WARPING CONSTANT OF STEEL SECTION

YONG JITT CHING

A thesis submitted in fulfillment of the requirements for the award of the degree of Master of Engineering (Civil-Structure)

Faculty of Civil Engineering
Universiti of Technology Malaysia

OCTOBER, 2003

UNIVERSITI TEKNOLOGI MALAYSIA

BORANG PENGESAHAN STATUS TESIS JUDUL: WARPING CONSTANT OF STEEL SECTION SESI PENGAJIAN: 2002/2003 Sava YONG JITT CHING (HURUF BESAR) mengaku membenarkan tesis (PSM/Sarjana/Doktor Falsafah)* ini disimpan di Perpustakaan Universiti Teknologi Malaysia dengan syarat-syarat kegunaan seperti berikut: Tesis adalah hakmilik Universiti Teknologi Malaysia. 2. Perpustakaan Universiti Teknologi Malaysia dibenarkan membuat salinan untuk tujuan pengajian sahaja. 3. Perpustakaan dibenarkan membuat salinan tesis ini sebagai bahan pertukaran antara institusi pengajian tinggi. **Sila tandakan () (Mengandungi maklumat yang berdarjah keselamatan atau **SULIT** kepentingan Malaysia seperti yang termaktub di dalam **AKTA RAHSIA RASMI 1972)** (Mengandungi maklumat TERHAD yang telah ditentukan TERHAD oleh organisasi/badan di mana penyelidikan dijalankan) TIDAK TERHAD Disahkan oleh (TANDATANGAN PENULIS) (TANDATANGAN PENYELIA) Alamat Tetap: 16, JALAN EMAS 21, P.M. IR. DR. MOHD. HANIM OSMAN TAMAN SRI PUTRI, 81300 SKUDAI, JOHOR Nama Penvelia Tarikh: 25.10.2003 Tarikh:

CATATAN: * Poto

* Potong yang tidak berkenaan.

** Jika tesis ini SULIT atau TERHAD, sila lampirkan surat daripada pihak berkuasa/organisasi berkenaan dengan menyatakan sekali sebab dan tempoh tesis ini perlu dikelaskan sebagai SULIT atau TERHAD.

Tesis dimaksudkan sebagai tesis bagi Ijazah Doktor Falsafah dan Sarjana secara penyelidikan, atau disertasi bagi pengajian secara kerja kursus dan penyelidikan, atau Laporan Projek Sarjana Muda (PSM).

"Saya akui bahawa saya telah membaca karya ini dan pada pandangan saya, karya ini adalah memadai dari segi skop dan kualiti untuk tujuan penganugerahan ijazah Sarjana Kejuruteraan Awam - Struktur".

Tandatangan :

: PROF MADYA.DR.Ir.MOHD HANIM OSMAN
: ユノルノレクロ3 Nama Penyelia

Tarikh

"I declare that this project report is the results of my own research except as cited in references. This project report has not been accepted for any degree and is not concurrently submitted in candidature of any degree."

Signature

Name of Candidate : YONG JITT CHING

Date

: 25 October 2003

...... For The God, Creator of the world.....

ACKNOELEDGEMENTS

Thanks for God who gives the wisdom and always lead the way till the completion of this project report.

The author whishes to express great thankful to his supervisor Profesor Madya Ir. Dr. Mohd Hanim Osman for his guidance and advices that paved the way to attain a smooth finishing of the study.

Also, the author is thankful to Tee Bun Ping and Goh Lyn Dee for their encouragement and help during all the way. Appreciation also acknowledged to all of the author's friends who were directly and indirectly involved in the completion of this project report.

Finally, for the author's lovely family that always giving supports and been a source of inspiration in achieving success up to this level.

