



OVERVIEW ON PREVIOUS COORDINATED CADASTRAL SYSTEM RESEARCH PROJECTS

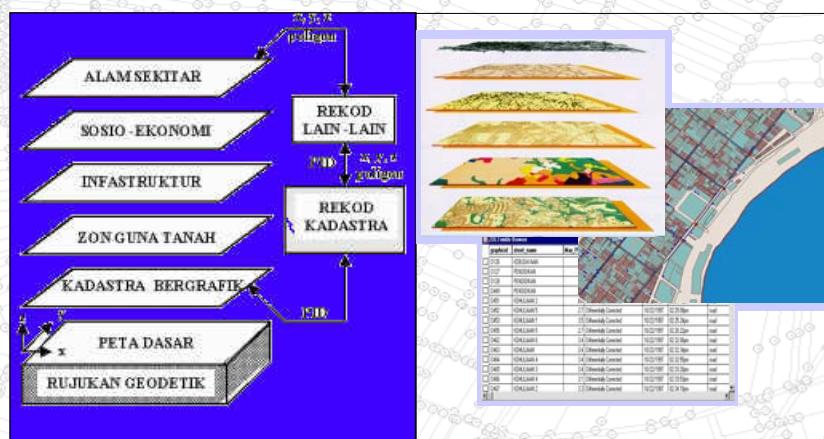
- PILOT STUDY : 1996
 - Pilot study has been conducted in Malaysia with the following objectives
 - To determine the effectiveness of using Global Positioning System for cadastral control network densification
 - To select most appropriate adjustment method for the large cadastral network
- FEASIBILITY STUDY: 1997-1999
 - Feasibility study has been carried out in the Melaka/Johor border with the following objectives:
 - To carry out larger cadastral network adjustment
 - To use Rectified Skew Orthomorphic (RSO) projection system in the cadastral network adjustment
 - To use geocentric datum (WGS84) as a National Geodetic Datum
 - To introduce GPS measurement for cadastral application
 - The outcome of this study are as follows :
 - RSO projection may be used to replace Cassini for cadastral mapping purposes.
 - Least squares adjustment technique is suitable and practical to be used in the large cadastral network adjustment
 - Potential use of geocentric datum has been realized
 - A guideline for using GPS in Cadastral surveying is produced
- It has been realized from the previous studies that Coordinated Cadastral System could potentially be implemented in Peninsular Malaysia

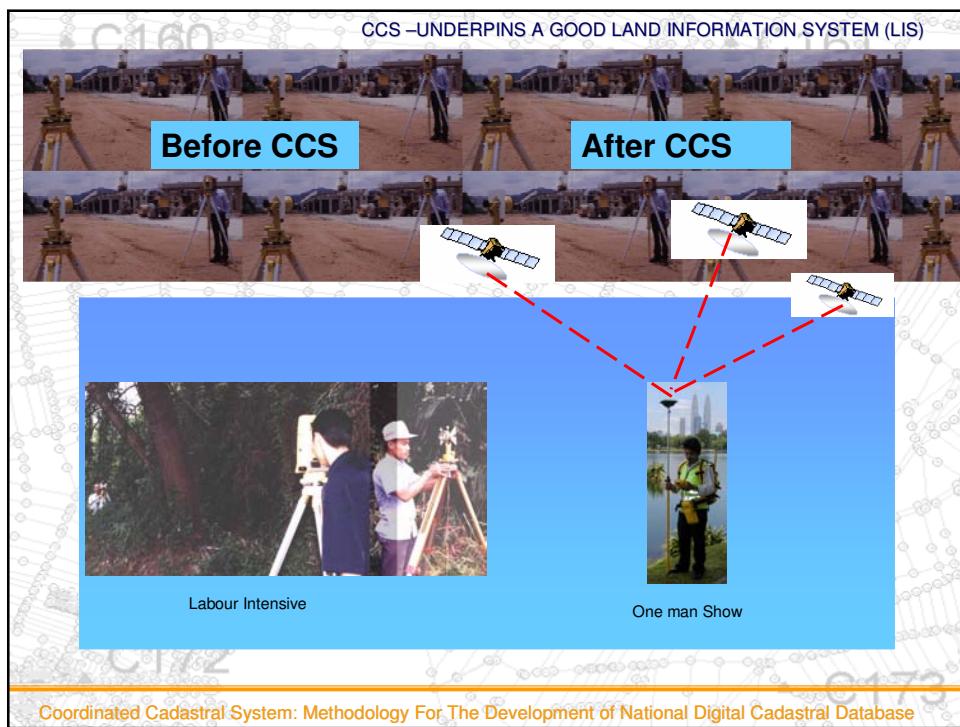
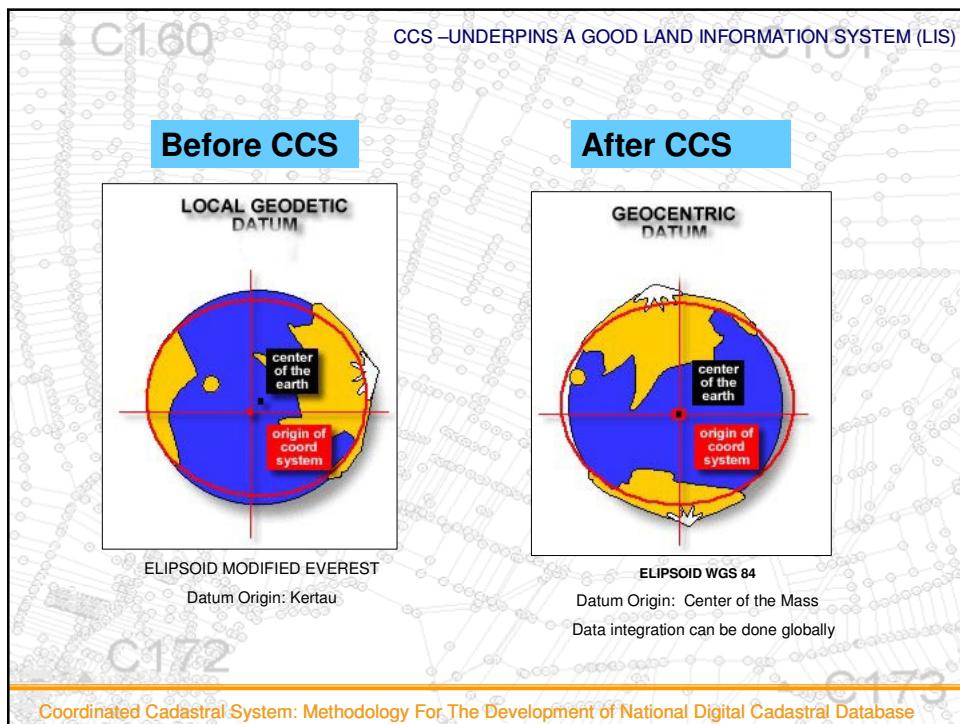
THE IMPORTANCE OF THE STUDY

- ✓ Coordinated Cadastral System (CCS) – a cadastral reform programme to improve the cadastral survey system. Implementation of CCS – *feasible*, according to results of feasibility study.
- ✓ Long term benefits to be accrued from CCS implementation:
 - Utilizing “Whole to the Part” methodology.
 - Facilitate use of rapid data acquisition, storage, processing & management techniques.
 - Improvement of the cadastral survey system.
 - Provides common reference system.
 - Facilitate data integration.
 - CCS – basis for / underpins a good LIS.

Coordinated Cadastral System: Methodology For The Development of National Digital Cadastral Database

CCS –UNDERPINS A GOOD LAND INFORMATION SYSTEM (LIS)





Before CCS

BLOCK 1 - RAMBIRAHAN						
	X	Y	Z	U	V	
02	211.27 23	8.1523		6.409	-5.296	3116.4 ± 0.000
03	211.27 35	8.1524		6.409	-5.274	3116.3 ± 0.000
03	24 34 26	32.2121		29.290	13.420	3154.7 ± 0.000
04	24 34 26	1.1778		1.451	0.445	3154.2 ± 0.000
04	24 34 26	1.1778		2.183	0.772	3154.2 ± 0.000
04	24 34 10	23.3579		21.257	9.733	3181.9 ± 0.000
04	24 34 26	13.764		17.196	7.174	3204.1 ± 0.000
05	24 34 26	13.769		13.800	9.259	3204.1 ± 0.000
05	24 34 26	12.0370		9.216	6.139	3230.4 ± 0.000
01	23 32 30	37.1296		34.194	14.897	3244.8 ± 0.000
00	21 29 29	3.841		2.393	1.423	3248.4 ± 0.000
09	20 25 30	7.864		6.770	2.738	3278.4 ± 0.000

