

## EFFECT OF DEPTH PLACEMENT OF GEOCELL REINFORCEMENT IN SAND; A REVIEW

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**Abstract:** In general, the tensile strength of the soil is poor. For this reason, the soil will need to be strengthened. The main objective of strengthening the soil mass is to improve stability, increase bearing capacity and reduce settlements and lateral deformation. There are several methods for improving the soil. One of the approaches is the use of geosynthetic materials. Geosynthetic is a well-known technique in soil reinforcement. The use of geosynthetic three dimensions can significantly improve the soil performance and reduce costs in comparison with conventional designs. In this paper, a review of experimental test carried out by different researchers in optimum depth of geocell in the sand had been made. Test results indicated that the inclusion of reinforcement in optimum depth of sand decreased settlements and leading to an economic design of the footings.

**Keywords:** *Bearing Capacity; Geocell; soil reinforcement; Sand.*

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### 1. Introduction

In recent decades, due to its economy, ease of construction and performance, reinforced soil has been widely exploited in geotechnical engineering applications such as in the construction of roads, railway embankments, retaining walls, stabilization of slopes and improvement of soft ground (Moghaddas Tafreshi et al., 2012). Soil reinforcement is determined as a process for improving the engineering characteristics of soil. The soil can be considered as four basic type combinations: gravel, sand, clay and silt. The soil usually has the characteristics of low shear and tensile strength and is highly dependent on environmental conditions (Ling et al. 2003).

The main objective of the soil reinforcement is to improve stability, increase capacity and reduce settlements and lateral deformation (Yarbasi et al.2007, Hejazi et al. 2012). Over the past 40 years, innovative approaches to improving soil have been extended to solve soil

capacity of sand have been clearly demonstrated by several investigators. The most recent advancement of reinforced soil is to provide three-dimensional confinement to the soil by using geocells (Dash *et al.*, 2001).

Shallow foundations are widely used in transmitting loads from the superstructure to the supporting soils. After the foundation is constructed, the soil is permanently loaded by both the gravity loads and the live loads of the superstructure. (El Sawwaf and Nazir, 2010). In this paper, an overview with the experimental test on the effect of optimum depth of geocell in sand on bearing capacity and settlement of soil will be discussed.

## **2. Geosynthetic reinforcement**

The types of soil improvement methods, including grouting, vertical drains, soil replacement, complete, piling and geosynthetic reinforcement has developed to solve the problems (Liu *et al.*, 2008, Rowe and Taechakumthorn, 2008). Among these methods, geosynthetic reinforcement has been used. (Rowe and Li, 2005).

Li *et al.* (2012) reported the work in this field of research. Geosynthetic produced from polymers is widely used to reinforce soils. The reinforced soil structures are under to stress or creep. (Leshchinsky *et al.*, 2010, Liu *et al.*, 2009). Geogrid is used in layers with aggregate fills or other suitable soils to create a strong layer. So the bearing capacity of soil under the load of the foundation will be improved. Many experiments have shown sand usually has used as backfill material. (Rowe and Taechakumthorn, 2011, Karimpour and Lade and Yeo and Hsuan, Kongkitkul *et al.*, 2010, Lade *et al.*, 2009, Kim *et al.*, Lade, Pham Van Bang *et al.*, 2007) and geogrid reinforcement material (Bathurst *et al.*, 2009, Jones and Clarke, 2007, Shinoda and Bathurst, 2004, Kuwano and Jardine, 2002, Li and Rowe, 2001, Perkins, 2000, Sawicki, 1998).

## **3. Geocell**

Geocell is honeycomb three-dimensional cell structures that provided containment of compacted fill soils. Decreased the lateral movement of the soil particles and form a mat or rigid for the distribution of loads applied to a wider area slab movement. Geocells were used in the construction of canals, embankments, retaining walls, railways and roads (Dash *et al.*, 2003).