ABSTRACT

This project report is aimed to study the warping behaviour of steel section. The main objective of this study is to obtain relationship of warping displacement and warping constant. In BS5950, there is already a basic derivation of warping constant with its relationship with the geometry of cross section. Somehow, this formula is unsuitable to be applied for some section member such as corrugated I-beam. Modifying the formula needs a lot of study and research; it is not economic both in cost and time. To overcome this, there comes a suggestion. There might be a relationship between warping displacements of member with different cross section as they are using the same material. By developing a relationship between warping constant and warping displacement, it is easy to find the warping constant of abnormal section with condition that we know its warping displacement. But, there is no reference available for the relationship governing the warping constant and warping displacement. In order to achieve the objectives, finite element analysis is carried out. Each parameter that affects the warping displacement and warping constant is under the consideration and been studied.

ABSTRAK

Matlamat utama projek sarjana ini adalah untuk mengkaji kelakukan keratan besi. Objektif utama projek sarjana ini adalah untuk mengkaji hubungan antara nilai lendingan dengan kelakukan lendingan. Dalam BS5950, sudah terdapat formula mengenai nilai lendingan. Tetapi, formula ini bukan sesuai digunakan untuk semua keadaan seperti keratan web trapezoid. Jadi, untuk mendapatkan nilai lendingan, pengubahsuaian harus dilakukan. Pengubahsuaian ini adalah memakan masa dan kos. Untuk mengatasi masalah ini, hubungan antara nilai lendingan dan kelakukan lendingan harus dikaji. Dengan adanya hubungan sebegini, ini boleh menyenangkan kerja mencari nilai lendingan sekiranya kelakukan lendingan boleh didapati. Namun, setakat kini, belum dapat sebarang rujukan mengenai hubungan antara nilai lendingan dan kelakukan lendigan. Untuk mencapai objektif projek sarjana ini, finite element analisis digunakan. Semua parameter mempengaruhi nilai lendingan dan kelakukan lendingan telah dikaji.

LIST OF CONTENT

CHAPTER	CONTENT	PAGE
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDGEMENTS	iv
	ABSTRACT	\mathbf{v}
	ABSTRAK	vi
	LIST OF CONTENT	vii
	LIST OF TABLE	x
	LIST OF FIGURE	xi
	NOTATION	xiii

CHAPTER I INTRODUCTION

1.1	Introduction
1.2	Background of study
1.3	Objectives of Study
1.4	Scope of Study

CHAPTER II	LITE	ERATURE	REVIEW	
	2.1	Introduc	etion	10
	2.2	Basic th	eory	12
	:	2.2.1	Uniform torsion of thin walled open section	12
	2.3	Derivati	on of warping constant equation	16
CHAPTER III	FINI	TE ELEMI	ENT ANALYSIS	
	3.1	Introduc	tion	31
	3.2	Modelin	g	32
		3.2.1	Creating model	32
		3.2.2	Meshing	36
		3.2.3	Material Property	38
		3.2.4	Support Condition	39
		3.2.5	Loading	41
	3.3	Paramet	ric Study	44
	3.4	Getting l	Result	46
CHAPTER IV	RESU	JLTS OF T	HE FINITE ELEMENT ANALYSIS	•
	4.1	Support	Reaction	51
	4.2	Warping	Displacement	57
	4.3	Equivale	nt Slenderness	64
CHAPTER V	DISC	USSION		
	5.1	Warping	Displacement	69
	5.2	Equivale	nt Slenderness	75

CHAPTER VI CONCLUSIONS AND SUGGESTIONS

6.1	Conclusion	76
6.2	Suggestion	77

LIST OF TABLES

NO.	OF TABLE TITLE	PAGE
1.1	The warping constant of various sections and the corresponding	
	lateral torsional buckling strength of 15 m strength.	5
3.1	Data of model created with different number of nodes and elemen	ts. 36
4.1	Displacement in x, y and z direction for nodes in cross section A-	A . 50
4.2	Forces of nodes in x , y and z direction.	56
4.3	Warping constant and warping displacement with Increasing	
	of depth of beam	58
4.4	Warping constant and warping displacement with increasing	
	of width of flange.	60
4.5	Warping constant and warping displacement with increasing	
	of thickness of flange.	62
4.6	Warping constant and warping displacement with increasing	
	of thickness of web.	63
4.7	Equilibrium slenderness and warping constant with increase	
	of thickness of flange.	65
4.8	Equilibrium slenderness and warping constant with increase	
	of thickness of web.	65
1.9	Equilibrium slenderness and warping constant with increase	
	of depth of beam.	66
1.10	Equilibrium slenderness and warping constant with increase	
	of width of flange.	66
5.1	Influence of each parameter on warping displacement.	71
5.2	Warping constant with increment of 0.66mm in each parameter.	74

