STRENGTHENING OF BOLTED SHEAR JOINT IN FERROCEMENT CONSTRUCTION

CHIONG CHUNG ENG

A project report submitted in a partial fulfillment of the requirement for the award of degree of Master of Engineering (Civil – Structure)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > JULY, 2007

To my beloved father and mother, family and dear friends.

ACKNOWLEDGEMENTS

Firstly, the author wishes to express utmost gratitude to his project supervisor, Professor Dr. M. A. Mansur for his patient guidance and valuable advice given throughout the planning and execution of the research. With his encouragement and help, the author even has the chance to publish a paper in the international journal. This is a special achievement that the author never thinks of before in his life.

Secondly, the author also greatly appreciates his co-supervisor, Associate Professor Dr. Mohd. Ismail for his valuable comments and advice given throughout the process of writing this report. His care and help to the author will not be forgotten.

Besides that, the author would also like to express his gratefulness to his PHD senior, Mr. Noor Ahmed for his advice and supplying of tools throughout the laboratory work of the research. His help has caused the author to save lots of time and money and most important prevent the author from making mistake during laboratory work.

The friendly and helpful technical staffs of the structural and material laboratory of Universiti Teknologi Malaysia, especially Mr. Jaafar and Mr. Razale also deserve a note of appreciation from the author. With their helps and supervisions, the author managed to operate very delicate machinery smoothly during testing.

Then, the author wishes to show his appreciation to Ms. Angela bt. Mohd Sabah, Sales Manager of BASF Construction Chemical Malaysia Sdn. Bhd. for showing her kind and generosity in providing the author with concrete admixtures free of charge.

With due respects, the author remembers her parents and friends on their countless blessings and supports which have always been a source of inspiration in achieving success to this level.

ABSTRACT

This study deals with strengthening of bolted shear joint in thin-walled ferrocement construction. Such a joint is attractive because it provides faster and neat means of assembling precast elements into a complete structure. Steel wires, bent into O-Shape and U-shape with or without extra straight wire, are considered as simple inserts around the bolt hole to enhance the joint strength. The parameters investigated include the number of layers of wire mesh (or volume fraction of reinforcement), edge distance of bolt hole and the effectiveness of different types of the steel inserts. Test results have shown that for small edge distance, failure occurs either in cleavage or shearing mode, and the strength of the joint increases with an increase in the edge distance. This continues up to an upper limit set by either tension or bearing failure. For a given edge distance and details of connected members, the strength of a joint can be significantly enhanced by using steel insert, while U-insert is most cost-effective. Available equations for predicting the joint strength in ferrocement composites can be slightly modified to include the effects of these inserts with a good level of accuracy. Since the cleavage failure equation is quite conservative, removing it from consideration or modifying it to reflect test data can improve the accuracy of the predictions of joint strength. As an alternative, strut-and-tie model, herein can predict the joint strength in ferrocement composite as proposed. However it does not perform that well if steel insert is included in the ferrocement plate, as the process to determine volume fraction of reinforcement becomes more complex.

ABSTRAK

Kajian ini adalah berkenaan sambungan ricih bolt dalam pembinaan dinding nipis ferrocement. Sambungan bolt sesuai kerana ia adalah cepat dan tersusun semasa menyambungkan elemen precast kepada suatu struktur yang menyeluruh. Dawai besi, yang dibengkokkan kepada bentuk O dan U yang ditambah atau tidak ditambah dengan dawai lurus, telah digunakan sebagai tetulang tambahan disekeliling bolt untuk meningkatkan kekuatan sambungan. Parameter yang dikaji termasuk bilangan lapisan jejaring dawai (atau peratus isipadu tetulang), jarak ke tepi dari lubang bolt dan kesesuaian tetulang tambahan besi yang berlainan. Keputusan ujian menunjukkan untuk jarak tepi yang kecil, kegagalan dalam "cleavage" atau rich berlaku. Kekuatan sambungan meningkat apabila jarak tepi bertambah. Ini berterusan sehingga satu tahap apabila kegagalan tegangan atau galas berlaku. Untuk jarak tepi dan keadaan sambungan yang ditetapkan, kekuatan sambungan boleh ditingkatkan keberkesannya dengan penggunaan tetulang tambahan besi, manakala tetulang tambahan bentuk U adalah paling kos-efektif. Formula yang ada untuk menjangka kekuatan komposit ferrocement boleh diubah sedikit untuk menambahkan kesan tetulang tambahan dengan ketepatan yang baik. Oleh kerana formula untuk kegagalan "cleavage" sangat konservatif, pengeluaran formula "cleavage" dalam pertimbangan atau pengubahsuaian formula tersebut dapat meningkatkan ketepatan jangkaan kekuatan sambungan. Modal strut-and-tie merupakan satu cara lain untuk menjangka kekuatan sambungan komposit Tetapi keputusannya menjadi kurang tepat jika tetulang tambahan ferrocement. digunakan dalam komposit ferrocement. Ini disebabkan pengiraan peratus isipadu tetulang untuknya adalah sukar.

