PERFORMANCE OF KAOLIN-TIRE DERIVED AGGREGATE MIXTURE AS A BACKFILL MATERIAL FOR RETAINING WALL

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PERFORMANCE OF KAOLIN-TIRE DERIVED AGGREGATE MIXTURE AS A BACKFILL MATERIAL FOR RETAINING WALL

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A thesis submitted in fulfilment of the requirements for the award of the degree of Doctor of Philosophy (Civil Engineering)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > SEPTEMBER 2014

Librarian

Perpustakaan Sultanah Zanariah UTM, Skudai Johor

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Thank you. Sincerely yours,

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To my beloved parents and sister, your courage and compassion have taught me humility

ACKNOWLEDGMENTS

I wish to express my sincere appreciation to my main supervisor, Prof. Dr. Khairul Anuar Kassim of the Faculty of Civil Engineering UTM for his dedication, encouragement, guidance, and support. I am also very thankful to my co-supervisors Assoc. Prof. Dr. Kamarudin Ahmad and Dr. Ahmad Safuan A Rashid for their guidance and advices. The assistance of Geotechnical laboratory staff especially Mr Zulkify and Mrs Ros are highly appreciated.

Last but not least, I would like to acknowledge my beloved parents and my sister for their support and love.

ABSTRACT

Backfill material for retaining wall should be lightweight in order to reduce the lateral pressure behind the wall. In addition, high permeability backfill walls are designed to eliminate or minimize the development of pore water pressure. Tire Derived Aggregate (TDA) has low unit weight and high permeability. Moreover, reutilizing solid wastes like TDA with cohesive soil as backfill material may reduce sand mining and preserve the environment to achieve sustainable construction. Therefore, the aim of this study was to evaluate the performance of Kaolin-TDA mixtures as a backfill material for retaining wall. Geotechnical properties of Kaolin, TDA and Kaolin-TDA mixtures were determined in addition to compaction and hydraulic conductivity tests. A total of 13 scaled down 1:20 physical model tests on polymer concrete retaining wall using Kaolin and different mixtures of Kaolin-TDA as backfill material were performed. Footing settlements and wall displacements due to loading on fabricated steel as strip footing in the model test were measured. The results were verified using three numerical methods, namely PLAXIS 2D, Genetic Programming and Multiple Linear Regression. The maximum dry density of Kaolin-TDA mixture was in the range of 964 kg/m³ to 1590 kg/m³, lighter than the maximum dry density of Kaolin 1750 kg/m³. Therefore, using Kaolin-TDA mixture as backfill material reduced the unit weight of backfill in a range of 9% to 45% resulted in a decrease in lateral pressure. Physical modeling and permeability tests results showed that mixture of Kaolin with 20% Granular (5-8 mm) TDA was the optimum mixture. The permeability of the optimum mixture was 2.56 times higher than that of the Kaolin. It was also observed from the physical modeling that by using the optimum mixture as backfill material, it resulted in footing stress roughly 3 times higher than using Kaolin without TDA. Thus, by mixing TDA with Kaolin, the weight of backfill decreases, permeability increases and footing stress increases compared to Kaolin as a backfill material for retaining wall.

ABSTRAK

Bahan kambus balik tembok penahan seharusnya ringan bagi mengurangkan tekanan sisi di belakang tembok. Di samping itu, kebolehtelapan yang tinggi bagi bahan kambus balik direka untuk menyahkan atau meminimumkan pembentukan tekanan air liang. Agregat Yang Didapati Daripada Tayar (TDA) mempunyai berat unit yang rendah dan kebolehtelapan yang tinggi. Disamping itu, penggunaan semula bahan-bahan buangan pepejal seperti TDA dengan tanah berjelekit sebagai bahan kambus balik dapat mengurangkan perlombongan pasir dan memelihara persekitaran bagi mencapai pembinaan lestari. Tujuan kajian ini ialah untuk menilai prestasi campuran Kaolin-TDA sebagai bahan kambus balik bagi tembok penahan. Sifat-sifat geoteknikal Kaolin, TDA dan campuran Kaolin-TDA telah ditentukan di samping ujian-ujian pemadatan dan keberaliran hidraulik ke atas campuran-campuran itu. Sejumlah 13 ujian model fizikal tembok penahan konkrit polimer yang telah diskala kecilkan 1:20 menggunakan Kaolin dan campuran Kaolin-TDA yang berlainan sebagai bahan kambus balik telah dilakukan. Enapan asas jalur dan pergerakan tembok penahan akibat pembebanan ke atas plat keluli yang difabrikasi sebagai asas jalur dalam ujian model telah diukur. Keputusan yang diperolehi daripada model fizikal telah disahkan menggunakan tiga kaedah berangka iaitu PLAXIS 2D, Pengaturcaraan Genetik dan Berbilang Regrasi Lelurus. Ketumpatan kering maksima bagi campuran Kaolin-TDA antara 964 kg/m³ dan 1590 kg/m³ adalah lebih ringan dari Kaolin yang mempunyai ketumpatan kering maksima 1750 kg/m³. Oleh itu, campuran Kaolin-TDA sebagai bahan kambus balik telah penggunaan mengurangkan berat unit antara 9% hingga 45% menyebabkan penurunan tekanan sisi di belakang tembok. Model fizikal dan hasil ujian kebolehtelapan menunjukkan bahawa campuran Kaolin dengan 20% Granular (5-8 mm) TDA adalah campuran yang optima. Kebolehtelapan campuran optima tersebut adalah 2.6 kali lebih tinggi dari kebolehtelapan Kaolin. Kajian dari model fizikal juga menunjukkan bahawa penggunaan campuran optima sebagai bahan kambus balik telah menyebabkan tekanan penapak asas jalur 3 kali lebih tinggi dari menggunakan Kaolin tanpa TDA. Oleh itu, dengan campuran TDA dan Kaolin, berat kambus balik berkurangan, kebolehtelapan meningkat dan tekanan asas jalur meningkat berbanding dengan Kaolin sebagai bahan kambus balik bagi tembok penahan.