LIST OF FIGURES

NO.	OF FIGURE TITLE	PAGE
1.1	Dimension of I-beam.	4
1.2	Relationship of warping constant with equivalent slenderness.	6
1.3	Relationship of warping constant with lateral torsional buckling strength	ı. 6
1.4	Non-uniformed cross section.	7
2.1	Simply supported I-beam subjected to twisting moment.	25
2.2	Deformed shape of beam.	25
2.3	Saint Venant shear stress distributions.	26
2.4	Dimension of I-beam.	26
2.5	Displacement of flange.	27
2.6	Warping at the root of the cantilever.	27
2.7	Rotation about the z-axis.	28
2.8	Middle line of an arbitrary section.	28
2.9	Movement of center of rotation from A to B.	29
2.10	An arbitrary bar distributed with normal stresses.	29
2.11	An element that is cut out from wall of the bar in Figure 2.10.	30
3.1	Cross section of I-beam	32
3.2	Creating model	33-35
3.3	Graph of nodal displacement against number of element	37
3.4	Model after meshing.	38
3.5	Support condition of model with one end rigidly supported.	40
3.6	Loading pattern to create pure torsion.	41-42
3.7	The loading applied to the model.	43-44

3.8	Dimension for I-beam.	45
4.1	Node where result of nodal displacement is taken.	47
4.2	Cross section A-A	48
4.3	Result of deform shape of Model.	50
4.4	Contour of warping displacement.	51
4.5	Support reaction of model.	52-53
4.6	Surface cross section of support.	54
4.7	Plots of reaction distribution.	56
4.8	Dimension of model.	57
4.9	Relationship of warping constant and displacement with	
	distance between shear center of flanges.	58
4.10	Dimension of model.	59
4.11	Relationship of warping constant and displacement with width of flange.	60
4.12	Dimension of model.	61
4.13	Relationship of warping constant and displacement with	
	thickness of flange.	62
4.14	Dimension of model.	63
4.15	Relationship of displacement with thickness of web.	64
4.16	Buckling parameter with increase of depth of beam.	67
4.17	Buckling parameter with increase of width of flange.	67
4.18	Buckling parameter with increase of thickness of web.	68
4.19	Buckling parameter with increase of thickness of flange.	68
5.1	Combination of relationship between warping constant and	
	displacement with parameters.	70