New types geocell are made of a new polymer structure characterized by low temperature flexibility similar to high density polyethylene (HDPE). (Pokharel, 2010, Yang, 2010). The base layer reinforced geocell mattress In road construction, acts as a rigid slab or a mattress for distribution the traffic load vertically on a broader subgrade. Therefore, the vertical forces applied to the subgrade was decreased and the capacity was increased. (Marto *et al.*, 2013). Pokharel *et al.* (2010) stated that the concept of lateral confinement cell structures dating back to 1970. Geocells come in different shapes and sizes. In Figure 2. As is shown in this figure, the typical configurations of geocell reinforcing elements: (1) Vertical perforated elements prepared as a cellular, honeycomb-like structure. (2) Vertical geogrid elements prepared by cutting geogrids. This type of geocell is hand made from geogrid chevron or diamond pattern.

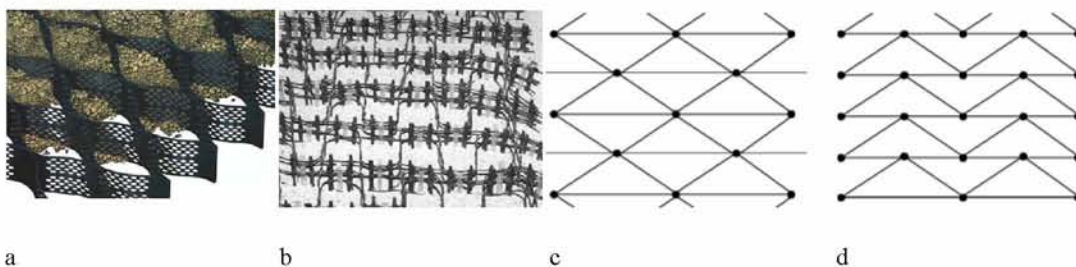


Figure 2: The typical configurations of geocell reinforcement elements. a: Perforated geocell (Bathurst and Jarrett, 1998). b: Handmade geocell (Dash *et al.*, 2003). c: Handmade geocell diamond pattern (Dash *et al.*, 2003). d: Handmade geocell chevron pattern (Dash *et al.*, 2003).

#### 4. Reinforcement mechanisms

As compared with the unreinforced base, the geocell-reinforced base can provide lateral and vertical confinement, tensioned membrane effect, and wider stress distribution. According to

Giroud and Noiray, (1981) lateral confinement, increased bearing capacity, and tensioned membrane effect was identified as the major reinforcement mechanisms for geotextile reinforcement. Boushehrian *et al.* (2011) studied experimentally and numerically the effect of the depth of the first reinforcement layer ( $u$ ), spacing between reinforcements ( $h$ ), and reinforcement stiffness on the bearing capacity of circular and ring foundations of sand. Using footing width,  $B$ , Chung and Cascante (2006) have shown that a zone between  $0.3B$  and  $0.5B$  is identified to maximize the benefits of soil reinforcement. They noticed that the accommodation of reinforcements within one footing width below the foundation can lead to an increase in bearing capacity ratio (BCR) and the low strain stiffness of the reinforced system. This increase is due to the transferring of the foundation loading to deeper soil layers, as well as a reduction in the stresses and strains underneath the foundation. Mosallanezhad *et al.* (2008) dealt with the influence of a new generation of reinforcement (named as Grid-Anchor) on the increase of the square foundation bearing capacity. It was found that the critical value of  $u/B$  was equal to 0.25. They also showed that BCR for this system was greater than ordinary geogrid. Shin *et al.* (2008) showed that within the soil-reinforcement system the shear modulus of soil increases with the number of layers in depth under cyclic loading. The geometry of the test configurations for the geocell considered in these investigations is shown in Figure 3.