TABLE OF CONTENT

| CHAPTER | | TITLE | PAGE |
|---------|------|--------------------------|------|
| | DE | CLARATION | ii |
| | DE | DICATION | iii |
| | ACI | KNOWLEDGMENTS | iv |
| | ABS | STRACT | v |
| | ABS | STRAK | vi |
| | TAI | BLE OF CONTENT | vii |
| | LIS | T OF TABLE | XV |
| | LIS | T OF FIGURE | xix |
| | LIS | T OF SYMBOLS | xxvi |
| | LIS | T OF ABREVIATION | xxix |
| | LIS' | T OF APPENDICES | xxix |
| 1 | INT | RODUCTION | 1 |
| | 1.1 | Background of the Study | 1 |
| | 1.2 | Problem Statement | 3 |
| | 1.3 | Objectives of the Study | 4 |
| | 1.4 | Scope of the Study | 5 |
| | 1.5 | Significant of the Study | 5 |
| | 1.6 | Thesis Organization | 6 |

| LITE | RATURE REVIEW | 7 |
|------|---|----|
| 2.1 | Introduction | 7 |
| 2.2 | Properties of Fine-Grained Soil | 7 |
| | 2.2.1 Microstructure of Fine-Grained Soil | 8 |
| | 2.2.2 Physical Properties of Fine-Grained Soil | 9 |
| 2.3 | Backfill Material | 10 |
| | 2.3.1 Fine-Grained Soil as a Backfill Material | 11 |
| | 2.3.1.1 Classification of Fine-Grained Soil | 11 |
| | 2.3.1.2 Compaction of Fine-Grained Soil | 11 |
| | 2.3.1.3 Permeability of Fine-Grained Soil | 12 |
| | 2.3.1.4 Shear Strength of Fine-Grained Soil | 14 |
| | 2.3.1.5 Shear Strength of Fine-Grained Soil-Granular | |
| | Mixture | 14 |
| | 2.3.2 Modification of Fine-Grained Soil | 16 |
| | 2.3.3 Recycled Material as a Backfill Material | 16 |
| 2.4 | Waste Tires | 19 |
| | 2.4.1 Shredded Tire Properties | 20 |
| | 2.4.2 Shredded Tire Size Limitation | 22 |
| | 2.4.3 Shredded Tire Preparation and Refining Processes | 24 |
| | 2.4.4 Preparing a Soil-Shredded Tire Mixture | 25 |
| | 2.4.5 Optimum Tire Shred-Soil Mixing Ratio | 26 |
| | 2.4.6 Laboratory Tests of Soil-Shredded Tire Mixture | 27 |
| | 2.4.6.1 Compaction on Sand-Shredded Tire Mixture | 27 |
| | 2.4.6.2 Specific Gravity of Soil-Shredded Tire Mixture | 30 |
| | 2.4.6.3 Permeability of Soil-Shredded Tire Mixture | 31 |
| | 2.4.6.4 Shear Strength of Soil-Shredded Tire Mixture | 36 |
| | 2.4.6.5 Lateral Earth Pressure Coefficient (K_0) of | |
| | Soil-Shredded Tire Mixture | 44 |

2

viii

| | 2.4.6.6 Poisson's Ratio | 45 |
|-----|--|----|
| | 2.4.6.7 Elastic Modulus of Soil-Shredded Tire Mixture | 48 |
| | 2.4.6.8 Stress-Strain Behavior and Unconfined | |
| | Compressive Strength | 48 |
| | 2.4.6.9 Porosity and Void Ratio | 49 |
| | 2.4.7 Bearing Capacity of Soil-Shredded Tire Mixture | 50 |
| | 2.4.8 Cost Effective of Shredded Tire on Soil | 51 |
| | 2.4.9 Shredded Tire as a Lightweight Material | 51 |
| | 2.4.10 Shredded Tire as a Draining Layer | 53 |
| | 2.4.11 Environmental Properties | 54 |
| | 2.4.12 Advantages of Shredded Tire Using | 55 |
| | 2.4.13 Shredded Tire Effects on the Water Table | 56 |
| | 2.4.14 Dynamic Properties of Dry Sand-Rubber (SRM) and Gravel-Rubber (GRM) Mixtures | 58 |
| 2.5 | Soil-Shredded Tire Applications | 58 |
| 2.3 | | 50 |
| | 2.5.1 Highway Embankment Application of Soil-Shredded Tire Mixture | 63 |
| | 2.5.2 Soil-Shredded Tire Mixture as Backfill Material | 64 |
| | 2.5.3 Soil-Shredded Tire Mixture as Landfill | 65 |
| | 2.5.4 Use of Waste Tire Rubber for Swelling in Expansive | |
| | Soils | 66 |
| | 2.5.6 Tire Derived Aggregates (TDA) as a Sustainable | |
| | Recycled Material for Retaining Wall Backfills | 67 |
| 2.6 | Retaining Walls | 67 |
| | 2.6.1 Active Earth Pressure Calculation in Retaining Walls | |
| | with Reinforced Backfill | 69 |
| | 2.6.2 Geometry and Instability Modes of Retaining Walls | 70 |
| | 2.6.3 Reinforced Soil and Retaining Wall Dimension | 73 |

| | 2.6.4 Shear Zone in Earth Pressure of a Retaining Wall | 74 |
|------|--|-----|
| | 2.6.5 Retaining Walls for Basement Construction in Stiff Clays | 74 |
| | 2.6.6 Response of Retaining Wall Backfilled with Shredded Tire | 74 |
| 2.7 | Polymer Concrete | 78 |
| 2.8 | Modeling | 82 |
| | 2.8.1 Finite Element Model Development and Simulation Procedure | 85 |
| | 2.8.2 Mesh Sensitivity Study | 86 |
| 2.9 | Genetic Programming (GP) | 87 |
| 2.10 | Summary | 91 |
| МЕТ | HODOLOGY | 94 |
| 3.1 | Introduction | 94 |
| 3.2 | General Characterization of Material | 95 |
| | 3.2.1 Kaolin | 97 |
| | 3.2.2 Tire Derived Aggregate (TDA) | 98 |
| | 3.2.2.1. TDA Types Used in the Mixtures | 98 |
| | 3.2.2.2. Physical Properties | 99 |
| | 3.2.3 Kaolin-TDA Mixtures | 99 |
| 3.3 | Sample Preparation | 100 |
| 3.4 | Experimental Tests | 100 |
| | 3.4.1 Sieve Analysis | 100 |
| | 3.4.2 Hydrometer Test | 100 |
| | 3.4.3 Atterberg Limit Test | 101 |
| | 3.4.4 Specific Gravity Test (G_s) | 102 |
| | 3.4.5 Compaction Test | 105 |

3

| 3.4.5.1 Material and Test Procedures | 107 |
|---|-----|
| 3.4.5.1.1 Sample Preparation | 107 |
| 3.4.5.1.2 Soil Compaction Test | 108 |
| 3.4.5.2 Compaction Energy | 109 |
| 3.4.5.3 Zero-Air-Void Unit Weight | 109 |
| 3.4.6 Permeability Test | 110 |
| 3.4.7 Triaxial Test | 111 |
| Modeling of Retaining Wall and Backfill | 114 |
| 3.5.1 Physical Modeling of Retaining Wall and Backfill | 114 |
| 3.5.1.1 Retaining Wall | 114 |
| 3.5.1.1.1 Retaining Wall Scaling | 115 |
| 3.5.1.1.2 Retaining Wall Dimension | 117 |
| 3.5.1.1.3 Retaining Wall Material | 118 |
| 3.5.1.2 Dimension and Scaling of Physical Modeling | |
| Box | 118 |
| 3.5.1.3 Backfill Material | 120 |
| 3.5.1.4 Strip Footing Model and Loading System | 121 |
| 3.5.1.5 Elasticity Modulus | 122 |
| 3.5.1.6 Types of Failure in Soil | 122 |
| 3.5.2 Numerical Modeling of Retaining Wall and Backfill | |
| Using PLAXIS | 124 |
| 3.5.2.1 Backfill Material Properties Used in PLAXIS | 125 |
| 3.5.2.2 Retaining Wall Properties Used in PLAXIS | 125 |
| 3.5.2.3 Finite Element Modeling Methods | 126 |
| 3.5.2.3.1 Selection of Appropriate Finite | |
| Element Mesh | 127 |
| 3.5.2.3.2 Selection of Appropriate Finite | |
| Element Domain | 127 |