JIMBAR 432.168 0.027 0.032

TITAKAN SUMUR S = 17449
TITAKAN BERPENG = 0.03646 42 01 10

After CCS

GPS

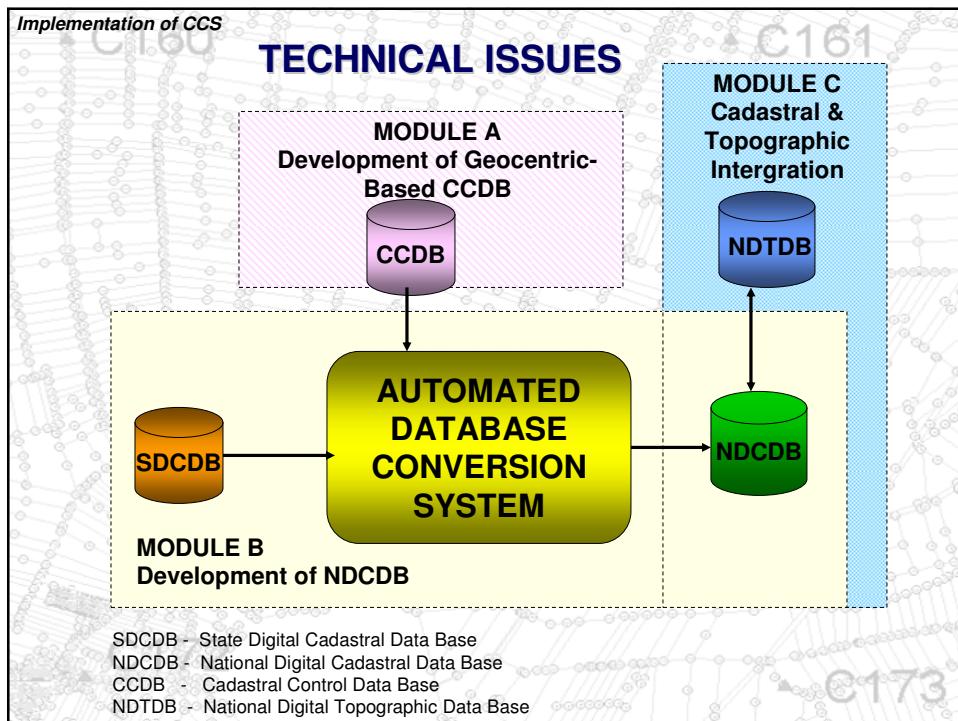
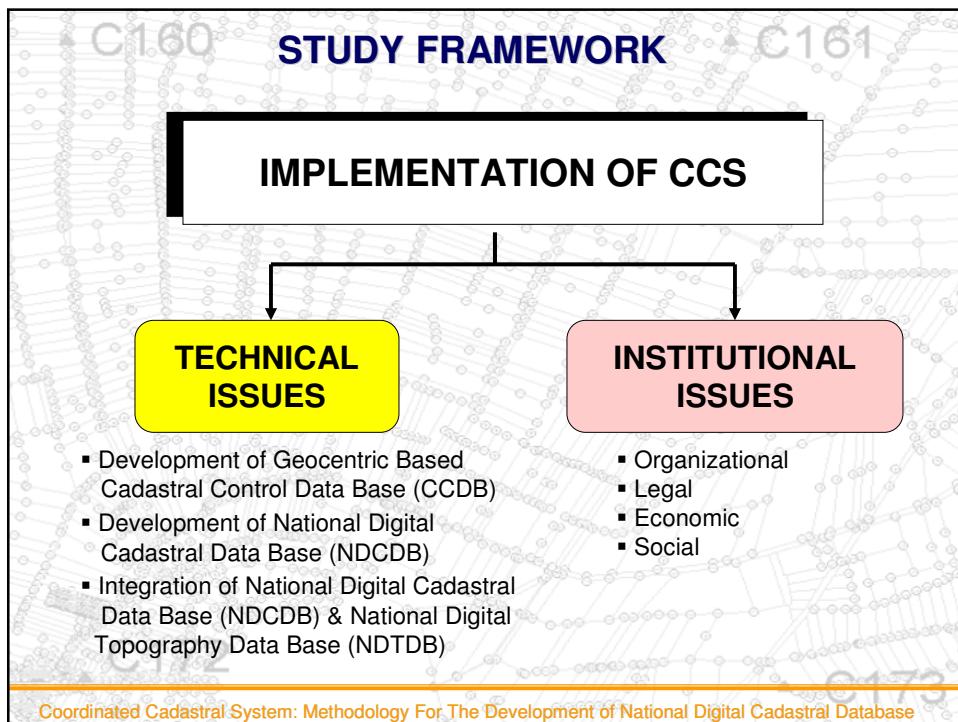
GPS

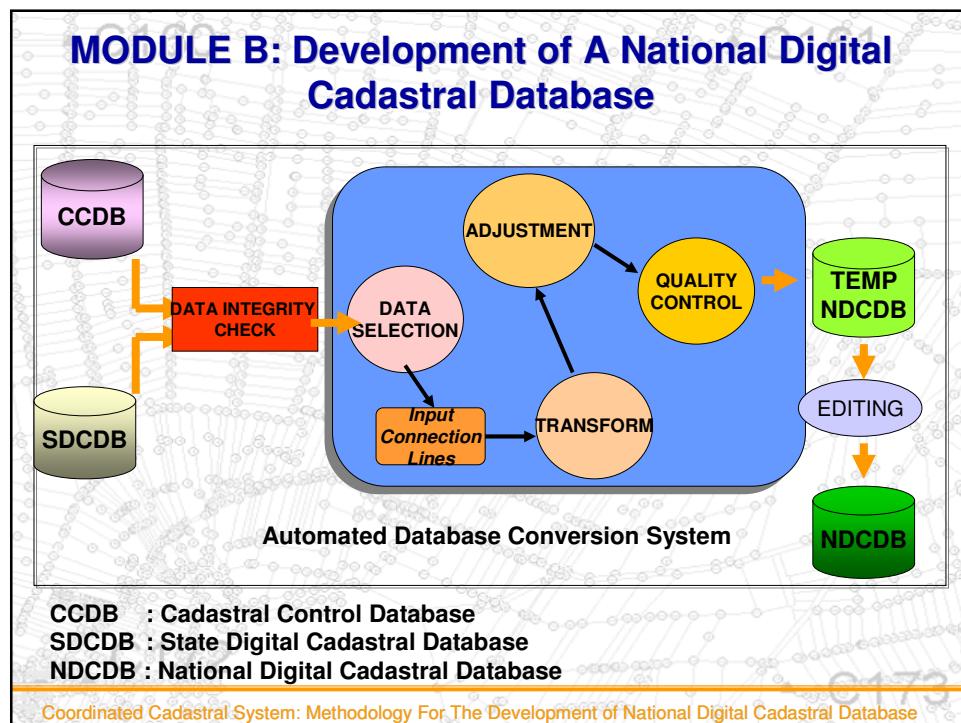
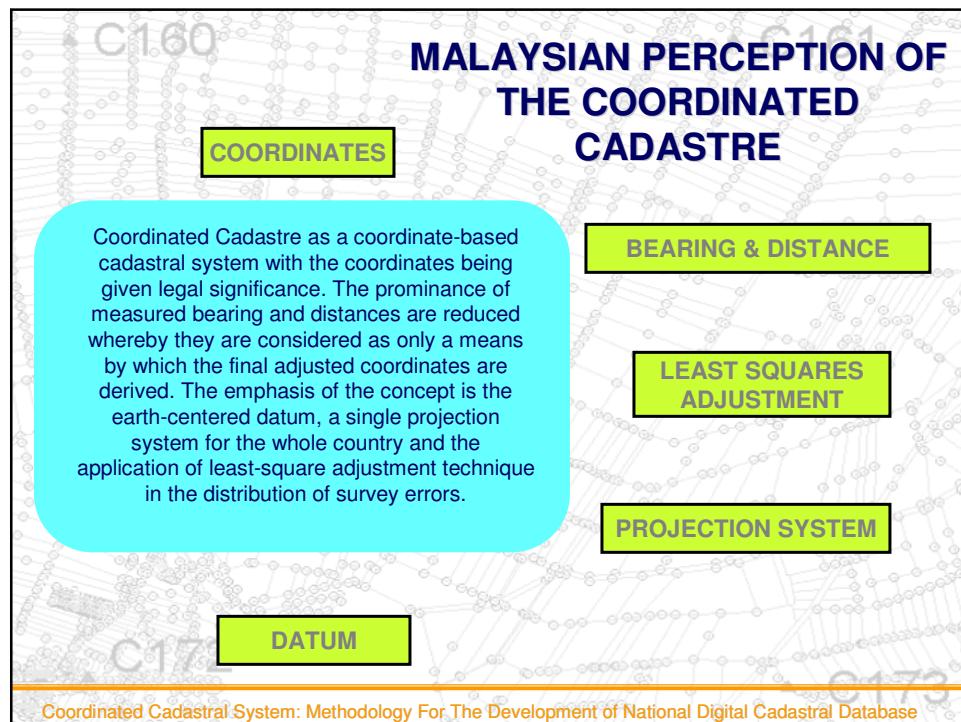
Network adjustment technique: Transit /Bowditch
Low accuracy and suitable for individual lot

Network adjustment technique: Least Squares
High accuracy and rigorous method

Coordinated Cadastral System: Methodology For The Development of National Digital Cadastral Database

The image is a side-by-side comparison of two photographs illustrating the impact of CCS on land surveying. The left photograph, titled 'Before CCS', shows a surveyor working at a desk, manually plotting coordinates onto large-scale maps using a compass and a ruler. The right photograph, titled 'After CCS', shows a modern surveying environment where the same task is performed using a computerized plotting system. The new system includes a large monitor displaying a digital map, a plotter machine, and a keyboard. The surveyor is now seated at a desk, interacting with the computer rather than performing manual calculations. The background of both images features faint, stylized architectural or cadastral drawings.



