specimens were tested in compression until failure under monotonic and cyclic loading conditions. Data collected from the test included mode of failure, loads at peak and ultimate condition, and longitudinal and lateral strains in both concrete and straps. The data were analysed based on volumetric ratio of confinement which is a function of strength of steel straps, compressive strength of concrete, spacing and number of layers of straps. The experimental results show that the pre-tensioning technique using steel straps enhanced the ductility as well as strength of the concrete as the volumetric ratio increases. The results also depicted the ability of the technique to improve the ductility and strength of the concrete, especially for concrete with higher compressive strength by effectively utilising the confinement material. Moreover, the layers of straps had also delayed the onset of volumetric expansion and, undoubtedly, the concrete failure. Based on the analysis, new equations of strength and strain enhancement with confinement coefficients of 2.62 and 11.6 respectively, using pre-tensioning technique had been developed to predict the stress-strain behaviour of confined high-strength concrete.

ABSTRAK

Konkrit kekuatan tinggi banyak digunakan terutamanya dalam pembinaan tiang bangunan tinggi di serata dunia. Walaupun konkrit ini mempunyai sifat-sifat yang baik, tetapi ia secara relatif merupakan bahan yang rapuh. Projek penyelidikan ini bertujuan untuk meningkatkan kemuluran konkrit berkekuatan tinggi menggunakan teknik kurungan sisi pra-tegangan. Ia melibatkan kajian kesan penggunaan teknik ini keatas kekuatan dan kemuluran konkrit dan pembangunan persamaan matematik bagi menganggar hubungan tegasan-terikan konkrit kurungan sisi. Asas penting teknik ini adalah dengan mengenakan daya pra-tegangan pada jalur keluli yang dililit pada permukaan silinder konkrit. Ujian makmal telah dilakukan ke atas spesimen silinder konkrit berukuran 100 mm dia. x 200 mm tinggi, dan kekuatan mampatan konkrit ialah 50, 60 dan 80 MPa. Parameter kajian ini termasuk penggunaan jalur keluli yang berlainan sifat, perubahan jarak langkau antara jalur dan bilangan lapisan jalur dan juga tahap daya pra-tegangan yang dikenakan. Spesimen telah diuji di bawah beban mampatan statik dan juga beban kitaran sehingga gagal. Data yang diambil termasuklah mod kegagalan, beban pada ketika puncak dan muktamad, terikan tegak dan sisi bagi konkrit dan jalur keluli. Data telah dianalisis berdasarkan nisbah isipadu bahan kurungan, di mana ia mempunyai hubungkait dengan kekuatan keluli, kekuatan mampatan konkrit, jarak langkau dan bilangan lapisan jalur keluli. Keputusan ujian menunjukkan bahawa kemuluran dan juga kekuatan konkrit meningkat apabila nisbah isipadu bahan kurungan bertambah. Keputusan ujian juga menunjukkan teknik ini berupaya meningkatkan kemuluran dan kekuatan konkrit terutamanya bagi konkrit yang mempunyai kekuatan mampatan lebih tinggi melalui keberkesanan penggunaan bahan kurungan. Jalur keluli dalam berbilang lapisan juga didapati berupaya melewatkan pengembangan isipadu dan seterusnya kegagalan konkrit. Berpandukan kepada analisis, persamaan bagi kekuatan dan terikan konkrit masing-masing dengan pekali kurungan sisi 2.62 dan 11.6 telah dibangunkan untuk menganggar hubungan tegasan-terikan konkrit berkekuatan tinggi kurungan sisi menggunakan teknik pra-tegangan.

TABLE OF CONTENTS

CHAPTER			TITLE	PAGE
	DEC	CLARA'	ii	
	ACI	KNOWI	LEDGEMENTS	iii
	ABS	STRAC	ſ	iv
	ABS	STRAK		v
	TAE	BLE OF	CONTENTS	vi
	LIS	T OF TA	ABLES	xiv
	LIS	T OF FI	GURES	xvi
	LIS	T OF SY	YMBOLS	xxxii
	LIS	T OF A	PPENDICES	xxxiii
1	INT	RODU	CTION	1
	1.1	Backg	round	1
	1.2	Backg	round on Concrete Confinement	3
	1.3	Signifi	cance of Research	4
	1.4	Object	ives	5
	1.5	Scope	of Research	6
	1.6	Outlin	e of Thesis	7
2	LIT	ERATU	RE REVIEW	9
	2.1	Genera	al	9
	2.2	Backg	round of High-Strength Concrete	10
		2.2.1	Definition of High-Strength Concrete	12
		2.2.2	High-Strength and Normal Strength	
			Concretes Characteristics	12

	2.2.3	Factors t	hat Govern the Strength of High-	
		Strength	Concrete	13
		2.2.3.1	Paste Properties	13
		2.2.3.2	Transition Zone Properties	14
		2.2.3.3	Aggregate Properties	14
	2.2.4	Advanta	ges and Disadvantages of High-	
		Strength	Concrete	15
2.3	Mecha	nical Prop	erties of High-Strength Concrete	16
	2.3.1	Modulus	s of Elasticity	16
	2.3.2	Poisson	Ratio and Dilation Rate of Concrete	18
2.4	Ductili	ty		20
	2.4.1	Ductility	of High-Strength Concrete	21
	2.4.2	Ductility	of Concrete Constitutive Material	22
	2.4.3	Ductility	of Reinforcement	23
2.5	Confin	ement		24
	2.5.1	Mechani	cs of Confinement	24
	2.5.2	Lateral C	Confinement	26
	2.5.3	External	Confinement	28
	2.5.4	Confined	d Concrete as Seismic Retrofitting	
		Method		30
	2.5.5	Effect of	f Configuration on Effectively	
		Confined	d Area	30
2.6	Advant	tages in Us	sing Steel Strap	34
2.7	Existin	g Confine	d Concrete Model Equations	35
	2.7.1	Stress-St	train Model by Mander, et. al., 1988	35
		2.7.1.1	For Monotonic Compression	
			Loading	35
		2.7.1.2	For Cyclic Loading at Slow Strain	
			Rates	39
	2.7.2	Stress-st	rain Model by Yong, et. al., 1988	42
		2.7.2.1	Parameter Considered	42
		2.7.2.2	For Rectilinear Confined High-	
			Strength Concrete	44

	2.7.3	Stress-Strain Model by Spoelstra and	
		Monti., 1999	45
	2.7.4	Stress-Strain Model by Cusson and	
		Paultree., 1995	46
	2.7.5	Stress-Strain Model by Hoshikuma et al.,	
		1997	48
	2.7.6	Stress-Strain Model by Karbhari and Gao.,	
		1997	49
	2.7.7	Strees-Strain Model by Kono et al., 1998	50
	2.7.8	Stress-Strain Model by EC 8, 2001	51
2.8	Conclu	iding Remarks on High-Strength Concrete	
	Confin	ement	51
ME	THODO	DLOGY	53
3.1	Introdu	action	53
3.2	Materi	als for High-Strength Concrete	55
3.3	Compr	essive Strength Test	55
3.4	Phases	of Experimental Work	58
3.5	Proper	ties of Confinement Materials and Pre-	
	tension	ning Technique	59
	3.5.1	Introduction to Steel Straps	59
	3.5.2	Material Properties of Steel Straps	60
	3.5.3	New and Innovative Steel Strap Clip and	
		Connection	62
		3.5.3.1 Test Result for the Efficiency of	
		the Connections	63
	3.5.4	New and Innovative Pre-Formed Steel Strips	
		Rings	64
	3.5.5	Distribution of Confining Stress in Multi-	
		Layer Continous Strap in Loaded Confined	
		Concrete	65
	3.5.6	Steel Strap of Hose-Clip Type	67
	3.5.7	Tensioner and Strapping Machines	69

		3.5.8	Pre-Ter	sioning Stress in Confined	
			Specim	en	70
		3.5.9	Propert	ies of Steel Tube	71
	3.6	Test S	pecimen A	Arrangement	72
		3.6.1	Labelin	g of Specimens	73
		3.6.2	Volume	etric Ratio of Confinement	74
		3.6.3	Typical	Results Presentations	75
		3.6.4	Test Sp	ecimens	76
4	INS'	TRUMI	ENTATIO	ON AND TESTING DEVICES	78
	4.1	Introdu	uction		78
	4.2	Device	es DV1, D	V2 and DL	78
		4.2.1	Design	of Device DV1 Rig for Longitudinal	
			LVDTs		78
		4.2.2	Device	DV2 – Longitudinal LVDTs for	
			Overall	Height	80
		423	Design	of Device DL Rig for Lateral	
		1.2.5	LVDTs		80
		4.2.4	Electric	al Strain Gauge	82
	4.3	Testing	g Loading	, Setup and Considerations	82
	4.4	Test R	esults for	Verification of the Devices DV1 and	
		DL			85
	4.5	Conclu	uding Ren	narks on Devices DV1 and DL	91
5	EXF	PERIMI	ENTAL R	RESULTS OF UNCONFINED	
	ANI	O CONF	FINED CO	ONCRETE CYLINDERS USING	
	PRE	-TENS	IONING	TECHNIQUE	92
	5.1	Introdu	uction		92
	5.2	Phase	1 – Uncor	ifined Concrete	93
		5.2.1	Results f	or Specimens C80	94
			5.2.1.1	Observed Behaviour of Failure	
				Mode	94
			5.2.1.2	Stress-strain Behaviour of	