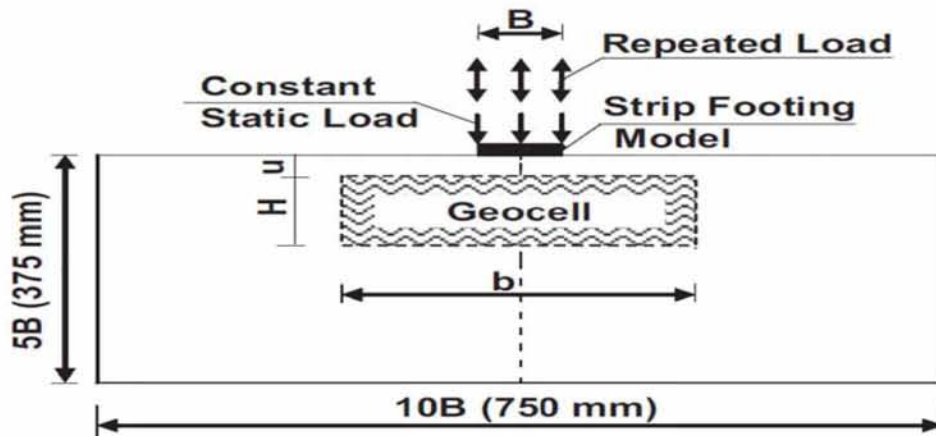


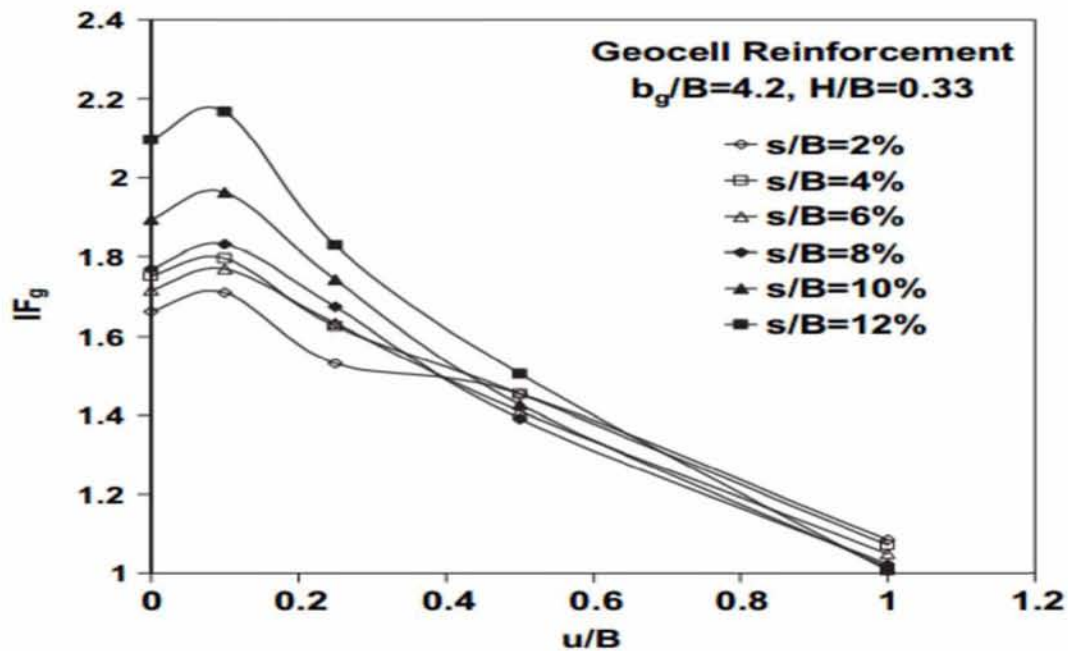
Figure 3: Geometry of the Geocell- reinforced foundation bed (Moghaddas & Dawson, 2012).

### **5. Laboratory tests conducted on geocell reinforced soil**

Researchers (Moghaddas and Dawson, 2012, Sitharam and Sireesh, 2012, Ling Zhang *et al.*, 2010, Madhavi *et al.*, 2009, Dash *et al.*, 2001) mentioned the load spreading action of the reinforced layer and a subsequent reduction in the vertical stress in the layer underlying the geocell layer. They showed that there is an increased performance on the footing over a buried geocell layer even with the geocell mattress width equal to the width of the footing. The geocell mattress transfers the footing load to a deeper depth through the geocell layer. An increase in the bearing capacity of the geocell mattress with an increase in the ratio of cell height to cell width was observed by Rea and Mitchell (1978) and Mhaikar and Mandal (1992). Dash *et al.* (2001) found that the load carrying capacity of the foundation bed increased with a rise in the cell height to diameter ratio, up to a ratio of 1.67, beyond which further improvements were marginal. The optimum ratio, reported by Rea and Mitchell (1978) was around 2.25. Krishnaswamy *et al.* (2000) reported an optimum ratio of about 1 for geocell supported embankments constructed over soft clays. Table 1 summarizes several previous research about the effect of geocell optimum parameters of soil reinforcement illustrated.