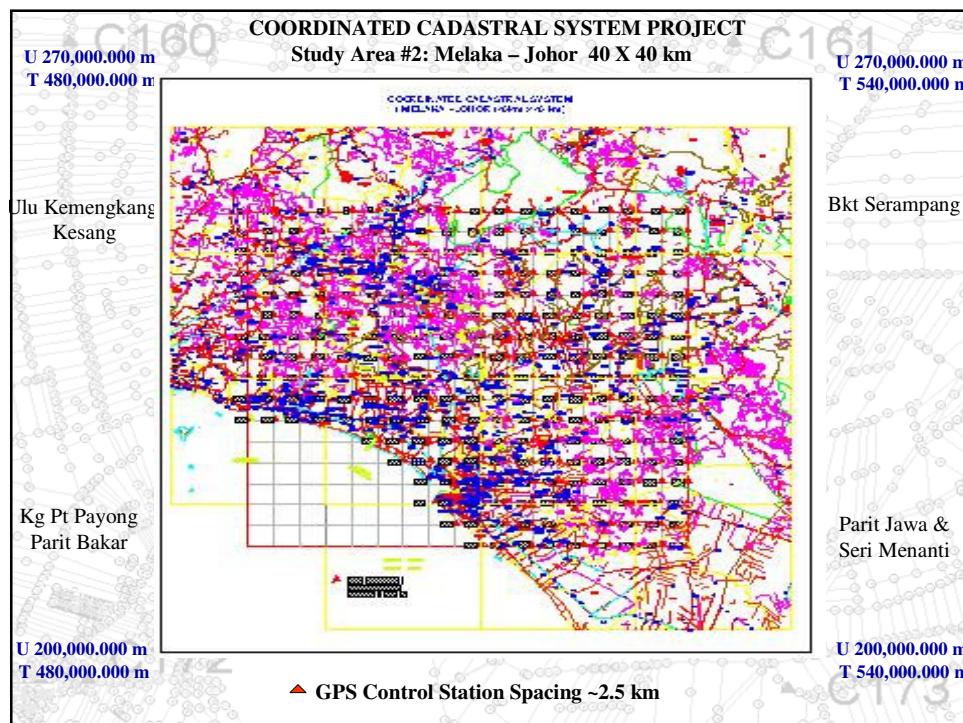
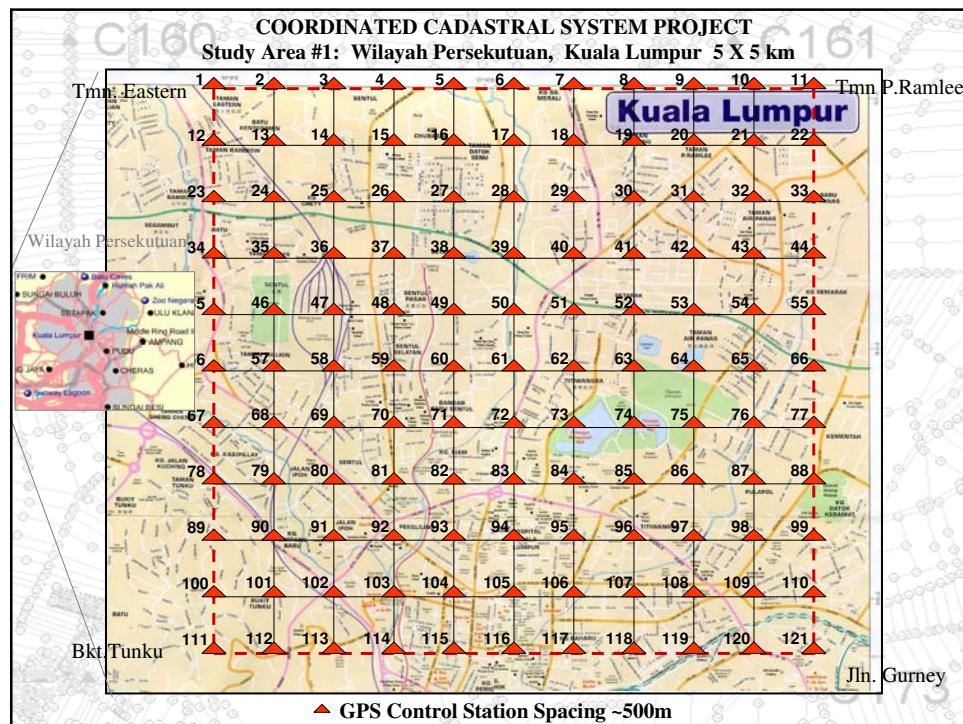


OBJECTIVES (Modified from original proposal)

- The study objectives of this module are:
 - ✓ To assess and analyze the use of the existing State Digital Cadastral Database (DCDB) as the primary input to Coordinated Cadastral System;
 - ✓ To develop Cadastral Control Infrastructure using GPS technology for both urban and rural areas;
 - ✓ To develop Automated Database Conversion System for the development of National Digital Cadastral Database

STUDY AREA





Cadastral Control Infrastructure Development

PHASE	MELAKA	REMARKS	JOHOR	REMARKS
Reconnaissance*	Mac 2001	131 of 132 stations have been identified	Mac 2001	89 of 132 stations have been identified
Monumentation*	Mac 2001	Standard Traverse, Pipe and Cadastre mark	Mac 2001	Standard Traverse, Pipe and Cadastre mark
GPS Field Observation *	19 Mac – 7 April	Jasin, Melaka Tengah, Alor Gajah	7 Apr – 14 April	Muar
GPS Processing and Results*	Results Delivered: 3/5/2001			

* Note: Geodesy Section, JUPEM Melaka and JUPEM Johor

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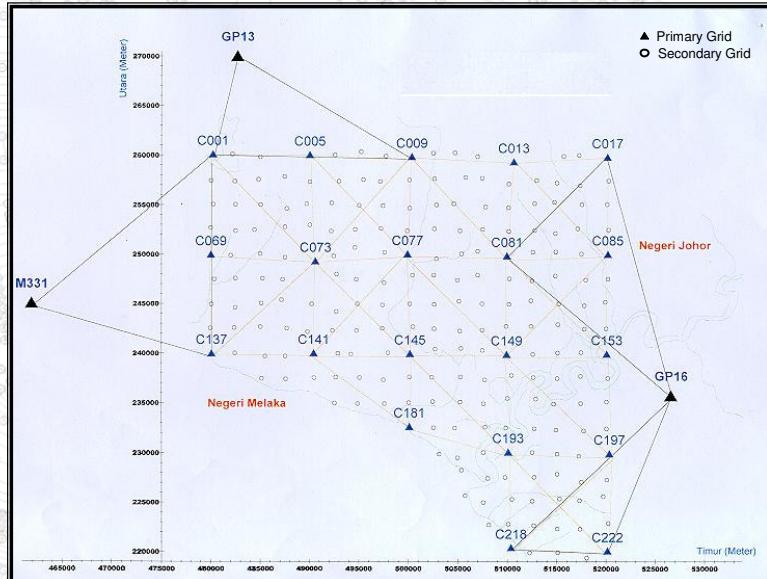
GPS Field Observation For Study Area # 2 (Melaka – Johor)

- 📁 GPS Network Grid consists of
 - Primary Grid (10 km x 10 km)
 - Secondary Grid (2.5 km x 2.5 km)

Observation Technique	Primary Grid	Secondary Grid
	Static Reference Stations: M331 (Tg.Keling, Melaka), GP13 (Tebong, Melaka), GP16 (Pagoh, Johor)	Rapid Static : Reference Stations Primary Grid Points
Observation Time	90 minutes	15-30 minutes
Total of Stations	23	197
GPS Processing Software	Trimble Geomatic Office V1.1	Trimble Geomatic Office V1.1
Adjustment Software	Geolab 2.4c	Trimble Geomatic Office V1.1