C80-Unc-02 95 5.2.1.3 Volumetric Strain for C80-Unc-02 98 5.2.2 Results for Specimen C50 100 5.2.2.1 **Observed Behaviour of Failure** 100 Mode 5.2.2.2 Stress-strain Behaviour of C50-Unc-03 102 5.2.2.3 Volumetric Strain for C50-Unc-03 104 5.2.3 Summary of the Results for Phase 1 107 Phase 2a – Confined Concrete in a Single Layer 5.3 **Straps** 111 Results for Specimens in C80 series 5.3.1 111 5.3.1.1 Observed Behaviour of Failure Mode 111 5.3.1.2 Stress-strain Response of C80 A0-1FT-01, C80 A10-1FT-01, and C80A15-1FT-02 114 5.3.1.3 Volumetric Strain of C80 A0-1FT-01, C80 A10-1FT-02, and C80A15-1FT-02 116 5.3.2 Results for Specimens in C50 series 121 5.3.2.1 **Observed Behaviour of Failure** Mode 121 5.3.2.2 Stress-strain Response of C50 A0-1FT-01, C50 A10-1FT-01, and C50A10-1FT-02 122 5.3.2.3 Volumetric Strain of C50 A0-1FT-01 and C50 A10-1FT-01 125 5.4 Phase 2b – Confined Concrete with Steel Straps Type Z 129 5.4.1 Results for Specimens in C80Z10 Series 129 Observed Behaviour of Mode 5.4.1.1

		Failure	129
	5.4.1.2	Stress-Strain of	
		C80Z10-1NT-01, C80Z10-1HT-02	
		and C80Z10-1HT-03	132
	5.4.1.3	Stress-strain Behaviour of C80Z10-	
		1FT-01, C80Z10-1FT-03 and	
		C80Z10-1FT-01cy	134
	5.4.1.4	Volumetric Strain for	
		C80Z10-1NT-01, C80Z10-1HT-02,	
		C80Z10-1FT-01 and C80Z10-1FT-	
		01cy	137
	5.4.2 Results	for Specimens in C50Z10 Series	142
	5.4.2.1	Observed Behaviour of Failure	
		Mode	142
	5.4.2.2	Stress-Strain Behaviour of	
		C50Z10-1NT-01, C50Z10-1HT-02	
		and C50Z10-1HT-03	145
	5.4.2.3	Stress-Strain Behaviour of	
		C50Z10-1FT-01, C50Z10-1FT-02	
		and C50Z10-1FT-02cy	147
	5.4.2.4	Volumetric strain of	
		C50Z10-1NT-01, C50Z10-1HT-	
		02/03, C50Z10-1FT-01/02 and	
		C50Z10-1FT-02cy	150
5.5	Concluding Re	marks	155
EXF	PERIMENTAL	RESULTS OF CONFINED	
CO	NCRETE CYLY	NDERS USING PRE-	
TEN	SIONED AND	MULTILAYER CONTINUOUS	
STR	AP		156
6.1	Introduction		156
6.2	Phase 3 – Cont	fined Concrete with Steel Straps Type	
	А		157

6

6.2.1	Results f	for Specimens in C80 series	157
	6.2.1.1	Observed Behaviour of Failure	
		Mode of Two- and Three-Layer	
		Confined Specimens	157
	6.2.1.2	Observed Behaviour of Failure	
		Modes of Four- and Five-Layer	
		Confined Specimens	159
	6.2.1.3	Stress-strain Behaviour of C80A10-	
		2FT-01/02 and C80A10-3FT-01/02	162
	6.2.1.4	Volumetric Strain of C80A10-2FT-	
		01 and C80A10-3FT-02	165
	6.2.1.5	Stress-strain behaviour of C80A10-	
		4FT-01/02/03, C80A10-5FT-	
		02/03/04, C80A10-5HT-03	167
	6.2.1.6	Volumetric Strain of C80A10-4FT-	
		03 and C80A10-5FT-02/03/04	171
6.2.2	Results f	for Specimens in C50 series	176
	6.2.2.1	Observed Behaviour of Failure	
		Mode of Two- and Three-Layer	
		Confined Specimens	176
	6.2.2.2	Observed Behaviour of Failure	
		Mode for Four- and Five-Layer	
		Confined Specimens	178
	6.2.2.3	Stress-Strain Behaviour of	
		C50A10-2FT-02/03 and C50A10-	
		3FT-02/03	181
	6.2.2.4	Volumetric Strain of C50A10-2FT-	
		02/03 and C50A10-3FT-02/03	184
	6.2.2.5	Stress-Strain Behaviour of	
		C50A10-4FT-01, C50A10-5FT-	
		01/03 and C50A10-5HT-03	187
	6.2.2.6	Volumetric Strain of C50A10-4FT-	
		01, C50A10-5FT-01/03 and	

xii

		C50A10-5HT-03	189
	6.3	Phase 4 - Confined Concrete with Steel Straps Type	e
		S Under Cyclic Loads	195
		6.3.1 Results for Specimens Subjected to Cyclic	
		Load	195
		6.3.2 Results for Specimens Confined in Steel	
		Tubes and with Steel Straps in 3 Individual	
		Layers	204
	6.4	Concluding Remarks	207
7	DISC	CUSSION OF THE RESULTS OF CONFINED	
	CON	ICRETE DUE TO PRE-TENSIONING AND	
	MUI	LTILAYER EFFECTS	208
	7.1	Introduction	208
	7.2	Gains in Strength, Strain and Ductility	208
	7.3	Comparison of Normalised Axial Stress and Strain	
		with Existing Models	218
	7.4	Comparison of Results from New Equations and	
		Experimental	222
	7.5	Comparison of New Model of Stress-strain with	
		Existing Models	225
	7.6	Schematic Stress-strain Behaviour	229
	7.7	Concluding Remarks	235
8	CON	ICLUSIONS AND RECOMMENDATIONS	237
	8.1	Conclusions	237
	8.2	Recommendations for Future Work	240
REFERENC	ES		242
APPENDICE	ESA-	Е	250 - 266

xiii

LIST OF TABLES

TABLE NO.	TITLE	PAGE
2.1	Modulus of elasticity equations	17
3.1	Properties of hardened concrete	57
3.2	Properties of steel straps	61
3.3	Experimental results of the efficiency of clips	64
3.4	Stress and strain at different levels of pre-tensioning for different type of straps	71
3.5	Mechanical properties of steel tube	72
3.6	Volumetric ratio of confinement	75
5.1	Strains (microstrain) at peak stress of unconfined specimens C80 and C50	108
5.2	Test results for unconfined specimens C80 subjected to monotonic and cyclic loadings	109
5.3	Test results for unconfined specimens C50 subjected to monotonic and cyclic loadings	110
5.4	Strains (microstrain) at peak stress of confined specimens of C80A	119
5.5	Test results for single layer confined specimens C80 subjected to monotonic loading	120
5.6	Strains (microstrain) at peak stress of confined specimens of C50A	127
5.7	Test results for single layer confined specimens C50 subjected to monotonic loading	128
5.8	Strains (microstrain) at peak stress of confined specimens of C80Z	140

5.9	Test results for confined concrete with steel straps type C80Z10	141
5.10	Test results for confined concrete with steel straps type C50Z10	154
6.1	Test results for C80A10-2/3FT specimens	174
6.2	Test results for C80A10-4/5FT specimens	175
6.3	Strains (microstrain) corresponding to peak stress for C50A10 specimens	193
6.4	Test results for C50A10-2/3/4/5FT specimens	194
6.5	Test results for C60S12-2FT specimens subjected to axial monotonic and cyclic loads	202
6.6	Test results for C60S12-4FT specimens subjected to axial monotonic and cyclic loads	203
7.1	Strength and strain values for graphs in Figures 5.34 and 5.35	235