3.5

| | | 3.5.2.3.3 Final Finite Element Model | 128 |
|---|-----|---|-----|
| | | 3.5.2.3.4 The Mohr-Coulomb Material Model | 128 |
| | | 3.5.2.3.5 Determination of the Mohr-Coulomb | |
| | | Parameters | 130 |
| | | 3.5.3 Numerical Modeling of Retaining Wall and Backfill | |
| | | Using Genetic Programming | 131 |
| | | 3.5.4 Numerical Modeling of Retaining Wall and Backfill | |
| | | Using Multiple Linear Regression (MLR) Analysis | 131 |
| | | 3.5.5 Comparison between Physical and Numerical | |
| | | Modeling Results | 131 |
| 4 | RES | ULTS AND DISCUSSION | 136 |
| | 4.1 | Introduction | 136 |
| | 4.2 | General Characterization of Material | 136 |
| | | 4.2.1 Kaolin Classification | 137 |
| | | 4.2.2 Atterberg Limit Test | 138 |
| | | 4.2.3 Specific Gravity (G_s) | 139 |
| | | 4.2.4 Compaction Test | 146 |
| | | 4.2.5 Permeability Test | 152 |
| | | 4.2.6 Triaxial Test | 156 |
| | | 4.2.6.1 Triaxial Sample Expansion | 158 |
| | | 4.2.7 Laboratory Test Results | 160 |
| | 4.3 | Modelling | 163 |
| | | 4.3.1 Physical Modeling | 164 |
| | | 4.3.1.1 Strip Footing Settlement on Different | |
| | | Kaolin-TDA Mixture Backfill | 165 |
| | | 4.3.1.2 Variation of Stress and Normalized Vertical | |
| | | Displacement of Footing on Height of | |
| | | Backfill | 170 |

| 4.3.1.3 Wall Displacement for Different Kaolin-TDA | |
|---|-----|
| Mixture Backfill | 174 |
| 4.3.1.4 Variation of Stress and Normalized | |
| Horizontal Displacement of Wall on Length | |
| of Backfill | 178 |
| 4.3.1.5 Evaluation of Stress on Different TDA | 182 |
| 4.3.1.6 The Relationship between Maximum Footing | |
| Stress and Different Percentage of TDA Mixed | l |
| with Kaolin | 186 |
| 4.3.1.7 The Relationship between Maximum Footing | |
| Stress and Different Types of TDA Mixed | |
| with Kaolin | 187 |
| 4.3.2 Numerical Modeling Using PLAXIS-2D | 190 |
| 4.3.2.1 Wall Movement | 191 |
| 4.3.2.2 Footing Settlement | 193 |
| 4.3.2.3 Equations Obtained from Numerical Results | 194 |
| 4.3.3 Comparison between Physical Modeling and | |
| Numerical Modeling (PLAXIS) Results | 196 |
| 4.3.3.1 Verification of Footing Settlement Results in | |
| Physical Modeling with Numerical Modeling | |
| Results | 197 |
| 4.3.3.2 Verification of Wall Displacement Results in | |
| Physical Modeling with Numerical Modeling | |
| Results | 199 |
| 4.3.4 Numerical Modeling Using Genetic Programming | 203 |
| 4.3.5 Numerical Modeling Using Multiiple Linear | |
| Regression Analysis | 207 |
| | |

| 5 | CONCLUSION AND RECOMMENDATION | | |
|---|-------------------------------|-----|--|
| | 5.1 Introduction | 210 | |

-

| 5.2 | Conclusion | 210 |
|----------------|---|-----------|
| | 5.2.1 General Characteristics of Kaolin-TDA Mixture | 211 |
| | 5.2.2 Performance of Polymer Concrete Retaining Wal and Different Backfill Materials | ll 212 |
| | 5.2.3 Verification of Physical Modeling Results by | |
| | Numerical Methods | 213 |
| 5.3 | Contribution of Study | 213 |
| 5.4 | Recommendation for Future Research | 214 |
| | | |
| REFERENCES | | 215 |
| Appendices A-F | | 240-279 |

LIST OF TABLES

| TA | BL | Æ | Ν | 0. |
|----|----|---|---|----|
|----|----|---|---|----|

TITLE

PAGE

| 2.1 | Basic properties of three types of clay (Horpibulsuk et al., 2011) | 10 |
|------|---|----|
| 2.2 | Shear strength of clay-granular material mixture as a function of | |
| | weight* (Vallejo and Mawby, 2000) | 15 |
| 2.3 | Advantages and disadvantages of using tire shreds as a backfill | |
| | material (Edeskär, 2004) | 18 |
| 2.4 | Representative values of technical properties of tire shreds | |
| | (Edeskär, 2004) | 22 |
| 2.5 | Unit weight of different size tire shreds (Reddy and Marella, 2001) | 29 |
| 2.6 | Values of permeability of tire shreds (Edeskär, 2004) | 33 |
| 2.7 | Hydraulic conductivity of different size tire shreds | |
| | (Reddy and Marella, 2001) | 35 |
| 2.8 | Shear strength parameters of processed tire wastes, sand and tire | |
| | waste-sand mixtures (Edinçliler et al., 2010) | 38 |
| 2.9 | Result from triaxial testing of five different tire products without | |
| | protruding steel cord (Edeskär, 2004) | 39 |
| 2.10 | Shear strength of different size tire shreds | |
| | (Reddy and Marella, 2001) | 40 |
| 2.11 | Summary of shear strength data for recycled material | |
| | (Edinçliler et al., 2004) | 42 |
| 2.12 | Summary of shear strength data (Edinçliler et al., 2004) | 44 |
| 2.13 | Average values for coefficient of lateral earth pressure at rest, K_0 , | |
| | for different depths and surcharges (After Tweedie et al., 1998) | 45 |
| 2.14 | Reported values calculated from the coefficient of lateral earth | |
| | pressure K_0 and poisson ratio (Edeskär, 2004) | 46 |