TABLE OF CONTENTS

CHAPTER

1

1.1

TITLE

PAGE

AUTHOR DECLARATION	ii
DEDICATION	iii
ACKNOWLEDGEMENTS	iv
ABSTRACT	v
ABSTRAK	vi
TABLE OF CONTENTS	vii
LIST OF TABLES	X
LIST OF FIGURES	xi
LIST OF APPENDICES	XV
INTRODUCTION	1
Research Problem	2

1.2Objective21.3Scope of Project3

2	L	ITERATURE REVIEW	4
	2.1	Ferrocement and Its Application	4
	2.2	Industrialized Ferrocement Housing System	5
	2.3	Basic Failure Modes and Equations in Bolted Shear Joint	6

		2.3.1	Shear Failure Mode	6
		2.3.2	Tension Failure Mode	7
		2.3.3	Cleavage Failure Mode	7
		2.3.4	Bearing Failure Mode	8
	2.4	Streng	thening of Bolted Shear Joint	8
	2.5	Strut-a	and-Tie Model	9
3]	МЕТН	IODOLOGI	16
	3.1	Deta	il of Test Program	17
		3.1.1	Materials	18
		3.1.2	Preparation of Specimens	20
		3.1.3	Test Setup, Instrumentation and Test Procedure	21
	3.2	Pred	ict Failure Loads of Test Program by Equations	22
	3.3	Strut	t-and-Tie Model for Bolted Shear Joint	24
		3.3.1	Compressive Strut Width, b	25
		3.3.2	Effectiveness Factor of Concrete, v	26
	3.4	Veri	fication of Data from Available Journal	26
	3.5	Mod	lification of Cleavage Failure Equation	27
4]	DATA	COLLECTION AND ANALYSIS	35
	4.1	Intro	oduction	35
	4.2	Yield	d Strength of Wire Mesh and Steel Bar	36
	4.3	Test	Program Results	36
		4.3.1	Prediction of results by Failure Mode Equations	36
		4.3.2	Prediction of results by Strut-and-Tie Model	37
		4.3.3	Experimental Results	37
	4.4	Anal	lysis of Test Data from Available Journal	37
	4.5	Mod	lification of Cleavage Failure Equation	38
5]	DISCU	JSSION OF RESULTS	47
	5.1	Intro	oduction	47

	5.2	Test	Program Results	47
		5.2.1	General Behavior	48
		5.2.2	Effects of Volume Fraction of Reinforcement, Vf	49
		5.2.3	Effect of Edge Distance, e	50
		5.2.4	Effect of Steel Insert in Shear Joint of Ferrocement	
			Plate	51
		5.2.5	Comparison of Experimental Results with	
			Theoretical Predictions	52
	5.3	Ana	lysis of Test Data Available	55
	5.4	Effe	cts of Using Modified Cleavage Equation	56
6		CONC	CLUSIONS AND RECOMMENDATIONS	66
	6.1	Con	clusions	66
	6.2	Reco	ommendations for Further Research	68