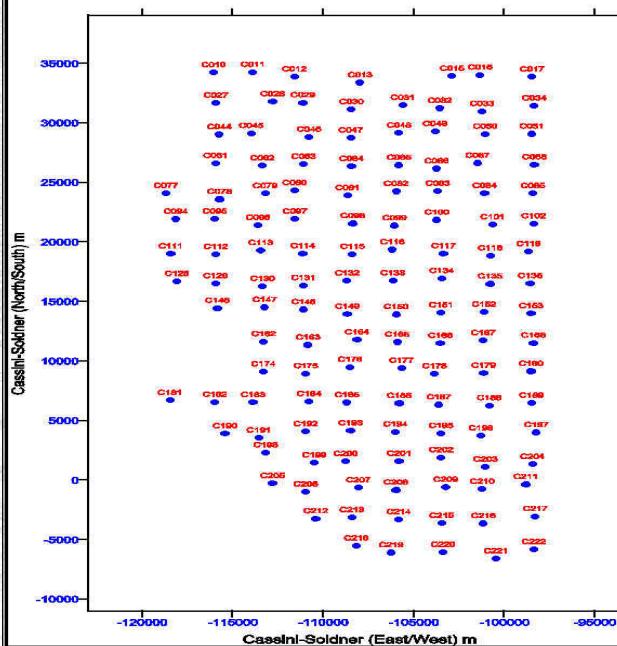
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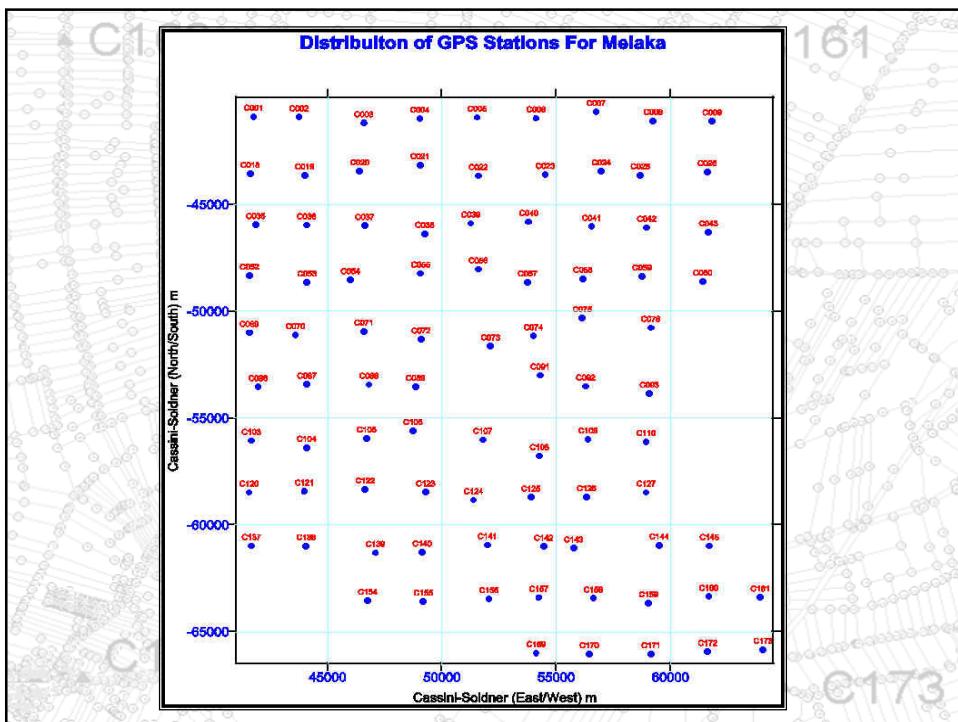
GPS Network Grid For Study Area # 2: Melaka



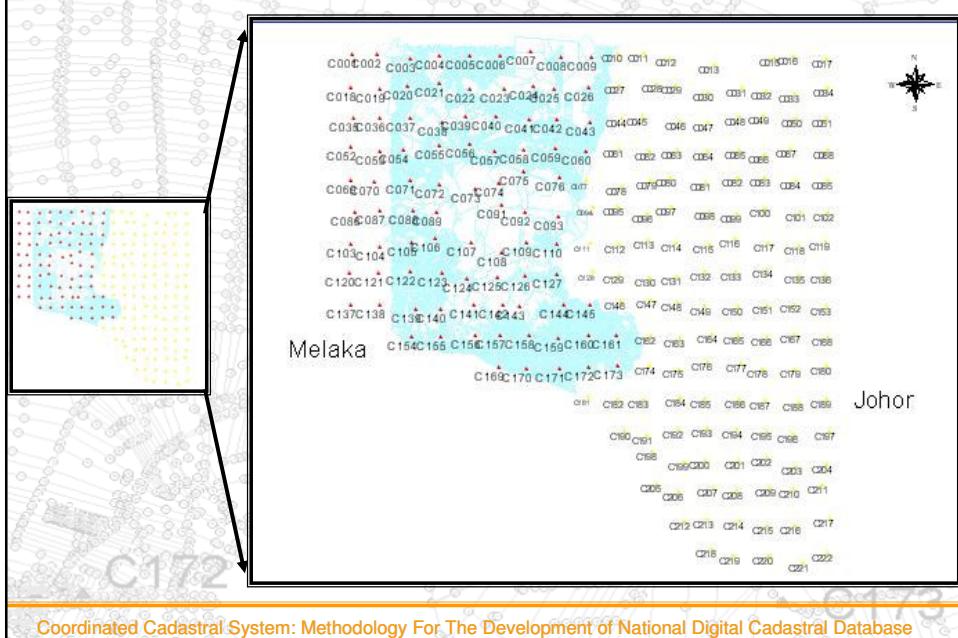
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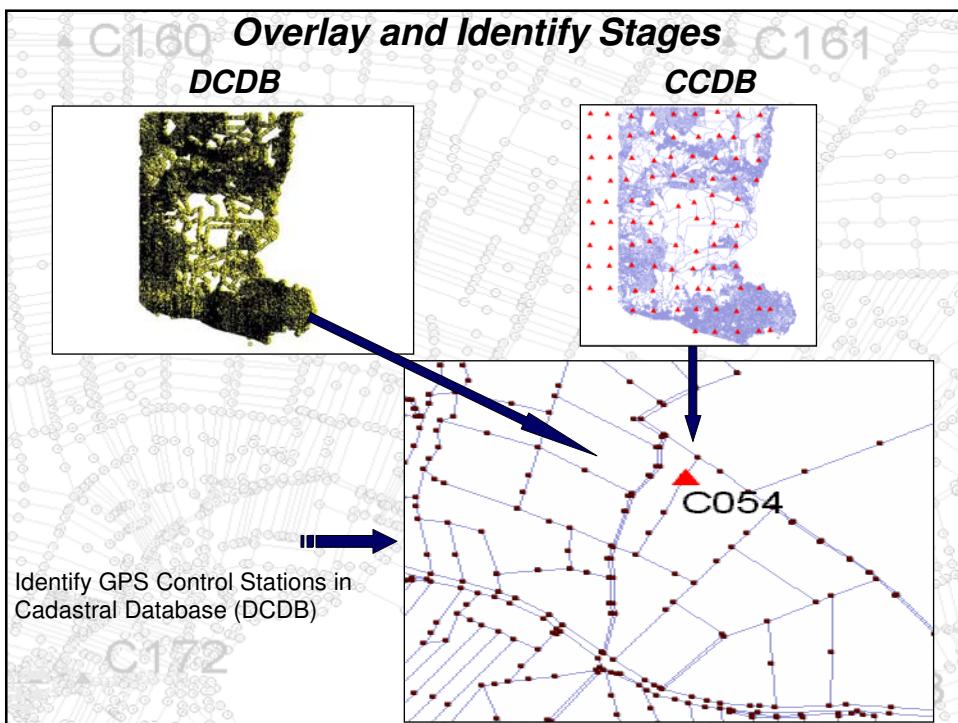
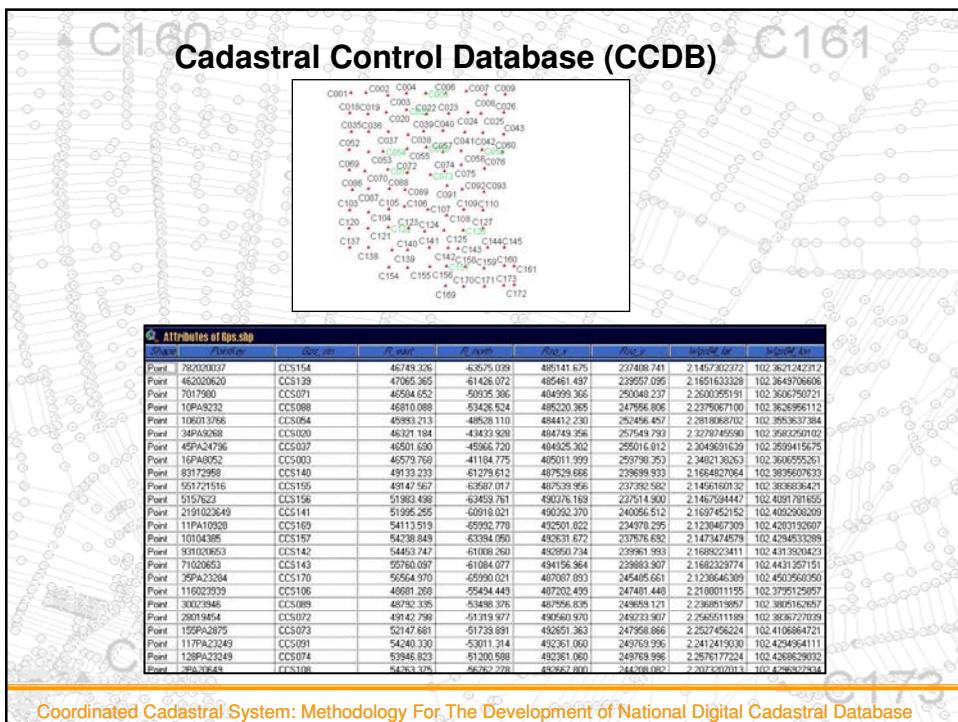
Distribution of GPS Stations For Johor