LIST OF FIGURES

FIGURE NO.	TITLE	PAGE
1.1	Concrete and steel stress strain relationship (ACI 363R)	2
2.1	Schematic structures of the cement paste in: a) a normal strength concrete; and b) a high-strength concrete	13
2.2	Modulus of elasticity vs compressive strength (ACI Committee 318)	17
2.3	Modulus of elasticity vs compressive strength (Tomosawa and Noguchi, 1993)	18
2.4	Modulus of elasticity vs compressive strength (CEB-FIP, 1990)	18
2.5	Poisson ratio at different stages of load in concrete (Choi and Shah, 1998)	20
2.6	Increased strain, stress and ductility by confinement (Ali et al. 2003)	25
2.7	Effect of confinement on concrete core for axisymmetric specimens	26
2.8	Stress-strain curves for confined and unconfined concrete (Saadatmanesh, et. al., 1994)	29
2.9	Arching action in confined concrete (Cusson et al., 1994)	31
2.10	Effectively confined area exerted by circular hoops	31
2.11	Comparison of confined area between spiral & circular hoops	33
2.12	Various zones view of $[\sigma-\epsilon]$ relations in circular (a) & spiral (b) lateral confined concrete (Karabinis et al., 1994)	34

2.13	Stress-strain model proposed for monotonic loading of confined and unconfined concrete (Mander et al, 1988b)	36
2.14	Stress-strain curve for unloading/reloading branches in the model of Mander, et al. (1988)	42
2.15	Stress-strain curve and parameter in Yong, et al. (1988) empirical model	43
3.1	Specimens are curing in a water tank for 28 days	56
3.2	Flow-chart for experimental work	58
3.3	(a) Dartex universal testing machine of 250 kN capacity and (b) strap specimen	60
3.4	Clip connections (a) original clip for box packaging (b) new and innovative clip (c) elevation view of the connection and (d) side view of the connection	63
3.5	Schematic diagram of the clip connection	63
3.6	New and innovative pre-fabricated strap rings (a) ready use strap ring (b) plan view of the ring	65
3.7	Schematic diagram of pre-fabricated strap ring	65
3.8	Distribution of confining stress in multi-layer ring sealed using conventional clip and connection	66
3.9	Distribution of confining stress in multi-layer ring sealed using new and innovative clip and connection	67
3.10	Steel strap in variety of applications	68
3.11	Plan view of steel straps of hose clip type (Type Z)	68
3.12	Schematic diagram of hose clip type of strap	69
3.13	Strapping tools (a) manually operated tensioning machine and (b) pneumatic strapping and sealing machines	70
3.14	(a) Pre-fabricated strapping rings fitted with strain gauge (b) process of strapping cylindrical specimen using manually operated strapping machine	70
3.15	(a) Stel tube (carbon steel seamless pipe) (b) the	

	grouting work in specimen with steel tube confinement	72
3.16	Typical stress-strain curves for longitudinal and lateral behaviour of the specimens in this project	76
3.17	View of tested specimens	77
4.1	New rig of device DV1 with vertical LVDTs for longitudinal strain measurement (a) elevation view (b) plan view (c) schematic plan view of top ring and (d) schematic plan view of bottom ring	79
4.2	New rig of DL device for lateral LVDTs arrangement (a) and (b) device placed onto specimen (c) and (d) plan view of horizontal LVDTs arrangement details	81
4.3	Schematic plan view of device DL for horizontal LVDT arrangement	81
4.4	Test setup (a) concrete cylinder instrumented with LVDTs – measurement rig (b, c) specimen in testing machine before loading and (d) data captured and displayed on the computer screen during testing	83
4.5	LVDTs of DV1, DV2 and DL arrangement for unconfined specimen (a) elevation view and (b) plan view	84
4.6	LVDTs of DV1, DV2 and DL arrangement for confined specimen (a) elevation view and (b) plan view	85
4.7	Stress-strain curves for concrete 20 MPa (a) Axial stress-strain curve derived from device DV1 and strain gauge (b) lateral dilation of concrete derived from device DL and strain gauge	87
4.8	Stress-strain curves for concrete 30 MPa (a) Axial stress-strain curve derived from device DV1 and strain gauge (b) lateral dilation of concrete derived from device DL and strain gauge	88
4.9	Stress-strain curves for concrete 40 MPa (a) Axial stress-strain curve derived from device DV1 and strain gauge (b) lateral dilation of concrete derived from device DL and strain gauge	89

4.10	Schematic stress-strain model from EC8, 2001	90
4.11	Comparison of peak strain derived from experimental result with EC8 Equation	90
4.12	Comparison of Elastic Modulus derived from experimental result with ACI 318 and CEB-FIB Equations	91
5.1	Failure modes of specimens (a) C80-Unc-06 (b) C80-Unc-05 (c) C80-Unc-04 (d) C80-Unc-02 and (e) close-up view of C80-Unc-02	95
5.2	Stress-strain curves in C80-Unc-02	97
5.3	Average stress-strain curves in C80-Unc-02	97
5.4	Normalised axial stress vs volumetric strain of the concrete core – C80-Unc-02	99
5.5	Normalised strain vs volumetric strain of the concrete core of C80-Unc-02	99
5.6	The mode of failure in static loading for plain specimens (a) and (b) C50-Unc-03 (c) and (d) C50-Unc-05 and (e) and (f) C50-Unc-06	101
5.7	Stress-strain curves in C50-Unc-03	103
5.8	Average stress-strain curves in C50-Unc-03	104
5.9	Average stress-strain curves in C50-Unc-01cy tested in cyclic loading	104
5.10	Normalised axial stress vs volumetric strain of the concrete core - C50-Unc-03	105
5.11	Normalised strain vs volumetric strain of concrete in C50-Unc-03	105
5.12	Normalised axial stress vs volumetric strain of concrete core – C50-Unc-01cy	106
5.13	Normalised strain vs volumetric strain of concrete in C50-Unc-01cy	106
5.14	Normalised strain vs volumetric strain of concrete in C50-Unc-02cy	106

5.15	Normalised axial stress vs volumetric strain of concrete core – C50-Unc-02cy	107
5.16	Mode of failure for specimens C80 wrapped in a single layer of straps type A (a) and (b) specimen C80A10-1FT-02 (c) specimen C80A10-1FT-03 and (d) specimen C80A15-1FT-02 under monotonic axial compression load	113
5.17	Stress-strain curves in C80A0-1FT-01	115
5.18	Average stress-strain curves in C80A0-1FT-01	115
5.19	Stress-strain curves in C80A10-1FT-02	115
5.20	Average stress-strain curves in C80A10-1FT-02	116
5.21	Stress-strain curves in C80A15-1FT-02	116
5.22	Normalised axial strain vs volumetric strain for C80A0-1FT-01	117
5.23	Normalised axial strain vs volumetric strain for C80A0-1FT-01	117
5.24	Normalised axial strain vs volumetric strain for C80A10-1FT-02	118
5.25	Normalised axial strain vs volumetric strain for C80A10-1FT-02	118
5.26	Normalised axial strain vs volumetric strain for C80A15-1FT-02	118
5.27	Mode of failure for specimens C50 wrapped in a single layer of straps type A and C (a) specimen C50A0-1FT-01 (b) specimen C50C10-1FT-02 and	
	(c) specimen C50A10-1FT-02 tested under axial compressive load.	122
5.28	Stress-strain curves in C50A0-1FT-01	123
5.29	Average stress-strain curves in C50A0-1FT-01	123
5.30	Average stress-strain curves in C50A10-1FT-01	124
5.31	Stress-strain curves in C50A10-1FT-02	124
5.32	Average stress-strain curves in C50A10-1FT-02	124

5.33	Normalised axial stress vs volumetric strain for C50A0-1FT-01	125
5.34	Normalised axial strain vs volumetric strain for C50A0-1FT-01	126
5.35	Normalised axial strain vs volumetric strain for C50A10-1FT-01	126
5.36	Normalised axial strain vs volumetric strain for C50A10-1FT-01	126
5.37	Normalised axial strain vs volumetric strain for C50A10-1FT-02	127
5.38	Mode of failure steel straps type Z confined specimens (a) C80Z10-1NT-01 and (b) C80Z10- 1NT-03 with no pre-tensioning force	131
5.39	Mode of failure steel straps type Z confined specimens (a) and (b) C80Z10-1HT-02 with half pre-tensioning force	131
5.40	Mode of failure steel straps type Z confined specimens (a) and (b) C80Z10-1FT-01 with full pre-tensioning force	132
5.41	Stress-strain curves in C80Z10-1NT-01	133
5.42	Average stress-strain curves in C80Z10-1NT-01	133
5.43	Stress-strain curves in C80Z10-1HT-02	133
5.44	Average stress-strain curves in C80Z10-1HT-02	134
5.45	Average stress-strain curves in C80Z10-1HT-03	134
5.46	Stress-strain curves in C80Z10-1FT-01	135
5.47	Average stress-strain curves in C80Z10-1FT-01	136
5.48	Average stress-strain curves in C80Z10-1FT-03	136
5.49	Average stress-strain curves in C80Z10-1FT-01cy	136
5.50	Normalised axial stress vs volumetric strain for C80Z10-1NT-01	137
5.51	Normalised strain vs volumetric strain for C80Z10-1NT-01	137