Several researchers have found an improvement in the load bearing capacity of the foundation with an increase in the mattress thickness, up to a geocell height of twice the width of the footing. Figure 4. Shows the corresponding improvement in bearing pressure factor (IF) with  $u/B$  at different values of settlement. Figure 5. Shows the Variation of improvement factors with settlement for different depths of placement of geocell. (Moghaddas and Dawson, 2012, Dash *et al.*, 2001). In Figure 5 shown the influence of the depth of placement of geocell layer (defined by  $u/B$  ratio) on the bearing capacity improvement factor (If). This is reflected in the reduction of If for

higher  $u/B$  ratios. These results suggest that to get maximum benefit, the top of the geocell mattress should be at a depth of  $0.1B$  from the bottom of the footing. Up to  $u/B$  ratio of  $0.25$ , the footings have not shown evidence of failure even at large settlements. When  $u/B$  was  $0.50$ , the footing had an initial failure at a settlement of about  $0.2B$  and later started taking higher loads and finally reached its ultimate load at settlement of about  $0.4B$ . When the  $u/B$  ratio was increased beyond  $0.5$ , the footings have reached ultimate pressures at much smaller settlements of about  $0.15B$ .

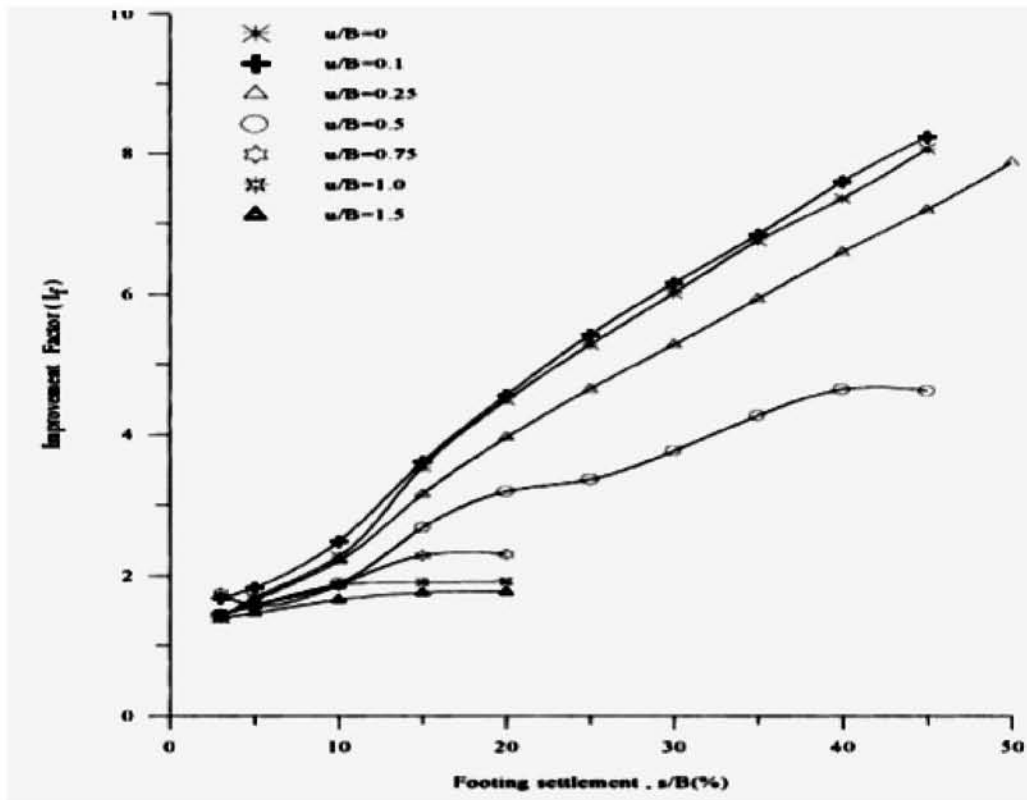


**Figure 4:** Variation of bearing pressure with settlement for static loading of unreinforced and reinforced foundation beds. (Moghaddas & Dawson 2012).