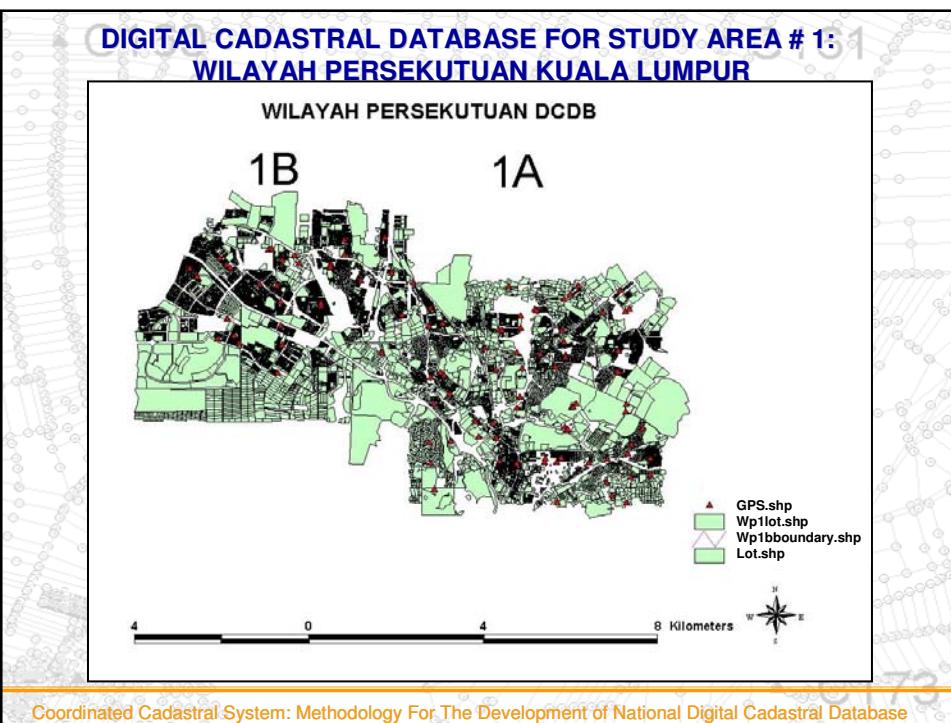
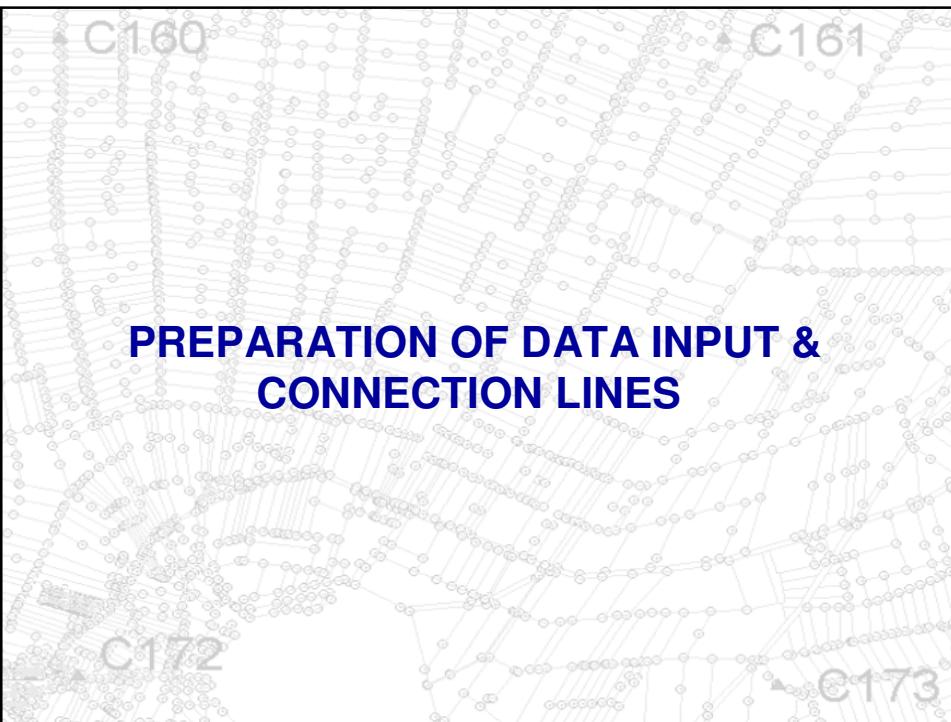


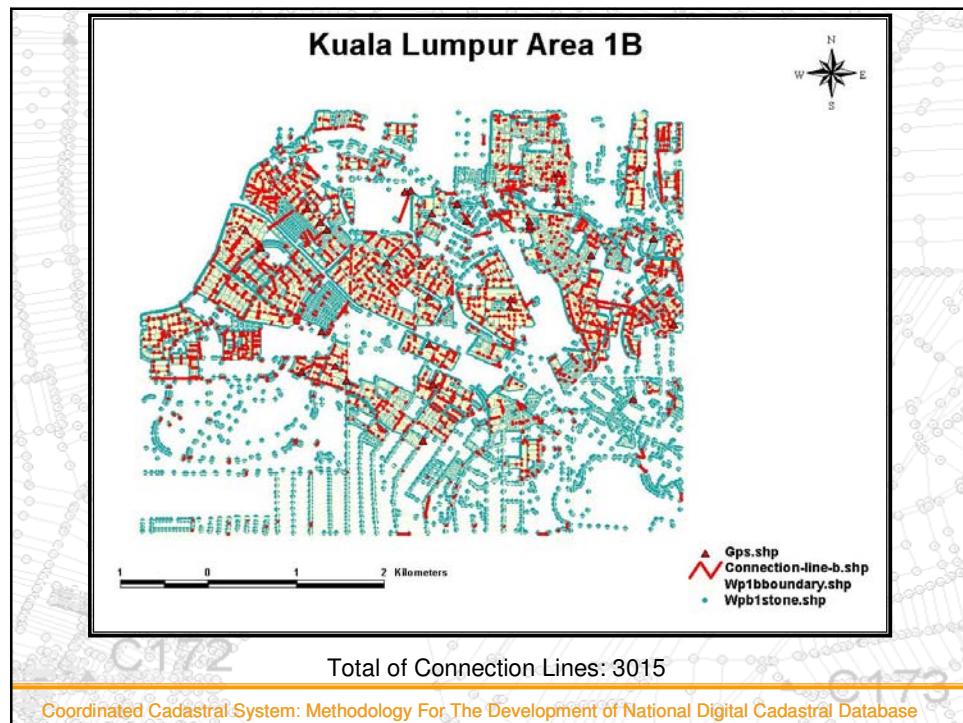
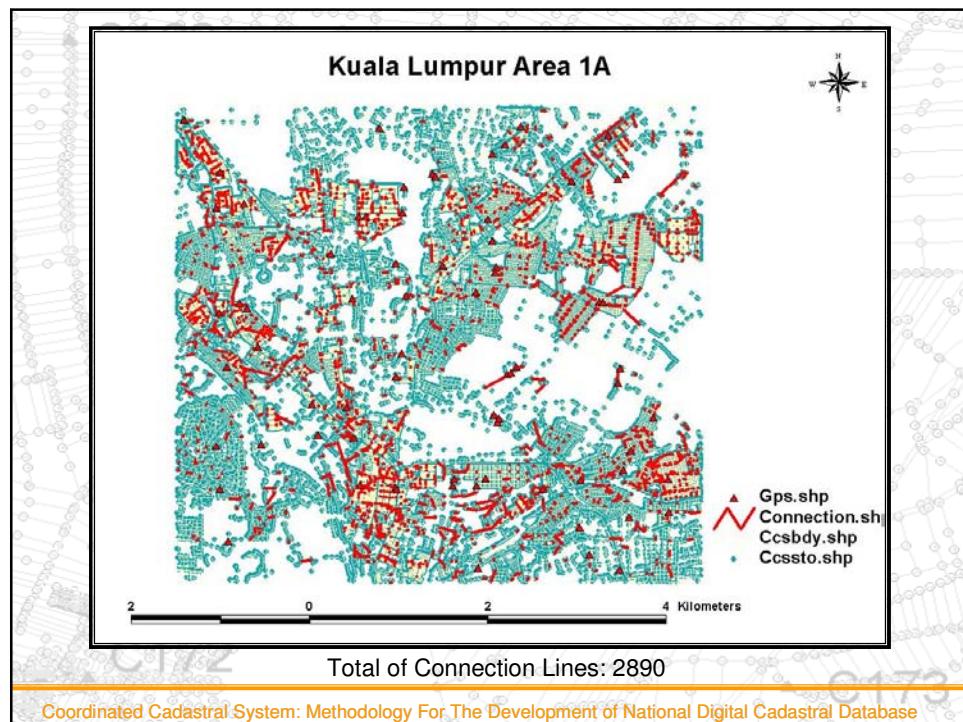