5.52	Normalised axial stress vs volumetric strain for C80Z10-1HT-02	138
5.53	Normalised strain vs volumetric strain for C80Z10-1HT-02	138
5.54	Normalised axial stress vs volumetric strain for C80Z10-1FT-01	138
5.55	Normalised strain vs volumetric strain for C80Z10-1FT-01	139
5.56	Normalised axial stress vs volumetric strain for C80Z10-1FT-01cy	139
5.57	Normalised strain vs volumetric strain for C80Z10-1FT-01cy	139
5.58	Failure conditions of specimens (a),(b) and (c) confined with no pre-tensioning (NT) and (d), (e) (f), (g) and (h) specimens with half pre-tensioning (HT) stresses in static loading	143
5.59	Failure conditions of confined (a) and (b) specimens C50Z10-1FT-01 and (c) and (d) specimens C50Z10-1FT-02 with full pre-tensioning force	144
5.60	Stress-strain curves in C50Z10-1NT-01	145
5.61	Average stress-strain curves in C50Z10-1NT-01	145
5.62	Stress-strain curves in C50Z10-1HT-02	146
5.63	Average stress-strain curves in C50Z10-1HT-02	146
5.64	Stress-strain curves in C50Z10-1HT-03	147
5.65	Average stress-strain curves in C50Z10-1HT-03	147
5.66	Stress-strain curves in C50Z10-1FT-01	148
5.67	Average stress-strain curves in C50Z10-1FT-01	148
5.68	Stress-strain curves in C50Z10-1FT-02	149
5.69	Average stress-strain curves in C50Z10-1FT-02	149
5.70	Average stress-strain curves in C50Z10-1FT-02cy	149

	٠	٠	٠
vv	L	L	н
ᆻ	L	I	L

5.71	Normalised axial stress vs volumetric strain for C50Z10-1NT-01	150
5.72	Normalised strain vs volumetric strain for C50Z10- 1NT-01	151
5.73	Normalised axial stress vs volumetric strain for C50Z10-1HT-02 and C50Z10-1HT-03	151
5.74	Normalised strain vs volumetric strain for C50Z10- 1HT-02	151
5.75	Normalised strain vs volumetric strain for C50Z10- 1HT-03	152
5.76	Normalised axial stress vs volumetric strain for C50Z10-1FT-01 and C50Z10-1FT-02	152
5.77	Normalised strain vs volumetric strain for C50Z10-1FT-01	152
5.78	Normalised strain vs volumetric strain for C50Z10-1FT-02	153
5.79	Normalised axial stress vs volumetric strain for C50Z10-1FT-02cy	153
5.80	Normalised strain vs volumetric strain for C50Z10-1FT-02cy	153
6.1	Pre-tensioned and two-layer confined specimens (a) and (b) C80A10-2FT-01 (c) specimen C80A10- 2FT-02 and (d) monitor display the load and strain measurements at failure	158
6.2	Failure modes of 3-layer pre-tensioned confined specimens (a) C80A10-3FT-01 (b) and (c) C80A10-3FT-02 (d) C80A10-3FT-05 and (e) C80A10-3FT-03 at failure load	159
6.3	Specimens (a) and (b) C80A10-4FT-01 (c) C80A10-4FT-02 and (d) C80A10-4FT-03 wrapped-in 4-layer of straps type A failed in monotonic axial compressive load	160
6.4	(a) and (b) Specimen C80A10-5FT-02 and (c) and (d) specimen C80A10-5FT-03 confined in five- layer straps and the strap were tensioned to full initial stress at failure load	161

6.5	Specimens (a) C80A10-5FT-04 (b) C80A10-5FT- 06 with five –layer confinement of straps type A and full pre-tensioning stress at failure load	162
6.6	(a) and (b) Specimen C80A10-5HT-03 with five- layer confinement and pre-tensioned to half-full stress at failure load	162
6.7	Stress-strain curves in C80A10-2FT-01	163
6.8	Average stress-strain curves in C80A10-2FT-01	163
6.9	Average stress-strain curves in C80A10-2FT-02	164
6.10	Stress-strain curves in C80A10-3FT-02	164
6.11	Average stress-strain curves in C80A10-3FT-02	164
6.12	Average stress-strain curves in C80A10-3FT-01	165
6.13	Normalised axial stress vs volumetric strain for C80A10-2FT-01	166
6.14	Normalised strain vs volumetric strain for C80A10- 2FT-01	166
6.15	Normalised axial stress vs volumetric strain for C80A10-3FT-02	166
6.16	Normalised strain vs volumetric strain for C80A10- 3FT-02	167
6.17	Stress-strain curves in C80A10-4FT-01	168
6.18	Average stress-strain curves in C80A10-4FT-01	168
6.19	Average stress-strain curves in C80A10-4FT-02	168
6.20	Stress-strain curves in C80A10-4FT-03	169
6.21	Average stress-strain curves in C80A10-4FT-03	169
6.22	Stress-strain curves in C80A10-5FT-02	169
6.23	Average stress-strain curves in C80A10-5FT-02	170
6.24	Stress-strain curves in C80A10-5FT-03	170
6.25	Average stress-strain curves in C80A10-5FT-03	170

6.26	Average stress-strain curves in C80A10-5FT-04	171
6.27	Average stress-strain curves in C80A10-5HT-03	171
6.28	Normalised axial stress vs volumetric strain for C80A10-4FT-03	172
6.29	Normalised strain vs volumetric strain for C80A10- 4FT-03	172
6.30	Normalised axial strain vs volumetric strain for C80A10-5FT-02	172
6.31	Normalised strain vs volumetric strain for C80A10- 5FT-02	173
6.32	Normalised strain vs volumetric strain for C80A10- 5FT-03	173
6.33	Specimens (a) C50A10-2FT-01 (b) C50A10-2FT- 03 (c) C50A10-2FT-02 wrapped-in with two layers of straps type A and pre-tensioned to the full stress and (d) load and strain readings at failure load.	177
6.34	Specimen confined with three layers of straps A (a) C50A10-3FT-01 (b) C50A10-3FT-02 and (c) and (d) C50A10-3FT-03 at failure load.	178
6.35	(a) Specimen C50A10-4FT-01 with four layers of straps and pre-tensioned to full initial stress at failure load and (b) straps placed at middle were broke at failure load.	179
6.36	(a) Specimens C50A10-5FT-01 confined with fiver layers of straps and pre-tensioned to full of initial stress at failure load (b) strains measurements for specimens C50A10-5FT-01 at failure load (c) specimen C50A10-5FT-03 and (d) strain readings at failure condition.	180
6.37	(a) and (b) Specimen C50A10-5HT-03 confined with five layers of straps and pre-tensioned to half of initial stress at failure load.	181
6.38	Stress-strain curves in C50A10-2FT-02	182
6.39	Average stress-strain curves in C50A10-2FT-02	182
6.40	Average stress-strain curves in C50A10-2FT-03	182

6.41	Stress-strain curves in C50A10-3FT-02	183
6.42	Average stress-strain curves in C50A10-3FT-02	183
6.43	Stress-strain curves in C50A10-3FT-03	183
6.44	Average stress-strain curves in C50A10-3FT-03	184
6.45	Normalised axial stress vs volumetric strain for C50A10-2FT-02	185
6.46	Normalised strain vs volumetric strain for C50A10- 2FT-02	185
6.47	Normalised axial stress vs volumetric strain for C50A10-2FT-03	185
6.48	Normalised strain vs volumetric strain for C50A10- 2FT-03	186
6.49	Normalised axial stress vs volumetric strain for C50A10-3FT-02	186
6.50	Normalised strain vs volumetric strain for C50A10- 3FT-02	186
6.51	Normalised axial stress vs volumetric strain for C50A10-3FT-03	187
6.52	Normalised strain vs volumetric strain for C50A10- 3FT-03	187
6.53	Average stress-strain curves in C50A10-4FT-01	188
6.54	Average stress-strain curves in C50A10-5FT-01	188
6.55	Average stress-strain curves in C50A10-5FT-03	189
6.56	Average stress-strain curves in C50A10-5HT-03	189
6.57	Normalised axial stress vs volumetric strain for C50A10-4FT-01	190
6.58	Normalised axial strain vs volumetric strain for C50A10-4FT-01	190
6.59	Normalised axial stress vs volumetric strain for C50A10-5FT-01	191
6.60	Normalised axial strain vs volumetric strain for	