REFERENCES

69

Appendices	71 - 102
	, 1 10

LIST OF TABLES

TABLE NO	D. TITLE	PAGE
3.1	Test scheme and specimen details	28
3.2	Test Scheme and Specimen Details (Journal)	29
4.1	Yield strength of wire mesh and steel bar	38
4.2	Comparison of calculated strength with experimental results -(Based on the Equation Method with or without considerin Cleavage Failure and Strut-and-Tie Model)	
4.3	Comparison of calculated strength with experimental results (<i>Journal</i>) - (Based on the Equation Method with or without considering Cleavage Failure and Strut-and-Tie Model)	s 41
4.4(a)	Comparison of calculated strength with experimental results - (Based on the Original Cleavage Equation and the Modifie Cleavage Equation)	
4.4(b)	Comparison of calculated strength with experimental results (Journal) - (Based on the Original Cleavage Equation and the Modified Cleavage Equation)	45

LISTS OF FIGURES

FIGURE N	NO. TITLE	PAGE
2.1	Production of ferrocement	11
2.2	Original 1848 boat in Brignoles Museum	11
2.3	Innovative use of ferrocement	12
2.4	Ferrocement sunscreens in Singapore	12
2.5	Precast ferrocement panels	13
2.6	Assembly of ferrocement panels	13
2.7	Bolted joints at different location	14
2.8	Components acting on bolted joint	14
2.9	Four basic modes of failure	15
2.10	Strut-and-tie model for a deep beam	15

2.11	Compression struts	15
3.1	Dimensions of sample	30
3.2	Diagram of test setup	30
3.3	Detail of specimen series	30
3.4	Mesh sample and steel bar under testing	31
3.5	Moulds with wire mesh bundle within	31
3.6	Freshly-cast specimens and final specimens	32
3.7	Dartec machine	32
3.8	Test setup of the specimen	32
3.9	Strut-and-tie Model for ferrocement shear joint	33
3.10	Strut-and-tie model at Staad Pro to get ratio of P	33
3.11	Compression struts for ferrocement shear joint	34
5.1	Effect of volume fraction of reinforcement on the strength of the joint	57
5.2(a)	Effect of Type A insert on the strength of the joint	57
5.2(b)	Effect of Type B insert and Type C insert on the strength of the joint	58

5.3(a)	Comparison of experimental results with predicted results for V2-E	58
5.3(b)	Comparison of experimental results with predicted results for V5-E	59
5.3(c)	Comparison of experimental results with predicted results for V2-D4-E	59
5.3(d)	Comparison of experimental results with predicted results for V1-E	60
5.3(e)	Comparison of experimental results with predicted results for V3-E	60
5.3(f)	Comparison of experimental results with predicted results for V1-D3-E	61
5.3(g)	Comparison of Experimental Results with Predicted Results for V1-D3-T-E	61
5.4	Comparison between experimental and calculated ultimate strengths by Equation Method (Considering Cleavage Failure)	62
5.5	Comparison between experimental and calculated ultimate strengths by Equation Method (Without Considering Cleavage Failure)	63
5.6	Comparison between experimental and calculated ultimate strengths by Strut-and-Tie Model (Without Steel Insert)	64

65

5.7	Comparison between experimental and calculated ultimate
	strengths by Modified Cleavage Failure Equation

LISTS OF APPENDICES

APPENDIX	TITLE	PAGE
A1	Yield strength of 1.60 mm diameter wire mesh (Specimen 1)	72
A2	Yield strength of 1.60 mm diameter wire mesh (Specimen 2)	72
A3	Yield strength of 1.60 mm diameter wire mesh (Specimen 3)	73
A4	Yield strength of 1.42 mm diameter wire mesh (Specimen 1)	73
A5	Yield strength of 1.42 mm diameter wire mesh (Specimen 2)	74
A6	Yield strength of 1.42 mm diameter wire mesh (Specimen 3)	74
A7	Yield strength of 3.60 mm diameter steel wire (Specimen 1)	75