| 2.15 | Reported values of Poisson's ratio at given confined stress and | |
|------|---|-----|
| | based on direct strain measurements in triaxial tests (Edeskär, | |
| | 2004) | 47 |
| 2.16 | Porosity for different sizes of tire shreds at different pressures | |
| | (Edeskär, 2004) | 50 |
| 2.17 | Advantages and disadvantages of using tire shreds as a | |
| | lightweight material (Edeskär, 2004) | 52 |
| 2.18 | Advantages and disadvantages of using tire shreds as draining | |
| | layer | 54 |
| 2.19 | Summary of engineering properties of tire shreds | |
| | (Shalaby and Ahmed, 2005) | 62 |
| 2.20 | Unit weight of various lightweight fill materials (Yoon et al., | |
| | 2006) | 64 |
| 2.21 | Summary of observed deformation modes in different projects | |
| | (Sabermahani et al., 2009) | 72 |
| 2.22 | Properties of shredded tires gained from the literature | |
| | (Ravichandran and Huggins, 2013) | 77 |
| 2.23 | PC compositions used in the study (Gorninski et al., 2004) | 80 |
| 2.24 | Modulus of elasticity of PC with isophtalic or orthophtalic | |
| | polyester with 8, 12, 16 and 20% fly ash (Gorninski et al., 2004) | 80 |
| 2.25 | Summary of literature review | 92 |
| 3.1 | Standards used in this study | 95 |
| 3.2 | Kaolin Properties (Kaolin Malaysia Sdn. Bhd.) | 97 |
| 3.3 | Kaolin-TDA mixture percentage by weight | 99 |
| 3.4 | Kaolin-TDA mixture by weight | 108 |
| 3.5 | Classification of soil according to the coefficient of permeability | |
| | (Terzaghi and Peck, 1967; Kulhawy and Mayne, 1990; | |
| | Bardet, 1997) | 110 |
| 3.6 | Standard design chart (Huang et al., 1998) | 116 |
| 3.7 | Properties of the polymer concrete wall | 118 |
| 3.8 | Properties of the plates comprising the retaining structure, per | |
| | unit length | 126 |
| 3.9 | Experimental database used for the development of the models | 132 |
| 3.10 | Training input (70%) for footing settlement | 132 |

| 3.11 | Testing input (30%) for footing settlement | 133 |
|------|---|-----|
| 3.12 | Training input (70%) for wall displacement | 133 |
| 3.13 | Testing input (30%) for wall displacement | 133 |
| 4.1 | Atterberg limit results | 138 |
| 4.2 | Specific gravity determined using water pycnometer | 139 |
| 4.3 | Specific gravity determined using ethyl alcohol pycnometer | 140 |
| 4.4 | Maximum dry density and optimum moisture content of | |
| | Kaolin-TDA mixture | 149 |
| 4.5 | Permeability of Kaolin-TDA mixture | 152 |
| 4.6 | Maximum water content in tire shreds (Humphrey et al., 1992) | 156 |
| 4.7 | Laboratory test results for Kaolin | 161 |
| 4.8 | Properties of Kaolin and Kaolin-TDA mixtures obtained from | |
| | this study | 161 |
| 4.9 | Elasticity modulus, shear modulus, bulk modulus and | |
| | oedometer elasticity modulus of backfill materials | 163 |
| 4.10 | Comparison of mixture results between this study and literature | 163 |
| 4.11 | Physical modeling results | 164 |
| 4.12 | Approximate required movements to reach minimum active | |
| | and maximum passive earth pressure conditions | |
| | (Clough and Duncan, 1991; Ebeling and Morrison, 1993) | 174 |
| 4.13 | Equations of the variations between maximum footing stress and | |
| | footing settlement from numerical results | 195 |
| 4.14 | Equations of the variations between maximum footing stress and | |
| | wall displacement from numerical results | 195 |
| 4.15 | Equations of the variations between maximum footing stress and | 1 |
| | wall displacement on two third of the wall height from numerica | 1 |
| | results | 196 |
| 4.16 | Training output (70%) for footing settlement | 203 |
| 4.17 | Testing output (30%) for footing settlement | 203 |
| 4.18 | Training output (70%) for wall displacement | 205 |
| 4.19 | Testing output (30%) for wall displacement | 205 |
| 4.20 | Input and output of MLR compared with physical modeling | |
| | and Genetic Programming for footing settlement | 207 |
| | | |

| 4.21 | Input and output of MLR compared with physical modeling | | |
|------|---|-----|--|
| | and Genetic Programming for wall displacement | 208 | |

LIST OF FIGURES

FIGURE NO.

TITLE

PAGE

| 1.1 | Stockpiling and burning the waste tire (Environmental | |
|------|---|----|
| | Engineering and Contracting, 2002) | 4 |
| 2.1 | Catipillar D-4 spreading tire shreds for lightweight embankment | |
| | fill at Portland Jetport Interchange (Humphrey, 1999) | 23 |
| 2.2 | Shredding of car tires by Ragn-Sells AB, Sweden (Edeskär, | |
| | 2004) | 24 |
| 2.3 | Mixing of tire shred and sand (Yoon et al., 2006) | 25 |
| 2.4 | Unit weight of different size tire shreds based on reported studies | |
| | (Reddy and Marella, 2001) | 30 |
| 2.5 | Shear stress vs. horizontal displacement curves for sand, tire | |
| | buffings and sand-tire buffings mixture at vertical stress of | |
| | 20 kPa, 40 kPa and 80 kPa (Edinçliler et al., 2004) | 43 |
| 2.6 | Deformations (strains) used in definition of Poisson's ratio | |
| | (Edeskär, 2004) | 45 |
| 2.7 | Stress-strain relationship with rubber content | |
| | (Kim and Kang, 2011) | 49 |
| 2.8 | Example of road construction with tire shreds used as | |
| | lightweight material (Edeskär, 2004) | 52 |
| 2.9 | Tire shreds used as a bottom-draining layer in a coal ash landfill | |
| | (Reddy et al., 2010) | 53 |
| 2.10 | Geometry of the analyzed wall (Zevgolis and Bourdeau, 2010) | 71 |
| 2.11 | Modes of instability: (a) overturning, (b) sliding, (c) bearing | |
| | capacity, and (d) excessive eccentricity (Zevgolis and Bourdeau, | |
| | 2010) | 71 |