xxvii

	C50A10-5FT-01	191
6.61	Normalised axial stress vs volumetric strain for C50A10-5FT-03	191
6.62	Normalised axial strain vs volumetric strain for C50A10-5FT-03	192
6.63	Normalised axial stress vs volumetric strain for C50A10-5HT-03	192
6.64	Normalised axial strain vs volumetric strain for C50A10-5HT-03	192
6.65	Stress-strain curves for C60S12-2FT-02cy(1) subjected to cyclic load in single cycle	196
6.66	Average stress-strain curves for C60S12-2FT- 02cy(1) subjected to cyclic load in single cycle	196
6.67	Normalised axial strain vs volumetric strain for C60S12-2FT-02cy(1)	196
6.68	Normalised axial strain vs volumetric strain for C60S12-2FT-02cy(1)	197
6.69	Stress-strain curves for C60S12-2FT-03cy(3) subjected to cyclic load in three cycle	197
6.70	Average stress-strain curves for C60S12-2FT- 03cy(3) subjected to cyclic load in three cycle	197
6.71	Normalised axial stress vs volumetric strain for C60S12-2FT-03cy(3)	198
6.72	Normalised axial strain vs volumetric strain for C60S12-2FT-03cy(3)	198
6.73	Stress-strain curves for C60S12-4FT-03cy(1) subjected to cyclic load in single cycle	198
6.74	Average stress-strain curves for C60S12-4FT- 03cy(1) subjected to cyclic load in single cycle	199
6.75	Normalised axial stress vs volumetric strain for C60S12-4FT-03cy(1)	199
6.76	Normalised axial strain vs volumetric strain for C60S12-4FT-03cy(1)	199

xxviii

6.77	Stress-strain curves for C60S12-4FT-03cy(3) subjected to cyclic load on three cycle	200
6.78	Average stress-strain curves for C60S12-4FT- 03cy(3) subjected to cyclic load in three cycles	200
6.79	Normalised axial stress vs volumetric strain for C60S12-4FT-03cy(3)	200
6.80	Normalised axial strain vs volumetric strain for C60S12-4FT-03cy(3)	201
6.81	Failure mode of local buckling and crushing of steel tube confined specimens (a) steel tube of 1.5 mm and (b) 3.0 mm thickness.	204
6.82	Average stress-strain behaviours for steel tube and steel straps confined specimens	205
6.83	Volumetric strain behaviour specimen C60TU- A1.5-01 specimen	206
6.84	Volumetric strain behaviour for C60S12-3FT(G)- 03 specimen	206
7.1	Strength ratio (f_{ccl}/f_{co}) versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ for type-Z single layer pretensioning for specimen 50 MPa and 80 MPa	210
7.2	Strength ratio (f_{ccl}/f_{co}) versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ for multilayer and pretensioned type-A straps	211
7.3	Dilation ratio ($\varepsilon_{ccr}/\varepsilon_{ccl}$) versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ for concrete strength 50 MPa	211
7.4	Dilation ratio $(\varepsilon_{ccr}/\varepsilon_{ccl})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ for concrete strength 80 MPa	212
7.5	Ductility ratio $(\varepsilon_{c85cl}/\varepsilon_{co})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ for pretensioned type-Z strap	212
7.6	Ductility ratio $(\varepsilon_{c85cl}/\varepsilon_{co})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ for multilayer and pretensioned type-A	
	suap	213

7.7	Lateral ductility ratio $(\varepsilon_{ccr}/\varepsilon_{cor})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$	213
7.8	Strain gain $(\varepsilon_{ccl} - \varepsilon_{co})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$	214
7.9	Ductility gain $(\varepsilon_{c50cl} - \varepsilon_{c50U})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$	214
7.10	Critical stress at expansion (f_{ccl}/f_{co}) versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ - pre-tensioned and multilayer specimens	215
7.11	Critical volume at expansion versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ - pre-tensioned and multilayer specimens for concrete strength 50 MPa	213
7.12	Critical volume at expansion versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ - pre-tensioned and multilayer specimens for concrete strength 80MPa	213
7.13	Normalised axial strain $(\varepsilon_{ccl}/\varepsilon_{co})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ - pre-tensioned and multilayer specimens	217
7.14	$(\varepsilon_{c50cl}/\varepsilon_{c50U})$ versus confinement ratio $\left(\rho_s \frac{f_y}{f_{co}}\right)$ - pre-tensioned and multilayer specimens	217
7.15	Normalised axial stress versus confinement ratio for C50 concrete - type Z straps of various pretensioning levels and type A straps of varying spacing	218
7.16	Normalised axial stress versus confinement ratio for C80 concrete – type Z straps of various pretensioning levels and type A straps of varying spacing	219
7.17	Normalised axial stress versus confinement ratio for C50 concrete – multilayer straps of type A	219
7.18	Normalised axial stress versus confinement ratio for C80 concrete – multilayer straps of type A	220

7.19	Normalised axial stress versus of confinement ratio for C60 concrete – type S straps subjected to cyclic load	220
7.20	Normalised axial strain versus of confinement ratio for C50 concrete – multilayer straps of type A	221
7.21	Normalised axial strain versus of confinement ratio for C80 concrete – multilayer straps of type A	222
7.22	Normalised axial stress (new equation) versus normalized axial stress (experiment)	223
7.23	Normalised axial strain (new equation) versus normalized axial strain (experiment)	223
7.24	Ductility ratio (new equation) versus ductility ratio (experiment)	224
7.25	Strain at 50% descending branch ($\varepsilon_{c50cl}/\varepsilon_{c50U}$) (new equation) versus ($\varepsilon_{c50cl}/\varepsilon_{c50U}$) (experiment)	224
7.26	Stress-strain prediction model proposed by Mander et al (1988) for concrete strength 50 MPa.	226
7.27	Stress-strain prediction model proposed by Mander et al (1988) for concrete strength 80 MPa.	226
7.28	Stress-strain prediction model proposed by EC8 (2001) for concrete strength 50 MPa.	227
7.29	Stress-strain prediction model proposed by EC8 (2001) for concrete strength 80 MPa.	227
7.30	Stress-strain prediction model proposed by Karbhari and Gao (1997) for concrete strength 50 MPa.	228
7.31	Stress-strain prediction model proposed by Karbhari and Gao (1997) for concrete strength 80 MPa.	228
7.32	Schematic stress-strain behaviour of concrete confined with steel strap of varyfying pre- tensioning stress levels, $f_{co} = 50$ MPa and volumetric ratio of confinement of 0.1645 (a) unconfined concrete (b) confined with no pre- tensioning force (c) confined with half pre- tensioning force and (d) with full pre-tensioning force	231

7.33	Schematic stress-strain behaviour of concrete confined with steel strap of varyfying pre- tensioning stress levels, $f_{co} = 80$ MPa and volumetric ratio of confinement of 0.1028 (a) unconfined concrete (b) confined with no pre- tensioning force (c) confined with half pre- tensioning force and (d) with full pre-tensioning	
	force	232
7.34	Schematic stress-strain curves of multi-layer continuous strap concrete specimens with $f_{co} = 50$ MPa, full pre-tensioning stress and of varying volumetric ratio of confinement.	233
7.35	Schematic stress-strain curves of multi-layer continuous strap concrete specimens with $f_{co} = 80$ MPa, full pre-tensioning stress and of varying volumetric ratio of confinement.	234

LIST OF SYMBOLS

f_{cc}	-	Axial stress for confined concrete
f_{co}	-	Axial stress for unconfined concrete
$f_{ m r}$	-	Lateral confining stress
f_{ccl}	-	Maximum strength of confined concrete
f_{co}	-	Maximum strength of unconfined concrete
\mathcal{E}_{ccl}	-	Maximum axial strain of confined concrete
\mathcal{E}_{co}	-	Maximum axial strain of unconfined concrete
\mathcal{E}_{ccr}	-	Maximum lateral strain of confined concrete
\mathcal{E}_{cor}	-	Maximum lateral strain of unconfined concrete
\mathcal{E}_{c50cl}	-	Axial strain corresponding to 50% of peak stress at post peak region
E _{c85cl}	-	Axial strain corresponding to 85% of peak stress at post peak region
\mathcal{E}_{v}	-	Volumetric strain
\mathcal{E}_l	-	Axial strain
<i>E</i> _r	-	Lateral strain

LIST OF APPENDICES

APPENDIX	TITLE	PAGE
A	Stress-strain graph as a guidance for calculating strength, strain and volumetric values as tabulated in tables in Chapters 4 and 5 for confined specimens	250
В	Experimental results for specimens in C80B10 series (Steel Straps Type B)	252
C1	Tabulated strain (microstrain) values corresponding to peak stress for C80B10 specimens.	259
C2	Summary of test results for C80B10 specimens	260
D	Experimental results for specimens in C50B10 series (Steel Straps Type B)	261
E1	Tabulated strain (microstrain) values corresponding to peak stress for C50B10 specimens	265
E2	Summary of test results for C50B10 specimens	266

CHAPTER 1

INTRODUCTION

1.1 Background

High-strength concrete is an advanced construction material which offers superior properties such as higher strength, higher stiffness and better durability performance as compared to normal strength concrete. It can be produced using similar ordinary raw materials that are used in the production of normal strength concrete with a low water-cement ratio and special admixtures added. Nowadays, high-strength concrete is frequently used in columns of multi-storey building, in precast concrete industries and in structures where strength and durability are emphasised in design consideration. It can reduce the cross-section area of the column and subsequently reduce the self-weight of the structure.