Table 1: Summary of previous studies on geocell reinforced soil

Name of researcher (Year)	Result

Sitharam and Sireesh (2012)	<p>1- Better performance of the footing can be obtained if the depth of placement of cellular mattress is 0.05D from the base of the footing in the case of sand beds.</p> <p>2- At 40 % footing settlement values, 30 % improvement is observed in load carrying capacity in the case of reinforced sand beds.</p>
Boushehrian, Hataf and Ghahramani (2012)	1- The large-scale results show that by using the grid-anchors, the amount of permanent settlement decreases to 30%, as compared with the unreinforced condition.
Hataf, Boushehrian and Ghahramani (2011)	1- The amount of dimensions settlement needed to reach its constant value decreases up to 17% relative to ordinary reinforcements and up to 50% relative to an unreinforced condition.
Moghaddas (2010)	<p>1- The optimum depth of the topmost layer of planar reinforcement is <math>u/B=0.35</math> while the depth to the top of the geocell should be approximately <math>u/B=0.1</math>.</p> <p>3- For bearing capacity greater than 200% and reductions in settlement by 75% can be achieved with the application of geocell reinforcement, whereas planar reinforcement arrangements can only deliver 150% and 64% for these two quantities, respectively.</p>
Moghaddas and Dawson (2010)	1- The optimum depth of planar reinforcement is $u/B=0.35$ and the 3D geotextile should be $u/B= 0.1$ .
Dash <i>et al.</i> (2001)	<p>1- To obtain maximum benefit, the top of geocell mattress should be <math>u/B=0.1</math> from the bottom of the footing.</p> <p>2- The optimum aspect ratio of geocell pockets for supporting strip footings was found to be around 1.67.</p>



**Figure 5:** Variation of improvement factors with settlement for different depths of placement of geocell. (Dash et.al, 2001).

## 6. Conclusions

Experimental study results obtained by previous researchers on reinforced soil with synthetic material can be concluded as follows:

1. The reinforcement reduces the magnitude of the final settlement.
2. In case of sand beds, the increased performance of the footing is observed to increase in footing settlement.
3. The optimum depth of geocell reinforcement is  $(u/B) = 0.1$ .
4. With the provision of a geocell layer, indicating that the geocell mattress transmits the footing load to a deeper depth, thereby bringing about a higher load carrying capacity.
5. The value of the mobilized shear stress ratio for geocell supported footings are only 0.35–0.5 unlike the unreinforced footing where it reaches 1.

## 7. Acknowledgments



This research is being carried out using the Universiti Teknologi Malaysia (UTM) short term research grant Scheme, No. (PY/2013/00870).