| 2.12 | Schematic view of the experimental model | |
|------|---|-----|
| | (Ahmadi and Hajialilue-Bonab, 2012) | 73 |
| 2.13 | A sketch of the problem being considered | |
| | (Ravichandran and Huggins, 2013) | 78 |
| 2.14 | Plane figures of (a) cell unit by a tire, (b) Tirecell, and (c) photo | |
| | of Tirecell (Yoon et al., 2008) | 82 |
| 2.15 | Shaking table test of retaining walls supporting loose backfill | |
| | under extreme seismic shaking: comparison of (a) physical | |
| | model before the test, with (b) deformed model after the test | |
| | (Anastasopoulos et al., 2010) | 84 |
| 2.16 | Schematic of the simulation domain and sample finite element | |
| | mesh (Ravichandran and Huggins, 2013) | 86 |
| 2.17 | Typical genetic programming tree representing function | |
| | $(2/x_1 + x_2)^2$ (Rezania and Javadi, 2007) | 89 |
| 3.1 | Flowchart of the research methodology | 96 |
| 3.2 | Kaolin used in this study | 97 |
| 3.3 | Tire derived aggregate used in this study | 98 |
| 3.4 | Plasticity chart (ASTM D-2487, 2006) | 102 |
| 3.5 | Use of Powdery TDA in water pycnometer | 103 |
| 3.6 | Materials and pycnometers used in specific gravity test | 104 |
| 3.7 | Triaxial samples: a) Kaolin, b) Kaolin-Shredded TDA | 111 |
| 3.8 | Preparation instrument of triaxial sample | 112 |
| 3.9 | Triaxial test apparatuses and equipment | 112 |
| 3.10 | Deviator stress for consolidated un-drained triaxial tests | |
| | (Colas <i>et al.</i> , 2010) | 113 |
| 3.11 | Retaining wall dimension (Huang et al., 1998) | 116 |
| 3.12 | Retaining wall dimension in this study | 117 |
| 3.13 | Physical modeling schematic scaled down at 1:20 | 119 |
| 3.14 | Physical modeling box | 119 |
| 3.15 | A general shear failure surface for a strip footing | |
| | (Terzaghi, 1943; Vesic, 1963; Terzaghi and Peck, 1967) | 123 |
| 3.16 | Failure type shape in physical modeling | 123 |
| 3.17 | Schematic of PLAXIS model | 124 |
| 3.18 | Generated Mesh | 127 |
| | | |

| 3.19 | Mohr-Coulomb stress-strain criterion based on PLAXIS input | |
|------|--|-----|
| | parameters (PLAXIS, 2011) | 129 |
| 4.1 | Kaolin classification | 137 |
| 4.2 | plasticity chart (ASTM D-2487, 2006) | 138 |
| 4.3 | Difference between water pycnometer and alcohol | |
| | pycnometer specific gravity for Kaolin-Powdery TDA | |
| | mixture | 141 |
| 4.4 | Difference between water pycnometer and alcohol | |
| | pycnometer specific gravity for Kaolin-Shredded TDA | |
| | mixture | 142 |
| 4.5 | Difference between water pycnometer and alcohol | |
| | pycnometer specific gravity for Kaolin-Granular (1-4 mm) | |
| | TDA mixture | 142 |
| 4.6 | Difference between water pycnometer and alcohol | |
| | pycnometer specific gravity for Kaolin-Granular (5-8 mm) | |
| | TDA mixture | 143 |
| 4.7 | Comparison of difference percentage of Kaolin-TDA | |
| | mixture in alcohol pycnometer specific gravity | 143 |
| 4.8 | Comparison of difference percentage of Kaolin-TDA | |
| | mixture in water pycnometer specific gravity | 144 |
| 4.9 | Correlation between Specific Gravities of Kaolin-TDA | |
| | mixture determined by means of water and alcohol | |
| | pycnometers | 145 |
| 4.10 | Variation of dry density and moisture content in | |
| | Kaolin-Powdery TDA mixtures | 146 |
| 4.11 | Variation of dry density and moisture content in | |
| | Kaolin-Shredded TDA mixtures | 147 |
| 4.12 | Variation of dry density and moisture content in | |
| | Kaolin-Granular (1-4 mm) TDA mixtures | 147 |
| 4.13 | Variation of dry density and moisture content in | |
| | Kaolin-Granular (5-8 mm) TDA mixtures | 148 |
| 4.14 | Compaction curves of Kaolin-TDA mixtures | 148 |
| 4.15 | Effect of TDA percentage on maximum dry density of | |
| | Kaolin-TDA mixture | 150 |

| 4.16 | Effect of TDA types on the MDD of Kaolin | 151 |
|------|---|-----|
| 4.17 | Permeability of Kaolin-TDA mixture | 152 |
| 4.18 | Schematic of specific surface of the TDA particles | 154 |
| 4.19 | SEM test results of the material used in research | 155 |
| 4.20 | Deviator stress depending on strain in triaxial tests on Kaolin | 156 |
| 4.21 | Expanded sample after triaxial test | 158 |
| 4.22 | Shredded TDA and Kaolin-Shredded TDA mixture | 159 |
| 4.23 | Physical modeling test instruments | 159 |
| 4.24 | Variation of settlement with stress in Kaolin-Powdery TDA | |
| | mixture | 166 |
| 4.25 | Variation of settlement with stress in Kaolin-Shredded TDA | |
| | mixture | 167 |
| 4.26 | Variation of settlement with stress in Kaolin-Granular | |
| | (1-4 mm) TDA mixture | 167 |
| 4.27 | Variation of settlement with stress in Kaolin-Granular | |
| | (5-8 mm) TDA mixture | 168 |
| 4.28 | Variation of settlement with stress in backfill included 20% | |
| | TDA | 168 |
| 4.29 | Variation of settlement with stress in backfill included 40% | |
| | TDA | 169 |
| 4.30 | Variation of settlement with stress in backfill included 60% | |
| | TDA | 169 |
| 4.31 | The stress-strain curve of Kaolin-Powdery TDA | 170 |
| 4.32 | The stress-strain curve of Kaolin-Shredded TDA | 171 |
| 4.33 | The stress-strain curve of Kaolin-Granular (1-4 mm) TDA | 171 |
| 4.34 | The stress-strain curve of Kaolin-Granular (5-8 mm) TDA | 172 |
| 4.35 | The stress-strain curve for 20% TDA mixed with Kaolin | 172 |
| 4.36 | The stress-strain curve for 40% TDA mixed with Kaolin | 173 |
| 4.37 | The stress-strain curve for 60% TDA mixed with Kaolin | 173 |
| 4.38 | Variation of wall displacement with stress in Kaolin-Powdery | |
| | TDA mixture | 175 |
| 4.39 | Variation of wall displacement with stress in Kaolin-Shredded | |
| | TDA mixture | 175 |