Generally, high-strength concrete can be produced by improving the density of the concrete mix so that it increases the strength of both the cement matrix and the interface between the matrix and the aggregates. However, an increase in the strength of the concrete results in increase in its brittleness or reduction in ductility. Concrete in brittle failure is not capable to resist any increase in load after reaching ultimate state. Then the load decreases very rapidly after peak and the type of failure is explosive. Whereas in ductile failure the load is remain constant at increasing deformation after peak stress. Therefore, in designing high strength reinforced concrete element, more precaution is required in term of ductility requirement especially for structural elements that are possibly exposed to lateral type of loading such as seismic, blast and wind loadings etc. The lack of ductility of high-strength concrete can be seen as a steep ascending slope of a stress-strain curve followed by a very rapid post-peak descending branch of the curve as shown in Figure 1.1. In structural design point of view, this type of stress strain behaviour is not allowed and therefore the need to improve the ductility of the concrete is very crucial. One way of improving the ductility of concrete is by confining the concrete laterally. This method has been used for normal strength concrete since decades and it has been established that concrete confined laterally can increase the ductility and strength very significantly. The lateral confinement works if the concrete under axial compression produce sufficient lateral dilation. However, it is a fact that high-strength concrete exhibits smaller lateral expansion compared to normal strength concrete when subjected to axial or cyclic compression loads (Parenchio et al., 1978, Persson et al., 1973, Lee et al., 1988). Thus, this classical technique may be questioned and not be as effective when it is applied to high-strength concrete.



Figure 1.1 Concrete and steel stress strain relationship (ACI 363R)

This research was to investigate the effectiveness of lateral confinement using low cost steel straps with an innovative pre-tensioning and multilayer techniques to high strength concrete columns. The pre-tensioning stress applied to high-strength concrete column during installation may assist the column to mobilise the confinement material effectively and thus, enhanced the performance of the concrete in term of ductility and strength. A continuous steel straps in the form of multilayer distributed the confining stress exerted from concrete core during loading evenly between layers, thus extended the deformation of the column and subsequently delayed the failure.

1.2 Background of Concrete Confinement

Lateral confinement was originally introduced by Considere (Sakai and Sheikh 1989) in the form of internal spiral reinforcements in concrete columns. Richart, Brandtzaeg and Brown (cited from Roy and Sozen 1964) proposed the following relationship for strength and confining pressure of confined concrete, based on the results of extensive experimental programme from the first published research. This relationship is applied to both passive spirally reinforced and active hydraulically confined columns.

$$f_{cc} = f_{co} + 4.1 f_r \tag{1.1}$$

where:

 f_{cc} is a longitudinal stress for confined concrete,

 f_{co} is a longitudinal stress for unconfined concrete, and

 f_r is a lateral confining stress.

Confinement reinforcement in a form of rectangular ties or spirals is commonly used to confine the concrete since the early years of reinforced concrete structural design. However, for high-strength concrete column, its high brittleness cannot be successfully eliminated using conventional ties and spirals (KrstulovicOpara and Thiedeman 2000); and the use of closely spaced interlocking ties or spiral to increase strength and ductility may associate with construction-related problems. For columns strengthening, there are three methods of confining the concrete, namely reinforced concrete jacketing, steel jacketing and fiber reinforced polymer (FRP) -wrapping. Reinforced concrete jacketing requires formwork and considerable increase in weight and cross-section of the column. Steel jacketing and FRP-wrapping are also labour intensive and costly. Although FRP confined concrete gains high strength enhancement, but evidences show that it fails in brittle mode (Ortega, 2006, Valdmanis et al. 2007). Furthermore, the jacket and wrapped confinement materials using the conventional techniques are proven not fully utilised especially for those concrete with small lateral dilation.

1.3 Significance of Research

The most fundamental issue in predicting the behaviour of reinforced concrete members is the stress-strain behaviour of the constituent materials. Concrete is used to resist compression and its behaviour in compression is important to the designer. If the behaviour of concrete subjected to uniaxial compression is known, the structural behaviour of reinforced concrete can be estimated.

The confinement steel requirements for normal-strength concrete are reasonably well established in current building codes (ACI 318, Canadian code, 1994) but not for the high-strength concrete; high-strength concrete real constitutive behaviour is almost nonexistent. This concern arise from the fact that the requirements for design and detailing of concrete in different model codes are primarily empirical and are developed based on experimental data obtained from testing the concrete specimens having compressive strength below 40MPa (ACI-363R, 1992). While designing a structure using high-strength concrete, the designer usually ignores the enhanced properties of confined concrete and possible changes in the overall response of the structure because of lack of adequate code guidance.

Also, the existing models for confined high-strength concrete are mostly derived by calibrating of experimental results of normal strength concrete. This may not be a safe approach since the mechanical properties of high-strength concrete differ with those of normal strength concrete.

This research provides experimental data on the stress-strain behaviour of confined and unconfined high-strength concrete. The data produced in this study were generated by considering variables such as different properties of steel straps, levels of pre-tensioning stress, numbers of layers and various amount of confinement. Such data are useful in order to develop a stress-strain model of high-strength concrete and compared with the existing models.

1.4 Objectives

The main objective of the research is to develop structurally ductile highstrength concrete with superior deformation property using a new technique of pretensioning and multilayer straps. The objective also includes generating of experimental data on an innovative confinement technique, with emphasis on investigating of the stress-strain response of confined concrete within elastic and inelastic ranges. The detailed objectives are as follows:

- 1. To develop a confinement technique using steel straps of various properties to provide lateral confining pressure on high-strength concrete.
- 2. To investigate the stress-strain behaviour of high-strength concrete confined with different levels of pre-tensioning force and varying number of strap layers.
- 3. To develop a model to predict the stress-strain behaviour of high-strength concrete with pre-tensioning lateral confinement technique.

1.5 Scope of Research

The objectives of the research programme were realised within the following scope:

- Review of previous research on the behaviour of concrete columns tested under different load conditions to determine relevant parameters for the present study.
- Review of existing stress-strain models for concrete confinement. The models are examined based on parameters used for their derivation such as concrete strength, type of confinement, type of confinement materials etc.
- Design and construction of stress and strain measurement devices for testing of high-strength concrete cylindrical specimens.
- Examination the suitable concrete mix designs and standards for highstrength concrete. Determination the proportions for concrete strength ranging from 50 to 80MPa.
- Investigation of the advantages and disadvantages of various properties of steel straps with the purpose of effective confining stress generating during wrapping and loading. The straps properties are vary in strength elongation and dimension as well.
- Examination of the various possibilities for connecting the straps and investigation of the effectiveness of the connection. Design suitable prefabricated straps in layers accounted for a self adjustment for purpose of distributing confining stress between layers.
- Evaluation interpretation and verification of test data with respect to specific parameters.

- Theoretical analysis of the behavior of confined concrete and compared with experimental results.
- Identification of a rational model for ductile high-strength concrete columns subjected to axial compressive loads.
- Preparation of thesis and presentation of results.

1.6 Outline of Thesis

In chapter 2, a literature survey on high-strength concrete and confinement is presented. How does confinement work and affects the ductility, the effect of configuration on confinement and the effective confinement area on different sections are discussed in different sections of this chapter. A review on conventional models of concrete confinement based on steel is also presented in this chapter.

Chapter 3 presents the experimental methodology. Four phases of the experimental programme are briefly described at the beginning of the chapter. Phases one and two focused on the effects of pre-tensioning stress to the confined concrete whereas phases three and four investigated on multi-layer effects to the concrete. All phases mentioned above investigated the behaviour of concrete in monotonic compression load. Some specimens were tested under cyclic load to verify the stress-strain response of the confined concrete. The prefabrication of innovative straps confinement is presented towards the end of the chapter.