Cadastral Control Infrastructure For Study Area # 2

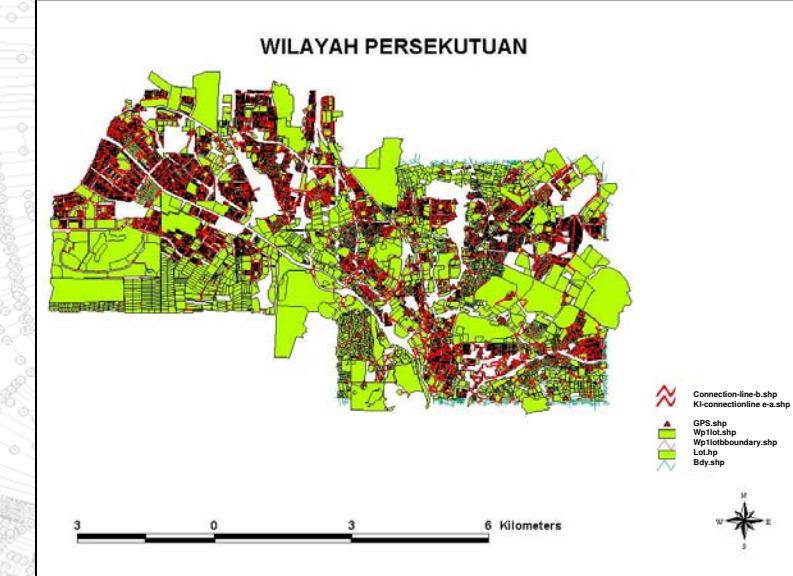






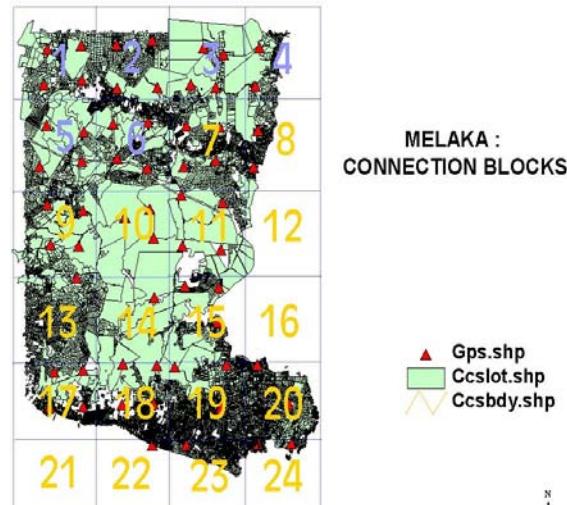


**CADASTRAL INFRASTRUCTURE FOR STUDY AREA # 1:
WILAYAH PERSEKUTUAN KUALA LUMPUR**

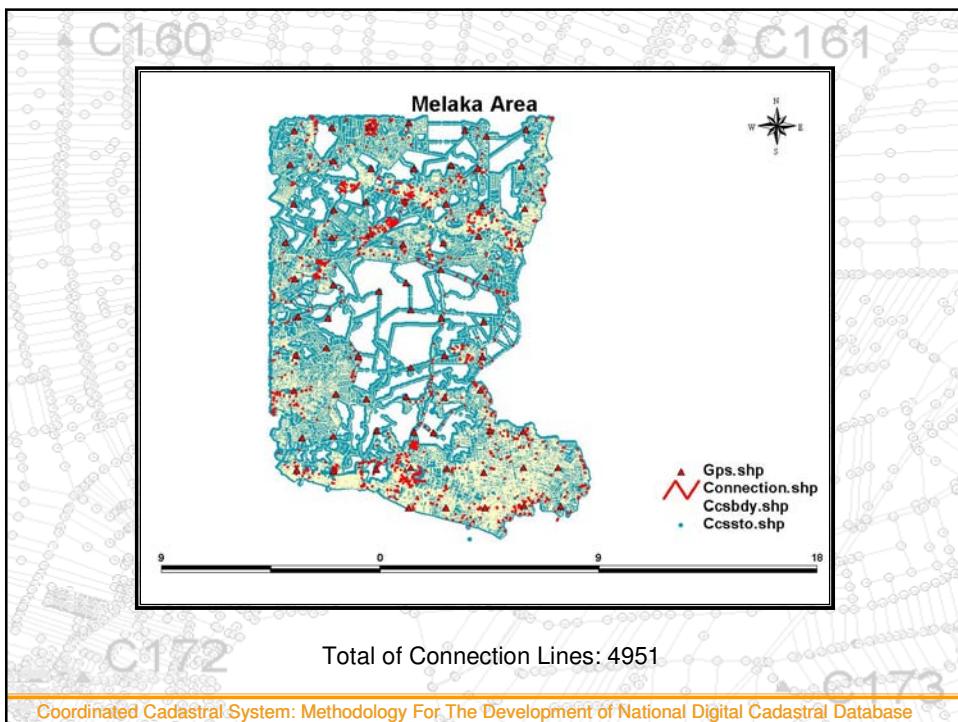


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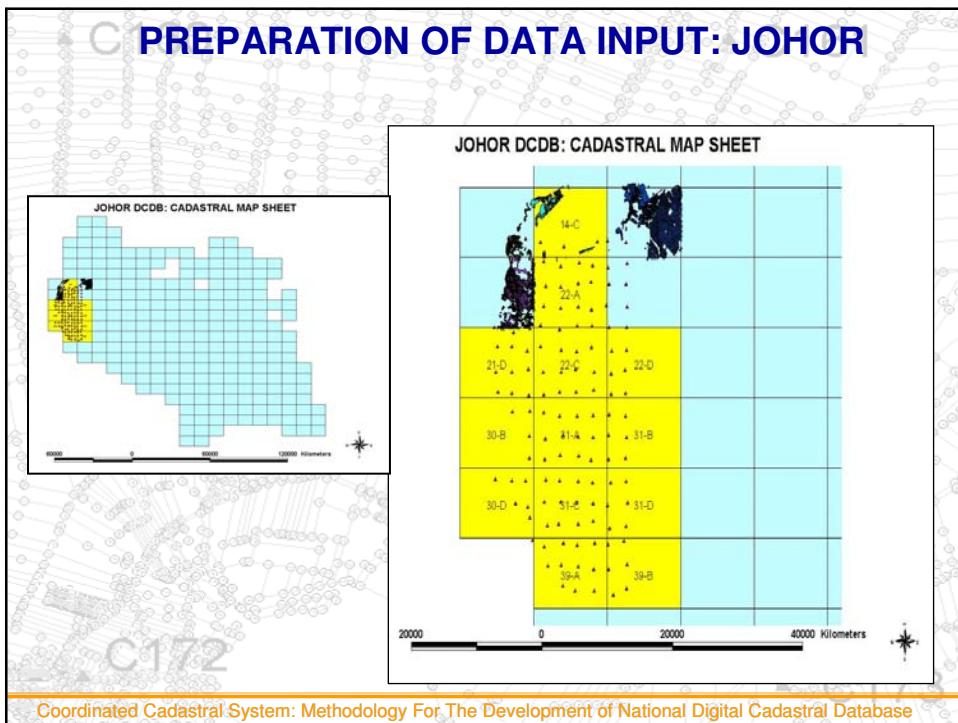
PREPARATION OF DATA INPUT: MELAKA