## 8. References

- Adams, M.T. and Collin, J.G. (1997) "Large Model Spread Footing Load Tests on Geosynthetic Reinforced Soil Foundations", *Journal of Geotechnical and Geoenvironmental Engineering*, 123 (1), pp. 66–72.
- Alamshahi, S., & Hataf, N. (2009) "Bearing capacity of strip footings on sand slopes reinforced with geogrid and grid-anchor", *Geotextiles and Geomembranes*, 27 (3), 217–226. doi:10.1016/j.geotextmem.2008.11.011.
- American Society for Testing and Materials (ASTM D4439-11), (2011). Standard Test Methods for Standard Terminology for Geosynthetics.
- Bathurst, R.J., Nernheim, A., Walters, D.L., Allen, T.M., Burgess, P., Saunders, D.D. (2009) "Influence of reinforcement stiffness and compaction on the performance of four geosynthetic-reinforced soil walls", *Geosynthetics International* 16 (1), 43e49.
- Boushehrian, et.al. (2011) "Modeling of the cyclic behavior of shallow foundations resting on geomesh and grid-anchor reinforced sand", *Geotextiles and Geomembranes*, 29(3), 242–248. doi:10.1016/j.geotextmem.2010.11.008.
- Boushehrian, A.H. and Hataf, N. (2008) "Bearing capacity of ring footings on reinforced clay", in Proc. 12th. Conf. of Int. Assoc. for Computer Methods and Advances in Geomechanics (IACMAG), Goa, India, pp. 3546-3551.
- Chung, W. and Cascante, G., (2006), "Experimental and numerical study of soil-reinforcement effects on the low-strain stiffness and bearing capacity of shallow foundations", *Journal of Geotechnical and Geological Engineering*, Original Paper.
- Dash, S. K. et.al. (2007) "Behaviour of geocell-reinforced sand beds under strip loading", 916, 905–916. doi:10.1139/T07-035.
- Dash, S. (2003) "Model studies on circular footing supported on geocell reinforced sand underlain by soft clay", *Geotextiles and Geomembranes*, 21(4), 197–219. doi:10.1016/S0266-1144(03)00017-7.
- Dash, S. K. et.al. (2001). Strip footing on geocell reinforced sand beds with additional planar reinforcement. *Geotextiles and Geomembranes*, 19(8), 529–538. doi:10.1016/S0266-1144(01)00022-X.
- El Sawwaf, M., & Nazir, A. K. (2012). "Cyclic settlement behavior of strip footings resting on reinforced layered sand slope", *Journal of Advanced Research*, 3(4), 315–324. doi:10.1016/j.jare.2011.10.002.

- El Sawwaf, M., & Nazir, A. K. (2010). "Behavior of repeatedly loaded rectangular footings resting on reinforced sand", *Alexandria Engineering Journal*, 49(4), 349–356. doi:10.1016/j.aej.2010.07.002.
- Ghazavi, M., & Mirzaeifar, H. (2010). "Bearing Capacity of Multi-Edge Shallow Foundations on Geogrid-Reinforced Sand", 600, 1–9.
- Guido, V.A., Biesiadecki, G.L. and Sullivan, M.J., (1985). "Bearing Capacity of a Geotextile Reinforced Foundation", *Proc. 11th Int. Conf. Soil Mech. And Found. Eng.*, San Francisco, Calif., pp. 1777~1780.
- Han, J., S. K. Pokharel, X. Yang, C. Manandhar, D. Leshchinsky, I. Halahmi, and R. L. Parsons. (2011). "Performance of geocell-reinforced RAP bases over weak subgrade under fullscale moving heel loads", *ASCE Journal of Materials in Civil Engineering*, 23, no.11:1525-1535.
- Hataf, N. et. al (2010). "Experimental and Numerical Behavior of Shallow Foundations on Sand Reinforced with Geogrid and Grid Anchor Under Cyclic Loading", 17(1), 1–10.
- Hejazi, S. M. et. al(2012). "A simple review of soil reinforcement by using natural and synthetic fibers. *Construction and Building Materials*", 30, 100–116.
- Jones, C.J.F.P., Clarke, D., (2007). The residual strength of geosynthetic reinforcement subjected to accelerated creep testing and simulated seismic events. *Geotextiles and Geomembranes* 25 (3), 155e169.
- Karimpour, H., Lade, P.V. (2010). "Time effects relate to crushing in sand", *ASCE Journal of Geotechnical and Geoenvironmental Engineering* 136 (9), 1209e1219.
- Khatib, A. (2010). "Bearing Capacity Of Granular Soil Overlying Soft Clay Reinforced with Bamboo-Geotextile Composite at the Interface". PhD. Thesis, Department of Geotechnics and Transportation, Faculty of Civil Engineering, Universiti Teknologi Malaysia, Johor Bahru.
- Kholdebarin, A. R. et.al. (2008). influence of soil improvement on seismic bearing capacity of shallow foundation.
- Kim, J.R., Kang, H., Kim, D., Lee, Y., Hwang, S.W., (2007). Viscoelastic analysis of constant creep tests on silicate-grouted sands at low stress levels. *ASCE Journal of Geotechnical and Geoenvironmental Engineering* 133 (9), 1162e1166.
- Koerner, R. M, (1990). *Designing with Geosynthetics*. (2nd Edition) Englewood Cliffs, N.J.: Prentice Hall.
- Kongkitkul, W., Tatsuoka, F., Hirakawa, D., Sugimoto, T., Kawahata, S., Ito, M., (2010). Time histories of tensile force in geogrid arranged in two full-scale high walls. *Geosynthetics International* 17 (1), 12e33.
- Kuwano, R., Jardine, R.J., (2002). On measuring creep behaviour in granular materials through triaxial testing. *Canadian Geotechnical Journal* 39 (5), 1061e1074.