NOTATIONS

b - Width of flange

D - Depth of beam

 t_w - Thickness of web

t - Thickness of flange

 H_s - Distance between shear center of flanges

E - Young's modulus

H - Warping constant

J - Torsional constant

 λ - Slenderness of a beam

u - Buckling parameter

v - Slenderness factor

x - Torsional index

n - Slenderness of correction factor

 λ_{LT} - Equivalent slenderness

 γ - A factor

M_b - Buckling resistance moment

 p_b - Bending strength

 θ - Angle of twist

 p_y - Design strength

L - Length of section

T - Torque

G - Shear modulus of material

 S_x - Plastic section modulus

 I_x - Second moment of area about the x-axis

 I_y - Moment of inertia of a section about y-axis

 M_{cr} - Critical moment

 V_f - Shear force across the width of flange

M - Equivalent uniform moment

m - Equivalent uniform moment's factor

 L_E - Effective length

 r_y - Radius of gyration of the minor axis

A - Cross-sectional area of a member

 T_w - Warping restraint torsion

 ω - Warping displacement

 Δx - Displacement in the x-direction

 ω_s - Warping function

 $\overline{\omega}_s$ - Average value of ω_s over the entire cross section

 ε_z - Axial strain

 $\sigma_{\rm s}$ - Normal stress

au - Shear stress

i - Initial point of an element

 H_I - Warping rigidity

j - End point of an element

n - Refers to nth element

 L_{ij} - Length of the element ij

 ρ_o - Distance from the shear center to an element under consideration

 ω_o - Unit warping with respect to the shear center

 W_n - Normalized unit warping of the cross section

CHAPTER I INTRODUCTION

1.1 Introduction

The use of steel structures has become increasingly popular in different fields of building technology including wall structures, framing systems, trusses and shielding materials.

For years, numerous researches had been conducted to investigate various strength properties of steel sections. One of the studies is on lateral torsional buckling strength of beams. In the study to develop design formulae and related curves, one of the important element is the torsional behaviour of the section. For sections with uniform web throughout the length, the torsional properties i.e, torsional constant and warping constant were defined in Appendix B BS5950: Part 1: 1990.

However, the empirical based design curve in BS5950 is represented by modified form of the Perry equation using a beam slenderness parameter λ_{LT} . The relationship between slenderness, warping constant and their influence in designing a steel member will be discussed in details in this study. The purpose of this study is to examine the warping behaviour of the beams with uniformed web profile i.e, I-beam subjected to pure torsion and lateral torsional buckling moment using finite element modeling.

1.2 Background of Study

Warping constant is an very important parameter in steel designing i.e, in lateral torsional buckling and moment resistance for beams. From BS5950, the buckling resistance moment is given as,

$$M_b = p_b S_x \tag{1.1}$$

where p_b is the bending strength.

 S_x is the plastic section modulus.

The bending strength, p_b is related with the equivalent slenderness, λ_{LT} , defined as,

$$\lambda_{LT} = nuv\lambda \tag{1.2}$$

where n is slenderness correction factor,

u is the buckling parameter,

v is the slenderness factor,

 λ is the slenderness of the beam, defined as $\lambda = \frac{L_E}{r_V}$ (1.3)

 L_E is the effective length of beam

 r_y is the radius of gyration about minor axis

To acquire the slenderness factor, it is needed to know the torsional index, x. Both buckling parameter, u and torsional index, x are dependant on the geometry of the cross section. They are related to warping constant, H, as,

$$u = \left(\frac{I_y S_x^2 \gamma}{A^2 H}\right)^{0.25} \quad \text{and} \quad x = 1.132 \left(\frac{AH}{I_y J}\right)^{0.5}$$
 (1.4)

where y is a factor, defined as

$$\gamma = 1 - \frac{I_y}{I_x} \tag{1.5}$$

 I_x is the second moment area about the major axis

 I_y is the second moment area about the minor axis

H is the warping constant, defined as

$$H = \frac{h_s^2 t_1 t_2 b_1^3 b_2^3}{12 (t_1 b_1^3 + t_2 b_2^3)}$$
 (Refer Figure 1.1)

J is the torsional constant, defined as

$$J = \frac{1}{3} (t_1^3 b_1 + t_2^3 b_2 + t_w^3 h_w) \quad (\text{Refer Figure 1.1})$$
 (1.7)

 t_1, t_2 are the flange thickness

 b_1, b_2 are the flange width

 t_w is the web thickness

 h_w is the web depth

 h_s is the distance between shear center of flanges

 S_x is the plastic modulus about the major axis

A is the area of cross section

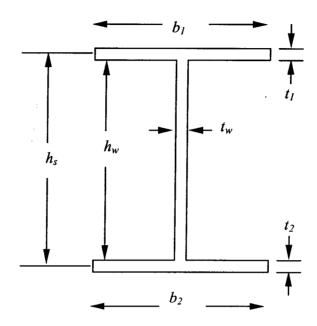


Figure 1.1: Dimension of I-beam.