Chapter 4 presents the fabrication and arrangement of new devices used to measure strain and deformation of confined concrete. The devices were calibrated using established confined normal strength concrete.

Chapter 5 presents all the results carried out in phases one and two. The stress-strain behaviour, volumetric strain and mode of failure for specimens confined

in a single layer of different properties of steel straps are discussed. The results of unconfined specimen are also presented. The results presented in this chapter were emphasised on the effects of pre-tensioning stress and properties of steel straps.

Chapter 6 presents the stress-strain behaviour, volumetric strain and the failure mode for specimens confined in multilayer straps using different properties of steel straps. This chapter presents the results of experimental works in phases three and four. The concrete cylinder compressive strengths used in this project were 50, 60 and 80 MPa and the type of loads applied to the specimens were axial monotonic load as well axial cyclic load.

In Chapter 7, a detail discussion of the results is presented. The experimental data tabulated in chapters 5 and 6 were presented in form of graphs and verified with the data produced from existing models. Six equations are proposed to predict the stress-strain behavior of high-strength concrete. The results generated from the proposed equations were compared with existing stress-strain models available in literature.

In the final chapter, the general conclusions are drawn from the work described above and are presented together with recommendations for future research in this area.

REFERENCES

- ACI-363R (1992), "State-of-the-Art Report on High Strength Concrete." American Concrete Institute, Detroit, Michigan, 55 pp.
- ACI Committee 363 report (1997): "State of the Art report on High Strength Concrete" ACI363R-1997
- ACI Committee 318 (2002) "Building Code Requirements for Structural Concrete (ACI 318-02) and Commentary (318R-02)", American Concrete Institure, Farmington Hills, MI, 443 pp.
- ACI-318(2005). "Building code requirements for structural concrete and commentary", American Concrete Institute, Farmington Hills, MI.
- Ali, A. M., Kypros, P., Ki, S. S., (2003), "RC column strengthening by lateral pretensioning of FRP", Construction and Building Materials, 17, (2003), pp. 491-497.
- Arthur, H. Nilson (1994), "Chapter 7 Structural Members, High Performance Concrete: Properties and Application", McGraw-Hill, Inc., 213-236.
- Attard, M. M. and Setunge, S. (1996), "Stress-strain relationship of confined and unconfined concrete," ACI Materials Journal, V93-M49, 432-442.
- Bagsarian, T. (2008). "*High-Strength Concrete, a Historical Background*." The Concrete Producer Online.
- Beeby, A. W. (1997), "Ductility in reinforced concrete: why is it needed and is it achieved?" The structural Engineering Vol. 75/No 18, pp. 311-318.
- Beshr, H., Almusallam, A. A., Maslehuddin, M., (2003), "Effect of coarse aggregate quality on the mechanical properties of high strength concrete," ELSEVIER, Construction and Building Materials 17 (2003) 97-103 pp.
- Bill, P., (2003), "High Strength Concrete," Advance Concrete Techonology, ELSEVIER Butterworth-Heinemann, 3/1-3/16.

- Boverket (1994). "Manual for concrete structures from the Swedish Board of Housing, Building and Planning." BBK 94, Band 1, Konstruktion, Byggavdelningen, Karlskrona, Sweden (in Swedish).
- Breitenbiicherb, D.-I. R. (1998). "Developments and applications of highperformance concrete." RILEM: Materials and Structures, 31, 209-215.
- Buyukozturk, O. and Hearing, B. (1998), "Failure Behaviour of Precracked Concrete Beams Retrofitted with FRP," Journal of Composites for Constructuin, August 1998, P138-144.
- Eurocode-2, (EC-2), (2004), EN 1992-1-1:2004: "Design of concrete structures-Part 1.1: General rules and rules for buildings", CEN Brussels, Dec 2004.
- Eurocode-8, (EC-8), (2003): "Design of structures for earthquake resistance- Part 1: General rules, seismic actions and rules for building" EN 1998-1
- CEB-FIP-1990, "CEB-FIP Model Code (1990)". Thomas Telford, 437
- CEB, (1994), "Working Group on High Strength/High Performance Concrete. Application of High Performance Concrete". CEB Bulletin No. 222, Lausanne, Switzerland, Nov, 66 pp.
- Cheong, H. K. and Perry, S. H. (1993), "*Cyclic Loading of Laterally Confined Concrete Columns*," Materials and Structures, **26**, (1993), 557 562.
- Choi, S. & Shah, S. P. (1998). "Fracture Mechanism in Cement-Based Materials Subjected to Compression". Journal of Engineering Mechanics, 124, 94-102.
- Chuang, P. H., and Kong, F.K. (1997). "Large-Scale Tests on Slender, Reinforced Concrete Columns." The Structural Engineer, Journal of the institution of Structural Egineers(London), vol.75, No. 23 & 24, pp. 410-416.
- CSA-A23.3-04, (2006). "Design of Concrete Structures", Canadian Standards Association.
- Cusson, D. and Paultre, P. (1994) "High-Strength-Concrete Columns Confined by Rectangular Ties." Journal of .Structural Engineering, 120, 783-803.
- Fardis, M. N., and Khalili, H. (1982). "FRP-encased concrete as a structural material." Mag. Concrete Res., 34(121), 191-202, Cement and Concrete Assoc., London, U.K.
- Farran J., (1956). "Contribution minérlogique á l'étude de l'adhérence entre constituantshydratés des cimenset les matériauxenrobés." Revue des Matériaux de Constructions 1956;(490):491–92.

- FEMA-273, (1997). NEHRP, "Guidelines for the seismic rehabilitation of buildings."Federal Emergency Management Agency, Washington, D. C.
- FIP/CEB (1990), "High Strength Concrete: State of the Art Report." Bulletin d'information No.197, London, UK, 61 pp.
- Foster, S. J. (1999). "Design and Detailing of High Strength Concrete Columns." School of Civil and Environmental Engineering, The University of New South Wales, Sydney, Australia.
- Harries, K. A. and Kharel, G. (2002), "*Experimental investigation of the behaviour of variably confined concrete*," Cement and Concrete Research 2267.
- Hisham, A. F. and Shuaib, H. A. (1989), "Behavior of hoop-confined high-strength concrete under axial and shear loads," ACI Structural Journal, V86-S63, 652-659.
- Ho, J. C. M., Lam, J. Y. K. and Kwan, A. K. H. (2010). "Effectiveness of adding confinement for ductility improvement of high-strength concrete columns." Engineering Structures, 32, 714-725.
- Hong, K.-N., Han, S.-H. and Yi, S.-T. (2006). "High-strength concrete columns confined by low-volumetric-ratio lateral ties." Engineering Structures, 28, 1346-1353.
- Hoshikuma, J., Kawshima, K., Nagaya. K., and Taylor, A.W. (1997). " Stress-strain model fo confined concrete in bridge piers.' Journal of Structural Engineering, 129(10), 1322-1329.
- Hsu, C.-T. T., Hsu, L. S. M. and Tsao, W.-H. (1995). "Biaxially Loaded Slender High-Strength Reinforced Concrete Columns With and Without Steel Fibres." Magazine of Concrete Research, 47, 299-310.
- Jiratatprasot, P. (2002). "Mechanical Properties and Stress-Strain Behavior of High Performance Concrete Under Uniaxial Compression." M.Sc Thesis, New Jersey Institute of Technology.
- Karabinis, A. I. and Kiousis, P. D. (1994). "Effects of Confinement on Concrete Columns: Plasticity Approach." Journal of Structural Engineering, 120, 2747-2767.
- Karbhari, V. M. and Gao, Y. (1997). "Composite Jacketed Concrete under Uniaxial Compression---Verification of Simple Design Equations." Journal of Materials in Civil Engineering, 9, 185-193.
- Klink, S. A. (1985). "Actual Poisson Ratio of Concrete." ACI, 813-817.