A8	Yield strength of 3.60 mm diameter steel wire (Specimen 2)	75
A9	Yield strength of 3.60 mm diameter steel wire (Specimen 3)	76
A10	Yield strength of 4.00 mm diameter steel wire (Specimen 1)	76
A11	Yield strength of 4.00 mm diameter steel wire (Specimen 2)	77
A12	Yield strength of 4.00 mm diameter steel wire (Specimen 3)	77
B1	Design Mix for Mortar A	79
B2	Design Mix for Mortar B	80
C1	Load – displacement curves of Series V2- E	83
C2	Load – displacement curves of Series V5- E	84
C3	Load – displacement curves of Series V2-D4-E	85
C4	Load – displacement curves of Series V1- E	86
C5	Load – displacement curves of Series V3- E	87
C6	Load – displacement curves of Series V1-D3-E	88
C7	Load – displacement curves of Series V1-D3-T-E	89
D1	Failure mode for Series V2-E	91

D2	Failure mode for Series V5-E	91
D3	Failure mode for Series V2-D4-E	91
D4	Failure mode for Series V1-E	92
D5	Failure mode for Series V3-E	92
D6	Failure mode for Series V1-D3-E	93
D7	Failure mode for Series V1-D3-T-E	93
E1(a)	Predicted Results by Failure Mode Equations (With Considering Cleavage Failure)	95
E1(b)	Predicted Results by Failure Mode Equations (Without Considering Cleavage Failure)	96
E2	Predicted Results by Strut-and-Tie Model	97
E3(a)	Predicted Results by Failure Mode Equations (Journal) (With Considering Cleavage Failure)	98
E3(b)	Predicted Results by Failure Mode Equations (Journal) (Without Considering Cleavage Failure)	99
E4	Predicted Results by Strut-and-Tie Model (Journal)	100
E5(a)	Predicted Results by Failure Mode Equations (Modified Cleavage Failure Equation)	101
E5(b)	Predicted Results by Failure Mode Equations (Journal) (Modified Cleavage Failure Equation)	102

CHAPTER 1

INTRODUCTION

Ferrocement is a type of thin reinforced-concrete construction where instead of reinforcing bars, larger amount of smaller diameter wire meshes are used uniformly throughout the cross section and, instead of concrete, mortar is used (Surendra & Naaman, 1978). The ferrocement technique was invented about 160 years ago by Joseph Lambot when he constructed the first ferrocement boat at Brignoles, France in 1848.

Recently, due to increasing labour cost and the need for producing high quality construction material, Industrialized Building System (IBS) is introduced. The system encourages structural components to be manufactured in the factory in mass quantity and assemble them on site by using suitable connections. Ferrocement can be fabricated into any desired shape and, being a thin-walled composite, the components will be lighter for handling and transportation. Assembling these components by using bolted joints will eliminate the requirement for messy wet connection on site and greatly expedite the construction process. The ACI Committee 549 had provided design guide for the fabrication of ferrocement, but there is still a lack of information on bolted connections in precast ferrocement panels.

Since 1994, a number of research programs on the behaviour and strength of bolted joint in ferrocement has been conducted at the National University of Singapore (Mansur et. al., 1994, 2001, Abdullah and Mansur, 1995, Tan, 1999). These investigations identified four different modes of failure for a shear joint and attempted to develop analytical models for predicting the ultimate strength of such a joint. A careful review of the resulting expressions reveal that, for a given geometry and connected member details, joint strength may be enhanced significantly by incorporating simple steel insert of desired shape at suitable location. The focus of this study has therefore been directed toward strengthening of bolted shear joint in ferrocement construction.

1.1 Research Problem

This study concentrates mainly on the problem of identifying the effect of steel inserts on bolted shear joint and their effectiveness in strengthening bolted shear joint.

1.2 Objectives

The objectives of the study are:-

 a) To propose suitable ways to strengthen bolted shear joint using steel insert by investigating the available mechanistic models and modifying the models for incorporation of inserts. b) To verify experimentally the effectiveness of the strengthening method by designing and conducting a series of tests and comparing with the analytical predictions.

1.3 Scope of Project

This research was aimed to propose suitable way to strengthen bolted shear joint. To archive that, some literature study had been done and the equations of four mode of failure were analyzed to see the relationship of the parameters involved. A strut-and-tie model was also constructed to achieve that purpose. From the analysis, steel inserts could significantly increase the strength of the joint. Furthermore, tests on the steel inserts on bolted shear were conducted to verify its effectiveness.