From these derivations, small changes in warping constant will influence the design of steel structures. By using different cross section, the relationship of equivalent slenderness, λ_{LT} and warping constant, H and influence of warping constant, H in moment resistance during designing will be studied. As an example, take a beam with effective length of 15 m, subjected to a uniformly distributed load

The equivalent slenderness can be obtained from equation 1.2. Subsequently, p_b is obtained from BS5950. Table 1.1 shows the parameters for the calculation of the lateral torsional buckling strength. The warping constant is shown in the table for comparison.

Serial	A.M.	"u	x	****	** r y **	· . A	ax	· Alt'	H:	p_b .
Size		16.7° 545			(cm)			S.\$71,202	(dm ⁶)	(N/mm²)
254 x	1.0	0.85	36	0.96	2.1	73.171	2.038	59.988	0.018	213.03
102 x										
22										
254 x	1.0	0.86	31	0.95	2.1	70.093	2.232	57.230	0.023	220.2
102 x		,								
25										
254 x	1.0	0.87	28	0.93	2.2	67.568	2.457	54.857	0.028	226.34
102 x										
28										
254 x	1.0	0.88	29	0.97	3.4	44.776	1.523	38.177	0.066	266
146 x										
31										
254 x	1.0	0.89	24	0.97	3.5	43.228	1.779	37.084	0.086	268.42
146 x										
37										
254 x	1.0	0.89	21	0.96	3.5	42.735	2.025	36.472	0.103	269.77
146 x										
43		-								

Table 1.1: the warping constant of various sections and the corresponding lateral torsional buckling strength of 15 m strength.

From Table 1.1, it is seen that with the increase in warping constant, equivalent slenderness will decrease as shown in Figure 1.2. And, the influence of warping constant on the lateral torsional buckling resistance is shown in Figure 1.3. Lateral torsional buckling resistance will increase with the increase of warping constant.

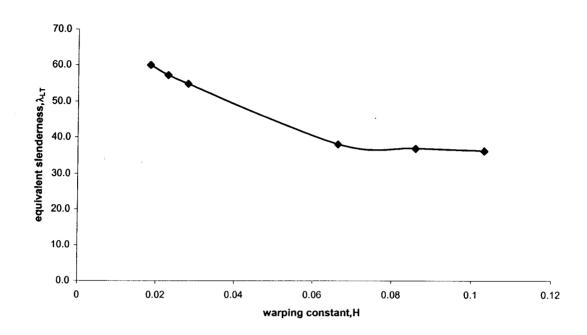


Figure 1.2: Relationship of warping constant with equivalent slenderness.

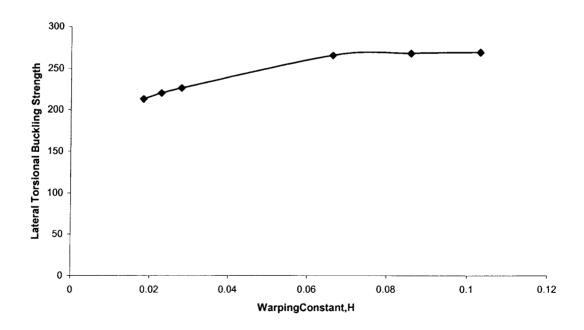


Figure 1.3: Relationship of warping constant with lateral torsional buckling strength.

Thus, it is significant to study in more details about the warping constant. Warping constant, *H* is important both for safety of structure and to increase efficiency in term of the materials used.

With the increase popularity of the use in steel in construction, various sizes and shapes of members are being used. Taking a short brief of our previous research and development (R&D) of material properties, we find that our R&D is much far behind needs of today's construction field. Most of the time, we found difficulties to derive the warping constant of some non-uniformed cross section as shown in Figure 1.4.