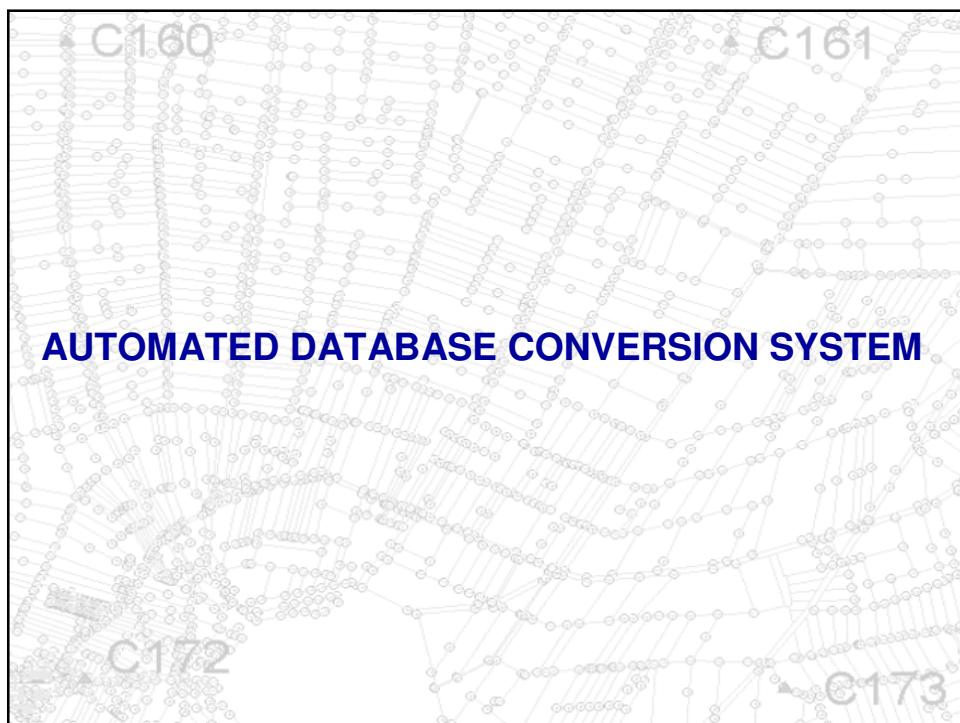
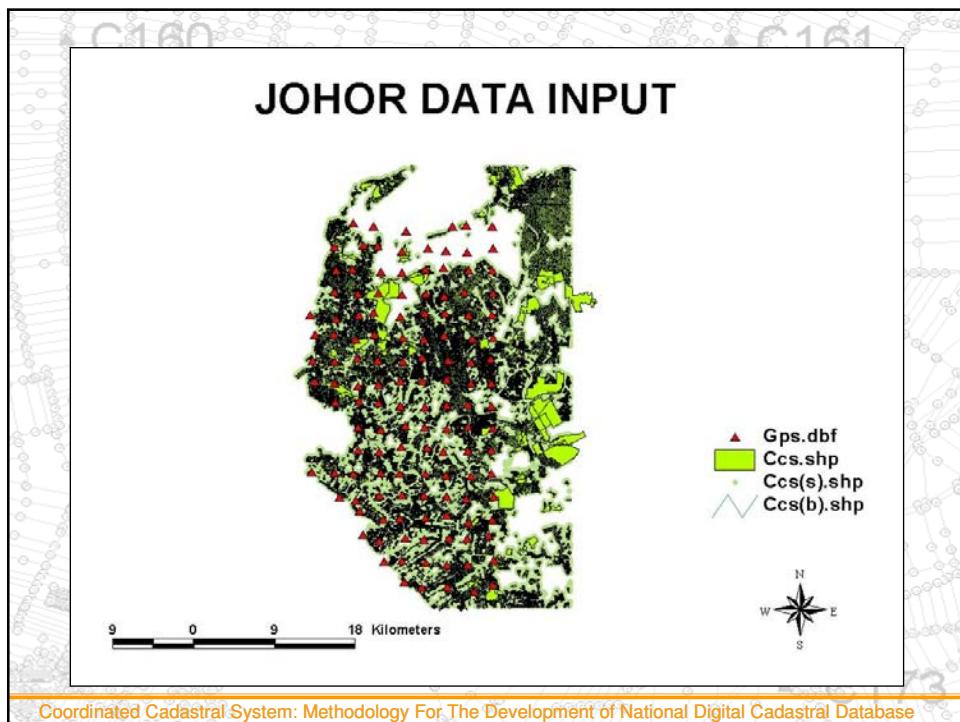


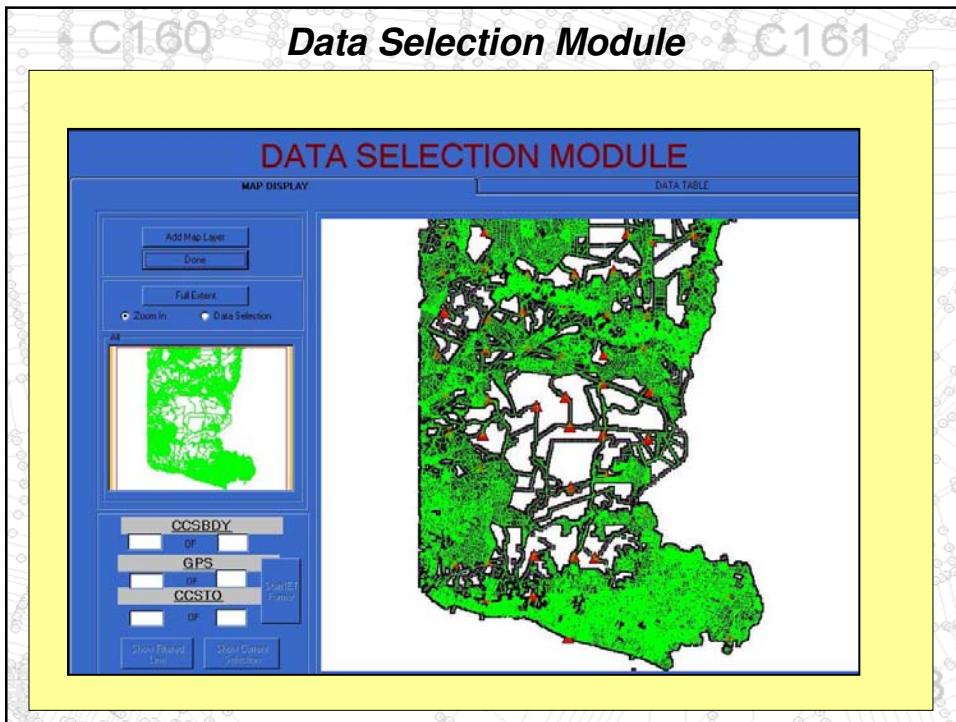
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PREPARATION OF DATA INPUT: JOHOR