| 4.40 | Variation of wall displacement with stress in Kaolin-Granular | |
|------|---|-------|
| | (1-4 mm) TDA mixture | 176 |
| 4.41 | Variation of wall displacement with stress in Kaolin-Granular | |
| | (5-8 mm) TDA mixture | 176 |
| 4.42 | Variation of wall displacement with stress in backfill included | |
| | 20% rubber | 177 |
| 4.43 | Variation of wall displacement with stress in backfill included | |
| | 40% rubber | 177 |
| 4.44 | Variation of settlement with stress in backfill included | |
| | 60% rubber | 178 |
| 4.45 | The stress-strain curve of Kaolin-Powdery TDA | 179 |
| 4.46 | The stress-strain curve of Kaolin-Shredded TDA | 179 |
| 4.47 | The stress-strain curve of Kaolin-Granular (1-4 mm) TDA | 180 |
| 4.48 | The stress-strain curve of Kaolin-Granular (5-8 mm) TDA | 180 |
| 4.59 | The stress-strain curve for 20% TDA mixed with Kaolin | 181 |
| 4.50 | The stress-strain curve for 40% TDA mixed with Kaolin | 181 |
| 4.51 | The stress-strain curve for 60% TDA mixed with Kaolin | 182 |
| 4.52 | Maximum footing stress and Powdery TDA percentage | 183 |
| 4.53 | Maximum footing stress and Shredded TDA percentage | 183 |
| 4.54 | Maximum footing stress and Granular (1-4 mm) TDA percentag | ge184 |
| 4.55 | Maximum footing stress and Granular (5-8 mm) TDA percentag | ge184 |
| 4.56 | Maximum footing stress and 20% of different TDA types | 185 |
| 4.57 | Maximum footing stress and 40% of different TDA types | 185 |
| 4.58 | Maximum footing stress and 60 % of different TDA types | 186 |
| 4.59 | The relationship between maximum footing stress and | |
| | Kaolin-TDA mixture | 187 |
| 4.60 | The relationship between maximum footing stress and types of | |
| | TDA mixed with Kaolin | 188 |
| 4.61 | Micro-photography of Kaolin mixed with 60% Shredded TDA | 189 |
| 4.62 | Deformed mesh | 190 |
| 4.63 | Wall displacement | 191 |
| 4.64 | Maximum footing stress-wall displacement results of numerical | |
| | modeling | 192 |
| 4.65 | Footing settlement | 193 |
| | | |

| 4.66 | Maximum footing stress-footing settlement results of numerical | |
|------|--|-----|
| | modeling | 194 |
| 4.67 | Variation of maximum footing stress and footing settlement of | |
| | Kaolin-Powdery TDA mixture in numerical and physical | |
| | modeling | 197 |
| 4.68 | Variation of maximum footing stress and footing settlement of | |
| | Kaolin-Shredded TDA mixture in numerical and physical | |
| | modeling | 198 |
| 4.69 | Variation of maximum footing stress and footing settlement of | |
| | Kaolin-Granular (1-4 mm) TDA mixture in numerical and | |
| | physical modeling | 198 |
| 4.70 | Variation of maximum footing stress and footing Settlement of | |
| | Kaolin-Granular (5-8 mm) TDA mixture in numerical and | |
| | physical modeling | 199 |
| 4.71 | Variation of maximum footing stress and wall displacement of | |
| | Kaolin-Powdery TDA mixture in numerical and physical | |
| | modeling | 200 |
| 4.72 | Variation of maximum footing stress and wall displacement of | |
| | Kaolin-Shredded TDA mixture in numerical and physical | |
| | modeling | 201 |
| 4.73 | Variation of maximum footing stress and wall displacement of | |
| | Kaolin-Granular (1-4 mm) TDA mixture in numerical and | |
| | physical modeling | 201 |
| 4.74 | Variation of maximum footing stress and wall displacement of | |
| | Kaolin-Granular (5-8 mm) TDA mixture in numerical and | |
| | physical modeling | 202 |
| 4.75 | Training output (70%) for footing settlement | 204 |
| 4.73 | Testing output (30%) for footing settlement | 204 |
| 4.74 | Training output (70%) for wall displacement | 206 |
| 4.75 | Testing output (30%) for wall displacement | 206 |
| 4.76 | Comparison of footing settlement results obtained from | |
| | MLR, Genetic Programming and physical modeling | 208 |

| 4.77 | Comparison of wall displacement results obtained from | | |
|------|---|-----|--|
| | MLR, Genetic Programming and physical modeling | 209 | |

LIST OF SYMBOLS

| a | - | Area of cross section of standpipe tube |
|--------------------|---|---|
| А | - | Area of the Mold |
| В | - | Number of Blows |
| Cu | - | Uniformity Coefficient |
| Cc | - | Coefficient of gradation |
| c | - | Cohesion |
| c' | - | Effective Cohesion |
| C_m | - | Meniscus correction |
| D | - | Particle Diameter |
| D ₁₀ | - | Effective Particle Size |
| D ₅₀ | - | Average Particle Diameter |
| D, d | - | Diameter |
| e | - | Void Ratio |
| e ₀ | - | Initial Void Ratio |
| E | - | Modulus Elasticity |
| E ₅₀ | - | Modulus Elasticity of soil |
| E _(oed) | - | Elasticity Modulus of Oedometer |
| F | - | Force |
| ft | - | foot |
| g | - | Gravity = 9.81 m/s |
| G | - | Shear Modulus |
| Gs | - | Specific Gravity |
| h | - | Height |
| h_1 | - | Initial water level in standpipe tube |
| h_2 | - | Secondary water level in standpipe tube |
| Н | - | Height of Hammer |

| H _r | - | Effective Depth |
|---------------------------|---|--|
| Ι | - | Moment of inersia |
| $k, k_x k_y$ | - | Hydraulic Conductivity, Permeability |
| Κ | - | Percent finer than D |
| Κ | - | Bulk Modulus |
| K_0 | - | Lateral Earth Pressure Coefficient at Rest |
| Ka | - | Active Lateral Earth Pressure Coefficient |
| K _p | - | Pasive Lateral Earth Pressure Coefficient |
| L | - | Length of sample |
| l | - | Length |
| m | - | Mass |
| m | - | Dry mass of soil |
| mo | - | Initial Dry mass of sample |
| M_{PW} | - | Mass of Pycnometer and Water |
| M_{PW} | - | Mass of Pycnometer, Water and Material |
| M_s | - | Mass of oven dry soil |
| Ν | - | Number of Layer |
| n | - | Porosity |
| Р | - | Pressure |
| R _o ′ | - | Reading in Dispersant |
| R _d | - | Modified Reading |
| R_h | - | True Reading |
| R_{h}' | - | Hydrometer Reading |
| $\mathbf{S}_{\mathbf{v}}$ | - | Volume Strain |
| t | - | Time |
| Т | - | Tempreture |
| V | - | Volume of the Mold |
| Va | - | Volume of Air |
| $V_{\rm v}$ | - | Volume of Voids |
| V_{w} | - | Volume of Water |
| Vs | - | Volume of solid |
| W | - | Weight of Hammer |
| \mathbf{W}_{a} | - | Mass of Original sample |
| W_b | - | Mass of pycnometer filled with mixtures |

| W _c | - | Mass of pycnometer filled with alcohol |
|----------------------------------|---|--|
| x | - | Displacement |
| σ | - | Stress |
| $\sigma_{\rm h}$ | - | Horizontal Stress |
| $\sigma_{\rm v}$ | - | Vertical Stress |
| ε | - | Strain |
| $\epsilon_{\rm h}$ | - | Horizontal Strain |
| $\mathbf{\epsilon}_{\mathrm{v}}$ | - | Vertical Strain |
| η | - | Viscosity of Water at 24.5° C |
| φ | - | Internal Friction Angle |
| φ′ | - | Effective Internal Friction Angle |
| ϕ_{f} | - | Interparticle Friction Angle |
| ρ | - | Density |
| ρ_d | - | Dry Density |
| ρ_s | - | Particle Density |
| ρ_{sat} | - | Saturation Density |
| γ | - | Unit Weight |
| $\gamma_{(d)}$ | - | Dry Unit Weight |
| $\gamma_{(z.z.v)}$ | - | Zero Air Unit Weight |
| υ | - | Poisson's Ratio |
| ω | - | Moisture Content |
| $\omega_{(opt)}$ | - | Optimum Moisture Content |
| Ψ | - | Dilation Angle |
| | | |