- Lackner, C., Bergado, D. T., & Semprich, S. (2013). "Prestressed reinforced soil by geosynthetics – Concept and experimental investigations", *Geotextiles and Geomembranes*, 37, 109–123. doi:10.1016/j.geotexmem.2013.02.002
- Lade, P.V., Nam, J., Liggio Jr., C.D. (2010). "Effects of particle crushing in stress drop relaxation experiments on crushed coral sand", *ASCE Journal of Geotechnical and Geoenvironmental Engineering* 136 (3), 500e509.
- Lade, P.V., Liggio Jr., C.D., Nam, J. (2009). "Strain rate, creep, and stress drop-creep experiments on crushed coral sand", *ASCE Journal of Geotechnical and Geoenvironmental Engineering* 135 (7), 941e953.
- Lade, P.V., (2007). Experimental study and analysis of creep and stress relaxation in granular materials. In: DeGroot, D.J., Vipulanandan, C., Yamamuro, J.A., Kaliakin, V.N., Lade, P.V., Zeghal, M., Shamy, U.E., Lu, N., Song, C.R. (Eds.), *Advances in Measurement and Modeling of Soil Behavior, Proceedings of Sessions of Geo-Denver 2007*. GSP No.173. ASCE, Reston, pp. 1e11.
- Leshchinsky, D., Zhu, F., Meehan, C.L. (2010). "Required unfactored strength of geosynthetic in reinforced earth structures", *Journal of Geotechnical and Geoenvironmental Engineering, ASCE* 136 (2), 281e289.
- Li, F.-L. et al. (2012). "FE simulation of viscous behavior of geogrid-reinforced sand under laboratory-scale plane-strain-compression testing", *Geotextiles and Geomembranes*, 31, 72–80. doi:10.1016/j.geotexmem.2011.09.005.
- Li, A.L., Rowe, R.K., (2001). Influence of creep and stress-relaxation of geosynthetic reinforcement on embankment behaviour. *Geosynthetics International* 8 (3), 233e270.
- Ling I, Leshchinsky D, Tatsuoka F. ( 2003). "Reinforced soil engineering: advances in research and practice", Marcel Dekker Inc.
- Liu, H.B., Wang, X.Y., Song, E.X. (2009). "Long-term behavior of GRS retaining walls with marginal backfill soils", *Geotextiles and Geomembranes* 27 (4), 295e307.
- Liu, S.Y., Han, J., Zhang, D.W., Hong, Z.S. (2008). "A combined DJM-PVD method for soft ground improvement", *Geosynthetics International* 15 (1), 43–54.
- Madhavi Latha, G., Amit Somwanshi, S. (2009). "Bearing capacity of square footings on geosynthetic reinforced sand", *Geotextiles and Geomembranes* 27 (4), 281e294.
- Matro, A., Oghabi, M., Eisazadeh, A. (2013). "The Effect of Geogrid Reinforcement on Bearing Capacity Properties of Soil Under Static Load ; A Review", *The Electronic Journal of Geotechnical Engineering* 18 (J), 1881–1898.

Mosallanezhad, M., & Hataf, N. (2010). "Numerical Analysis of Granular Soils Bearing Capacity , Reinforced with Innovative Grid-Anchor System", 555(1), 1–8.