- Kono, S., Arai, Y., Bechtoula, H., and Watanabe, F. (2003). "Damage assessment of reinforced concrete columns under high-axial loading." International Conference on Performance of Construction Materials, Cairo. Vol.1: 291-300.
- Kono, S., Inazumi, M., and Kaku., T. (1998). "Evaluation of Confining Effects of CFRP Sheets on Reinforced Concrete Members." International Conference on Composites in Infrastructures (ICCI'98), 343-355.
- Kotsovos, M. D., Pavlovic, M. N. and Cotsovos, D. M. (2008). "Characteristic features of concrete behaviour: Implications for the development of an engineering finite-element tool." Computers and Concrete, 5.
- Kotsovos, M. D. and Perry, S. H. (1986). "*Behaviour of Concrete Subjected to Passive Confinement*." Materials and Structures, Vol-19: 259-264.
- Lahlou, K., Aitcin, P. C. and Chaallal, O. (1992). "Behavior of High-Strength Concrete under Confined Stresses." Cement & Concrete Composites, 14, 185-193.
- Lam, L., Teng, J. G, Cheung, C. H., and Xiao, Y. (2006), "FRP-confined concrete under axial cyclic compression," ELSEVIER, Cement & Concrete Composite, 28, 949 – 958.
- Lee, G. C., Shih, T. S.,and Chang, K.C. (1988). "Mechanical Properties of High-Strength Concrete at Low Temperature." Journal of Cold Regions Engineering, Vol. 2, No.4, pp. 169 – 178.
- Li, G. Y., Xie, H. C., and Xiong, G. J., (2001), "Transition zone of new and old concrete with different binder, cement & concrete compesite", Elsevier
- Liu, J. and Foster, S. J. (1998). "Finite-Element Model for Confined Concrete Columns." Journal of Structural Engineering ASCE, 124, 1011-1017.
- Madas, P. (1993) "Advanced Modelling of Composite Frames Subjected to Earthquake Loading," PhD Thesis, Imperial College, University of London, London, UK.
- Mander, J. B., Priestley, M. J. N. and Park, R. (1988). "Observed Stress-Strain Behavior of Confined Concrete." Journal of Structural Engineering, 114, 1827-1849.
- Mander, J. B., Priestley, M. J. N., and Park, R. (1988) "Theorectical stress-strain model for confined concrete," J. Struct Eng., ASCE, 114(8), (1988), 1804-1826.

- Martinez-Rueda, J. E. and Elnashai, A. S. (1997), "Confined concrete model under cyclic load," Materials and Structures, Vol. 30, pp 139-147.
- Mehta, P. K. and Aïtcin, P.-C. (1990), "Principles Underlying Production of High-Performance Concrete," Cement, Concrete, and Aggregates, ASTM, Vol. 12, No. 2, Winter, pp. 70-78.
- Minder, S. et. al. (1994), "Chapter 1 Material Selection, proportioning and quality control," High Performance Concrete: Properties and Application, McGraw-Hill, Inc., 1-25.
- Mirmiran, A., and Shahawy, M., (1997), "Dilation Characteristics of Confined Concrete", Mechanics of Cohesive-Frictional Materials, 2(237-249).
- Mirmiran, A., Rizkalla, S., Russell, P., and Mast, R., (2003), "NCHRP Project 12-64, Interim Report", National Cooperative Highway Research Program, Raleigh, pp. 118.
- Moghaddam, H., Pilakoutas, K., Samadi, M., Mohebbi, S.,(2010), "Axial Compressive Behaviour of Concrete actively Confined by Metal Strips; Part ; Experimental Study," Material And Structures
- Nawy, E. G., (1996). "Fundamentals of High Performance Concrete", 1sted, Longman, Londan.
- Neville, A. M. (2002), "Properties of concrete, Fourth and Final Edition," Pearson, Prentice Hall, ISBN 0-582-23070-5, OCLC 33837400.
- Nielsen, C. V. (1998). "Triaxial Behavior of High Strength Concrete and Mortar." ACI Materials Journal, 95.
- Ortega, J.A. (2006), "Assessment of FRP-confined concrete: Understanding behavior and issues in nondestructive evaluation using radar." M.Sc Thesis, Massachusetts Institute of Technology.
- Panagiotakos, T. B. & Fardis, M. N. (2001). "Deformations of Reinforced Concrete Members at Yielding and Ultimate." ACI Structural Journal, 98.
- Parenchio, W. F., and Klieger, P. (1978). "Some Physical Properties of High-Strenth Concrete." Research and Development Bulletin RD056.01T, Port., Cem., Assoc., 6p.
- Park, R. and Paulay, T, (1975), "Reinforced concrete structure." New York: John Wiley & Sons; 769 pages.

- Paulay, T. and Priestley, M.J.N., (1992). "Seismic Design of Reinforced Concrete and Masonry Buildings," John willey & Sons, Inc., New York.
- Paudel, S. (2006). "Development of Very High Strength Concrete" "The Art of an Engineer". Structural Engineer at Commonwealth.
- Persson, B. (1999). "*Poisson's ratio of high-performance concrete*." Cement and Concrete Research, 29, 1647-1653.
- Pessiki, S. & Pieroni, A. (1997). "Axial Loaded Behavior of Large Scale Spirally Reinforced High Strength Concrete Columns." ACI Structural Journal, 94.
- Popovics, S., (1973). "A Numerical Approach to the Complete Stress-Strain Curves for Concrete." Cement and Concrete Research, 3(5), 583-599.
- Rashid, M. A., and Mansur, M. A. (2009). " Considerations in Producing High-Strength Concrete". Journal of Civil Engineering, The Institution of Engineers (IEB), Bangladesh, Vol. 37, No.1, 43 – 55.
- Razvi, S. R. & Saatcioglu, M. (1994). "Strength and Deformability of Confined High Strength Concrete Columns." ACI Structural Journal.
- Razvi, S. R. and Saatcioglu, M., (1996). "Tests of high strength concrete columns under concentric loading", Dept. of Civil Eng., University of Ottawa, Report OCEERC 96-03, 147 pp.
- Ross, J. and Muhammad, N. S. Hadi. (2008), "The effect of confinement shapes on Over-reinforced HSC Beams", Proceedings of World Academy of Science, Engineering and Technology Volume 30, July 2008, ISSN 1307-6884.
- Saadtamanesh, H., Ehsani, M. R., and Li, M. W. (1994), "Strength and ductility of concrete columns externally reinforced with fiber composite straps," ACI Structural Journal, V91-S43, 434-447.
- Saatcioglu, M. and Razvi, S. R., (1992), "Strength and ductility of confined concrete." J. Struct. Div., 118: 1590-1607.
- Saatcioglu, M. and Razvi, S. R. (1998). "High-Strength Concrete Columns with Square Sections under Concentric Compression." Journal of Structural Engineering, 124, 1438-1447.
- Sakai, J. and Kawashima, K. (2006), "An unloading and reloading stress-strain model for concrete confined by tie reinforcements," J. StructEng, ASCE; 132(1): 112-22.

- Sakai, K. and Sheikh, S. A. (1989). "What Do We Know about Confinement in Reinforced Concrete Columns?" (A Critical Review of Previous Work and Code Provisions). ACI Structural Journal, 86, 192-206.
- Setunge, S., Attard, M. M., and Darvall, P.I., (1993), "Ultimate Strength of Confined Very High-Strength Concretes." ACI Structural Journal, 90(6), 662-641.
- Sheikh, S. A., and Uzumeri, S. M., (1980), "Strength and ductility of tied concrete columns." J. Struct. Div., 106: 1079-1101.
- Sheikh, S. A., and Uzumeri, S.M., (1982), "Analytical Model for Concrete Confinement in Tied Columns." Journal of the Structural Division, ASCE, 108(12), 2703-2722.
- Sinha, B. P., Gerstle, K. H., and Tulin, L. G. (1964), "Stress-strain relation for concrete under cyclic loading," J. Amer. Concr. Inst. 61 (2), 195-211.
- Sugano, S., Kimura, H. and Shirai, K. (2007). "Study of New RC Structures Using Ultra-High-Strength Fiber-reinforced Concrete (UFC) - The Challenge of Applying 200 MPa UFC to Earthquake Resistant Building Structures." Journal of Advanced Concrete Technology, 5, 133-147.
- Tan, T. H. and Kong, F. K. (1992), "Effects of external confinement on concrete columns".
- Tomosawa, F. and Noguchi, T. (1993). "Relationship Between Compressive Strength and Modulus of Elasticity of High-Strength Concrete." The Third International Symposium on Utilization of High Strength Concrete held in Lillehammer, Norway in June 1993).
- Trezos, C. G. (1997). "*Reliability considerations on the confinement of RC columns for ductility*." ELSEVIER: Soil Dynamics and Earthquake Engineering, 16.
- Vallenas, J., Bertero, V. V., and Popov, E. P. (1977). "Concrete confined by rectangular hoops and subjected to axial loads." Report No. UCB/EERC-77/13, Earthquake Engineering Research Center, University of Califonia, Berkeley, Calif.
- Wehbe, N. I., Saiidi, M. S. and Sanders, D. H. (1998). "Confinement of Rectangular Bridge Columns for Moderate Seismic Areas." MCEER Bulletin, 12.
- Yong, Y. K., Malakah, G. N., and Edward, G. N. (1988), "Behavior of laterally confined high-strength concrete under axial loads," Journal of Structural Engineering, ASCE, Vol.114-No2, 322-351.

Zisman, J. G. (1982), "Behaviour of concrete under biaxial cyclic compression," Master of Science Thesis, Massachusetts Institute of Technology.