LIST OF ABBREVIATIONS

| ASTM | - | American Society for Testing and Materials | |
|-----------------|---|---|--|
| BCR | - | Bearing Capacity Ratio | |
| BS | - | British Standard | |
| CCME | - | Canadian Council of Ministers of the Environment | |
| СН | - | Clay Soil with High Plasticity | |
| CL | - | Clay Soil with Low Plasticity | |
| CU | - | Consolidated Undrained Test | |
| EDX | - | Energy Dispersive X-ray | |
| FSR | - | Free Swell Ratio | |
| GP | - | Genetic Programming | |
| GRM | - | Gravel Rubber Mixture | |
| IPCs | - | Inorganic Polymer Concretes | |
| K100 | - | 100 Percent of Kaolin | |
| K80S | - | 80 Percent of Kaolin mixed with 20 Percent Sand | |
| K80G | - | 80 Percent of Kaolin mixed with 20 Percent Gravel | |
| K60S | - | 60 Percent of Kaolin mixed with 40 Percent Sand | |
| K60G | - | 60 Percent of Kaolin mixed with 40 Percent Gravel | |
| K40S | - | 40 Percent of Kaolin mixed with 60 Percent Sand | |
| K40G | - | 40 Percent of Kaolin mixed with 60 Percent Gravel | |
| K80-P20 | - | 80 % Kaolin mixed with 20 % Powdery TDA | |
| K60-P40 | - | 60 % Kaolin mixed with 40 % Powdery TDA | |
| K40-P60 | - | 40 % Kaolin mixed with 60 % Powdery TDA | |
| K80-SH20 | - | 80 % Kaolin mixed with 20 % Shredded TDA | |
| K60-SH40 | - | 60 % Kaolin mixed with 40 % Shredded TDA | |
| K40-SH60 | - | 40 % Kaolin mixed with 60 % Shredded TDA | |
| K80-G (1-4mm)20 | - | 80 % Kaolin mixed with 20 % Granular (1-4mm)TDA | |
| K60-G (1-4mm)40 | - | 60 % Kaolin mixed with 40 % Granular (1-4mm)TDA | |

| K40-G (1-4mm)60 | - | 40 % Kaolin mixed with 60 % Granular (1-4mm)TDA | | |
|-----------------|---|---|--|--|
| K80-G (5-8mm)20 | - | 80 % Kaolin mixed with 20 % Granular (5-8mm)TDA | | |
| K60-G (5-8mm)40 | - | 60 % Kaolin mixed with 40 % Granular (5-8mm)TDA | | |
| K40-G (5-8mm)60 | - | 40 % Kaolin mixed with 60 % Granular (5-8mm)TDA | | |
| kPa | - | Kilo Pascal | | |
| LL | - | Liquid Limit | | |
| LRFD | - | Load and Resistance Factor Design | | |
| LVDT | - | Linear Variable Differential Transducer | | |
| MC | - | Mohr-Coulomb | | |
| MDD | - | Maximum Dry Density | | |
| MEKP | - | Methyl Ethyl Ketone Peroxide | | |
| MH | - | Silt Soil with High Plasticity | | |
| ML | - | Silt Soil with Low Plasticity | | |
| MLR | - | Multiple Linear Regression | | |
| NCHRP | - | National Cooperative Highway Research Program | | |
| OMC | - | Optimum Moisture Content | | |
| PC | - | Polymer Concrete | | |
| pcf | - | Per Cubic Foot | | |
| pH | - | Power of Hydrogen | | |
| PI | - | Plasticity Index | | |
| PL | - | Plastic Limit | | |
| RMA | - | Rubber Manufacturers Association | | |
| SEM | - | Scanning Electron Microscope | | |
| SRM | - | Sand Rubber Mixture | | |
| TDA | - | Tire Derived Aggregate | | |
| TEM | - | Transmission Electron Microscope | | |
| UU | - | Unconsolidated Undrained Test | | |
| XRD | - | X-ray Diffraction | | |
| | | | | |

LIST OF APPENDICES

| APPENDI | X TITLE | PAGE |
|---------|-----------------------------------|------|
| A | Hydrometer test results | 240 |
| В | Compaction procedures | 241 |
| C | Micro photography of the mixtures | 243 |
| D | Physical modeling images | 250 |
| Е | PLAXIS outputs | 253 |
| F | Genetic programming output | 256 |

CHAPTER 1

INTRODUCTION

1.1 Background of the Study

Tire Derived Aggregate (TDA) has a low unit weight, is highly permeable and insulating. These characteristics make it an excellent choice as fill material for embankments build on unstable ground and also for landslide stabilization. TDA can be a proper backfill material for retaining walls and bridge abutments. Moreover, it may be use for insulation against frost penetration beneath roads, and as a drainage layers in landfills. In addition, the reuse of waste tires avoids problems associated with their disposal as well as stockpiling as scrap tires.

Literatures have shown that combining TDA and cohesive soil brings about additional benefits. For instance, adding TDA to cohesive soil reduces the need for sand mining and creates more sustainable construction. Kaolin-TDA mixtures are lightweight and create less horizontal pressure against the back of a retaining wall. In addition to that, the mixture may sustain and provide effective drainage, reduce water pressure, and insulation against frost damage to walls. Interestingly, TDA has been tested by the University of Maine and used in the construction of the Merrymeeting Bridge in Maine (Whetten *et al.*, 1997; Humphrey *et al.*, 1998; Tweedie *et al.*, 1998; Cosgrove and Humphrey, 1999). However, the use of shredded tires as a way to reduce pressure on rigid frame bridges remains largely untested.

In one study, series of triaxial tests were conducted on a mixture of tires chips and Ottawa sand (Ahmed, 1993). The results of this study demonstrated that apparent cohesion was increased while friction angle decreased when more tire chips were added to the mixture. According to Edil and Bosscher (1994) a formulation consisting of 25% chips (size 20 to 80mm) and 75% sand resulted in a mixture with superior shear strength at low normal stresses. More so, Lee *et al.* (1999) found that dilatancy behavior in TDA-sand was influenced by the stress strain relationship between pure sand and pure chips. In a similar study, Rao and Dutta (2006) discovered that the stress strain volumes of sand and tire chip mixtures exhibited various responses in triaxial compression tests. The results of their study also indicated that adding tire chips to sand mixtures may lead to a slight increase in frictional angles. The work of Dutta and Rao (2009) showed that triaxial testing on TDA mixed with soil was better able to absorb energy and decrease stress when the aspect ratio, chip content and confining pressure were increased.