Mosallanezhad, M., Hataf, N. and Ghahramani, A. (2008). "Experimental Study of Bearing Capacity of Granular Soils, Reinforced with Innovative Grid-Anchor System", Journal of geotechnical and geological engineering, Vol. 26(3), pp. 299~312.

Noorzad, R., Mirmoradi, S.H. (2010). "Laboratory evaluation of the behavior of a geotextile reinforced clay", Geotextile and Geomembranes 28 (4), 386e392.

Perkins, S.W., 2000. Constitutive modeling of geosynthetics. Geotextiles and Geomembranes 18 (5), 273e292.

Pokharel, S. K. et .al. (2010). "Investigation of factors influencing behavior of single geocell-reinforced bases under static loading", Geotextiles and Geomembranes, 28(6), 570–578. doi:10.1016/j.geotextmem.2010.06.002.

Rowe, R.K., Taechakumthorn, C. (2011). "Design of reinforced embankments on soft clay deposits considering the viscosity of both foundation and reinforcement", Geotextiles and Geomembranes 29 (5), 448e461.

Rowe, R.K., Taechakumthorn, C. (2008). "Combined effect of PVDs and reinforcement on embankments over rate-sensitive soils", Geotextiles and Geomembranes 26 (3), 239–249.

Sawicki, A., (1999). Creep of geosynthetic reinforced soil retaining walls. Geotextiles and Geomembranes 17 (1), 51e65.

Shin, E.C., Das, B.M., Atalar, C., (2008). Cyclic plate load test on geogrid-reinforced granular pad Unpublished Material.

Shinoda, M., Bathurst, R.J., (2004). Lateral and axial deformation of PP, HDPE and PET geogrids under tensile load. Geotextiles and Geomembranes 22 (4), 205e222.

Sitharam, T.G, Sireesh, S. (2012). Behavior of Embedded Footings Supported on Geogrid Cell Reinforced Foundation Beds, 28(5), 1–12.

Tafreshi, S. N., & Dawson, a. R. (2012). "A comparison of static and cyclic loading responses of foundations on geocell-reinforced sand", Geotextiles and Geomembranes, 32, 55–68. doi:10.1016/j.geotextmem.2011.12.003.

Tafreshi, S N Moghaddas. et.al. (2011). "Experimental and numerical investigation on circular footing subjected to incremental cyclic loads", 9(4).

Tg, S., & Sireesh, S. (2012). "Behavior of Embedded Footings Supported on Geogrid Cell Reinforced Foundation Beds", 28(5), 1–12.

Yang, X. et.al. (2012). "Accelerated pavement testing of unpaved roads with geocell-reinforced sand bases", Geotextiles and Geomembranes, 32, 95–103. doi:10.1016/j.geotextmem.2011.10.004.

- Yang, X.M. (2010). "Numerical Analyses of Geocell-Reinforced Granular Soils under Static and Repeated Loads", Ph.D. dissertation, CEAE Department, the University of Kansas.
- Yarbasi N, Kalkan E, Akbulut S. (2007). "Modification of freezing–thawing properties of granular soils with waste additives", *Col Reg Sci Technol*;48:44–54.
- Yeo, S.S., Hsuan, Y.G. (2010). "Evaluation of creep behavior of high density polyethylene and polyethylene-terephthalate geogrids", *Geotextiles and Geomembranes* 28 (5), 409e421.
- Zhang, L. et.al. (2010a). Bearing capacity of geocell reinforcement in embankment engineering. *Geotextiles and Geomembranes*, 28(5), 475–482. doi:10.1016/j.geotexmem.2009.12.011.
- Zhang, L. et.al.(2010b). Bearing capacity of geocell reinforcement in embankment engineering. *Geotextiles and Geomembranes*, 28(5), 475–482. doi:10.1016/j.geotexmem.2009.12.011.
- Zidan, a. F. (2012). "Numerical Study of Behavior of Circular Footing on Geogrid-Reinforced Sand Under Static and Dynamic Loading", *Geotechnical and Geological Engineering*, 30(2), 499–510. doi:10.1007/s10706-011-9483-0