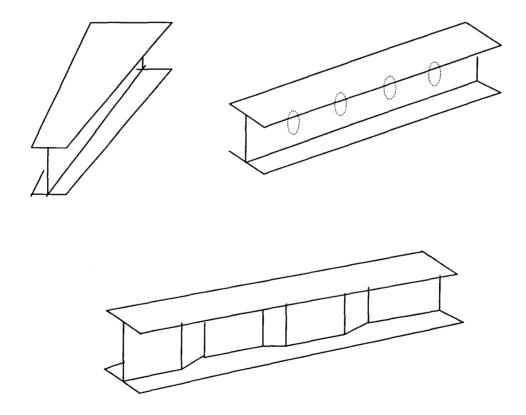


Figure 1.4: Non-uniformed cross section.

In BS5950, there is already a basic derivation of warping constant with its relationship with the geometry of cross section. Somehow, this formula is unsuitable to be applied for some section member such as corrugated I-beam. Modifying the formula

needs a lot of study and research; it is not economic both in cost and time. To overcome this, there comes a suggestion. There might be a relationship between warping displacements of member with different cross section as they are using the same material. By developing a relationship between warping constant and warping displacement, it is easy to find the warping constant of abnormal section with condition that we know its warping displacement. But, there is no reference available for the relationship governing the warping constant and warping displacement. Hence, this project report is carried out.

1.3 Objectives of Study

The objectives of this study are:

- 1. The use of finite element method in the study of warping behaviour.
- 2. To examine the relationship between warping constant and warping displacement of the beams with uniformed web profile subjected to pure torsion.

1.4 Scope of Study

The scope of study in this project report can be divided as,

1. Basic derivation of warping constant and warping displacement.

REFFERENCES

- Vlasov.V. Z. (1961). "Thin Walled Elastic Beam." Israel Program for Scientific Translations Ltd., Jerusalem.
- 2. Umansky, A. A. (1939). "Bending and Torsion of Thin Walled Aircraft Structures." Oborongiz, Moscow.
- 3. Nowinski, J. (1959). "Theory of Thin Walled bars." Appl. Mech Rev., Vol 12 No 4, 219-227.
- 4. Gjelsvik, A. (1981). "The Theory of Thin Walled Bars." John Willey and Sons Inc., New York.
- 5. Koo K. K. and Cheung Y. K. (1989). "Mixed variation formulation for Thin Walled Beams with Shear lag." ASCE, Vol 115, No 10, 2271-2286.
- 6. Prokic, A. (1996). "New Warping Functions for Thin Walled Beams." ASCE Journal of Structural Engineering, Vol 122, No 12, 1437-1442.
- 7. Timoshenko, S. (1934). "Theory of Elasticity." McGraw-Hill Book Co., New York.
- 8. Hermann, L. R. (1965). "Elastic Torsional Analysis of Ireggular Shape." ASCE, Vol 91, No 6, 11-19.

- 9. Boresi, A. P. and Lynn, P. P. (1974). "Elasticity in Engineering Mechanics." Prentice-Hall, New Jersey.
- Reisner, E. (1979). "Some Considerations on the Problem of Torsion and Flexure of Prismatical Beams." Inst. J. Solids and Struct., Vol 15, No 5, 385-392.
- 11. Reisner, E. (1983). "Futher Considerations on the Problem of Torsion and Flexure of Prismatical Beams." Inst. J. Solids and Struct., Vol 19, No 5, 385-392.
- 12. Reisner, E. (1992). "A note on the Problem of Torsion and Flexure of Prismatical Beams." Inst. J. Solids and Struct., Vol 30, No 4, 455-462.
- 13. Kirby, P.A and Nethercot, D.A. (1979). "Design of Structural Stability." John Wiley & Sons, New York.
- 14. Chen, W.F. and Lui, E.M. (1987). "Structural Stability (Theory and Implemention)." Elsevier Science Publishing, New York.
- 15. Heins, C.P. (1975). "Bending and Torsional Design in Structural Members." D.C. Health and Company.
- 16. Yu. W.W. (2000). "Cold-Formed Steel Design, 3rd Ed." John Willey & Sons, New York.
- 17. Stephen P. Timoshenko and James M.Gere (1968). "Theory of Elastic Stability."