Other researchers have also investigated on the performance of mixtures composed of sand and waste tires. For instance, Yoon *et al.* (2008) conducted plate load tests on sand reinforced with TDA. The sand samples used in Yoon's study had relative densities of 40%, 50%, and 70%. They found that the Bearing Capacity Ratio (*BCR*) of loose sand improved when the volume of TDA was added. However, they also found that *BCR* decreased when density increased. According to the study of Tafreshi and Norouzi (2012), the results of plate load tests on square footing resting on soil mixed with TDA was also significantly improved. Overall, direct shear tests on material containing scrap tire chips demonstrate that the addition of tire chips increases shear strength (Naval *et al.*, 2013).

This study investigates Geotechnical properties of Kaolin-TDA mixture as backfill material. Also, it compares the effect of various TDA shapes and percentages in Kaolin. Physical modeling of the retaining wall is conducted in the laboratory using normal and modified soil as backfill. The result of physical modeling was verified by the numerical modeling (PLAXIS 2D) for retaining wall using the modified soil as backfill. Tables and charts of modified soil properties are established that may be used in retaining wall design.

1.2 Problem Statement

Backfill material with poor permeability used in reinforced structure is of great concern as it has resulted in maintenance issues and structural failures (Mitchell and Zornberg, 1995). Often, materials with low permeability are used in inexpensive wall systems. These systems are vulnerable to deformation and may fail. If fine materials such as silt or clay are used as backfill, then any water pressure in the regions in front of, behind, or underneath the backfilled area must be collected and removed through proper filtration and drainage methods. Moreover, the region above the backfilled area must be waterproofed using geomembrane or a geosynthetic clay liner. The process is necessary so as to prevent surface water from entering the backfilled area (Koerner and Soong, 2001). On the other hand, clay soils have a plastic behavior. Therefore, combining the elastic properties of shredded tires with the plastic properties of clay soil in a single mixture creates a perfectly Elastoplasticity backfill material for retaining walls.

Moreover, reutilizing solid wastes like TDA with cohesive soil as backfill material would reduce sand mining and preserve the environment to achieve sustainable construction. The proper disposal of waste tires is a global issue as an environmental problem as the secondary problem statement of this study. Scrap tires cannot be easily disposed of by burning because they release pollutants into the water and air when they are set alight. When disposed of in landfills, waste tires float to the surface and compromise the surface of the landfill providing a way for rodents, insects and water to enter the landfill (Figure 1.1). Stockpile of scrap tires represents a threat to environment and public health. Fortunately, scrap tires can be reused in several ways such as creating tire chips for lightweight fill (Humphrey and Manion, 1992; Foose *et al.*, 1996; Yoon *et al.*, 2008). In this study, the benefits of Kaolin-TDA mixtures as a backfill material for retaining walls were examined.



Figure 1.1: Stockpiling and burning the waste tire (Environmental Engineering and Contracting, 2002)

1.3 Objectives of the Study

The aim of this study is to evaluate the performance of Kaolin-TDA mixture as a backfill material on reinforced polymer concrete retaining wall. The following objectives are identified in order to achieve this aim:

- i. To identify material characteristics of Kaolin-TDA mixture including different shapes, sizes and amount of the TDA.
- To determine the performance of the polymer concrete retaining wall using different backfill material prepared from the Kaolin-TDA mixtures based on physical modeling.
- iii. To verify the physical modeling test results with numerical simulation using PLAXIS 2D, Genetic Programming (GP) and developing an equation based on Multiple Linear Regressions.

1.4 Scope of the Study

The study investigates the geotechnical properties of Kaolin pre-admixed with the TDA, for the purpose of use as a backfill material for polymer concrete retaining wall. The scopes of the study are as follows:

- i. Test samples were mixed at optimum moisture content obtained from compaction test
- ii. The basic tests were performed based on the ASTM (1992) and BS 1377-(1990).
- iii. Physical modeling tests of the retaining wall were performed in the laboratory using Kaolin and Kaolin-TDA mixtures as backfill material.
- iv. The results of physical modeling were verified using commercial numerical modeling program PLAXIS 2-D (based on Mohr-Coulomb model), Genetic Programming and Multiple Linear Regression while, active force was considered for retaining wall and backfill.
- v. Physical modeling tests were conducted in dry condition because the main reason for conducting the physical modeling was investigating the use of lightweight material as backfill. However, permeability tests were conducted to show the increase in permeability for different mixtures.
- vi. In this study the mixing ratio of 0:100, 20:80, 40:60 and 60:40 were chosen to conduct the laboratory tests, physical modeling tests and numerical modeling.

1.5 Significance of the Study

Consideration of the performance of Kaolin-TDA mixture as backfill material on the prepared polymer concrete retaining wall is the most important purpose of this investigation. TDA is a light weight material while the use of this waste in geotechnical engineering may decrease weight of structures. Hence, Kaolin-TDA mixtures prevent overturning in retaining wall because of low horizontal pressure. In addition, removal of waste tire as integral parts of solid waste of a nation is a good solution for saving the environment. The removal can result in because of preventing of burning and stockpiling the tire while, thus adding economic value to it.

1.6 Thesis Organization

The thesis consists of five chapters. Chapter 1 presents the background, problem statements, objectives, scope, and significance of this research. Chapter 2 reviews previous studies related to this study. Topics such as clay soil properties, shredded tire properties, soil-shredded tire applications, fill materials, recycled materials, backfill material and retaining wall, polymer concrete, physical modeling and numerical modeling were discussed. Chapter 3 describes the research methodology including various laboratory tests carried out. The tests included compaction test, permeability test and triaxial test on Kaolin mixed with different percentages and shapes of Tire Derived Aggregate (TDA). Besides physical modeling, equipment and procedures of the loading were illustrated. The design of the box and steel strip footing dimensions and instrumentation were addressed. In Chapter 4, laboratory test results and the results from the physical modeling tests and numerical simulations are presented and discussed. The results cover several issues such as the settlement of the improved ground, retaining wall displacement at failure and backfill failure modes. The results from triaxial tests on the backfill materials were also presented. The effects of shape of TDA, percentage of TDA and arrangement of the material particles in mixtures were presented. Furthermore, comparison between the Load-displacement of experimental tests with numerical are performed. Consequently, the equations were created for predicting the footing settlement and wall displacement. Finally, Chapter 5 lists the conclusions and recommendations for future research on Kaolin-TDA mixture as a backfill material.

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