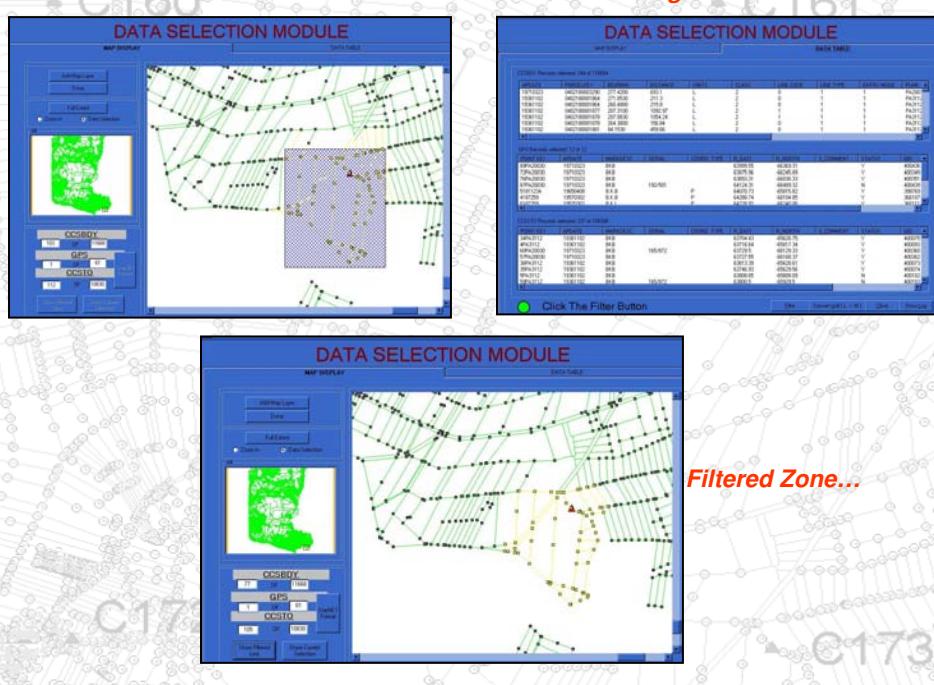


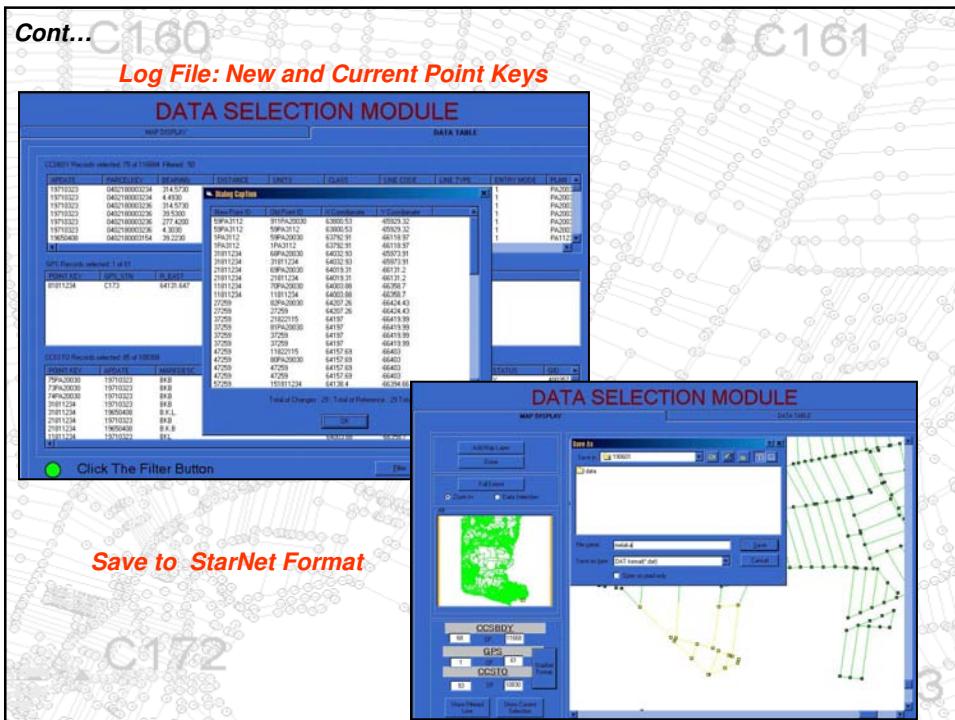


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Selection Process...

Filtering Process...

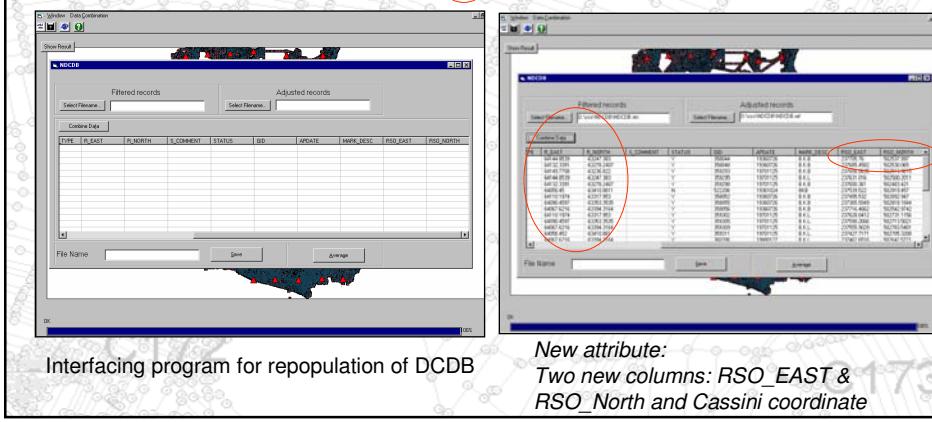


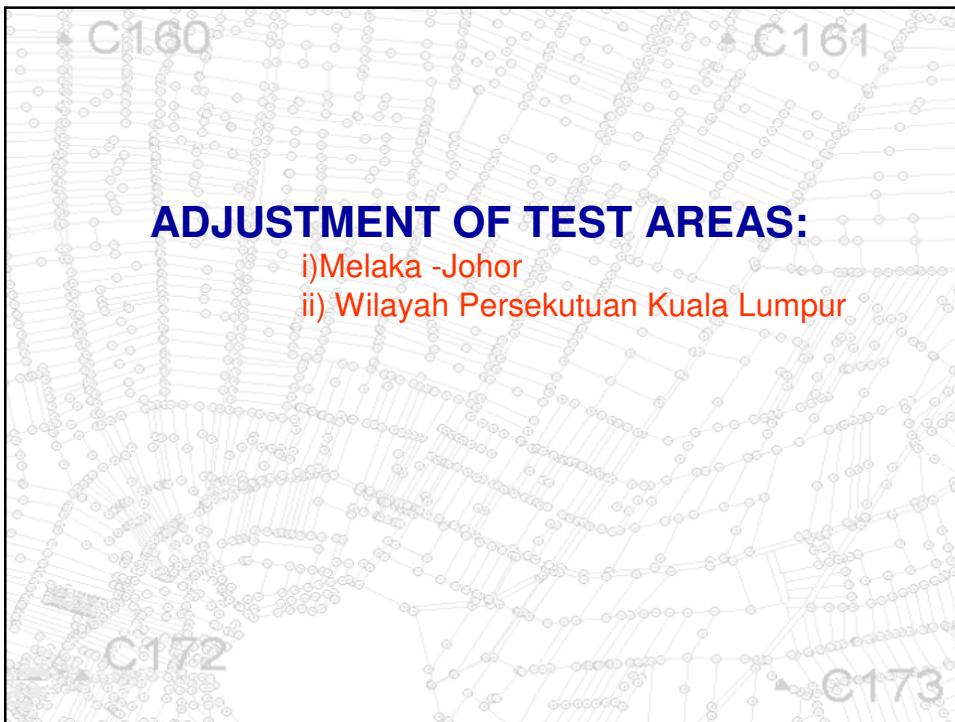


Attributes of Ecstos.ashp		Shape	Order	Mark desc	Serial	Contd by	Start at	End at	S_comment	Status
Point	26181300	13930728	8 K.B.	P	0.0000	0.0000	Y	Y	36012	
Point	70181300	13930728	8 K.B.	166/139	P	0.0000	0.0000	Y	Y	36030
Point	77181300	13930728	8 K.B.	166/139	P	0.0000	0.0000	Y	Y	36030
Point	77181300	13930728	8 K.B.	166/139	P	0.0000	0.0000	Y	Y	36034
Point	82181300	13930728	8 K.B.	159/432	P	0.0000	0.0000	Y	Y	36034
Point	82181300	13930728	8 K.B.	159/432	P	0.0000	0.0000	Y	Y	36034
Point	82181300	13930728	8 K.B.	159/432	P	0.0000	0.0000	Y	Y	36034
Point	81820305	18890728	7 K.L.	P	0.0000	0.0000	Y	Y	36021	
Point	81820305	18910119	K.L.	P	0.0000	0.0000	Y	Y	36021	
Point	418370	13930819	8 K.B.	P	0.0000	0.0000	Y	Y	36030	
Point	418370	13930819	8 K.B.	P	0.0000	0.0000	N	N	36112	
Point	34943665	13931024	8 K.B.	166/182	P	640468.68	63696.52	Y	Y	52218
Point	179A865	13931024	8 K.B.	P	639742.97	53692.75	Y	Y	52218	
Point	179A865	13931024	8 K.B.	P	639742.97	53692.75	N	N	52220	
Point	179A865	13931024	8 K.B.	P	639816.63	63530.35	N	N	52220	
Point	171181300	13930726	8 K.B.	156/732	P	0.0000	0.0000	Y	Y	36030
Point	171181300	13930726	8 K.B.	156/732	P	0.0000	0.0000	N	N	36030
Point	181181300	13930726	8 K.B.	P	0.0000	0.0000	Y	Y	36030	
Point	201181300	13930726	8 K.B.	P	0.0000	0.0000	Y	Y	36034	
Point	211181300	13930726	8 K.B.	P	0.0000	0.0000	Y	Y	36034	
Point	211181300	13930726	8 K.B.	P	0.0000	0.0000	N	N	36034	
Point	601820307	13930726	8 K.B.	P	0.0000	0.0000	Y	Y	36030	
Point	591820307	13930726	8 K.B.	P	0.0000	0.0000	N	N	36030	
Point	591820307	13930726	8 K.B.	P	0.0000	0.0000	N	N	36030	
Point	591820307	13930726	8 K.B.	P	0.0000	0.0000	N	N	36030	
Point	591820307	13930726	8 K.B.	157/494	P	0.0000	0.0000	Y	Y	36030

Populasi-PDUK

Original Boundary file
(Cassini coordinate only for
selected boundary stones)

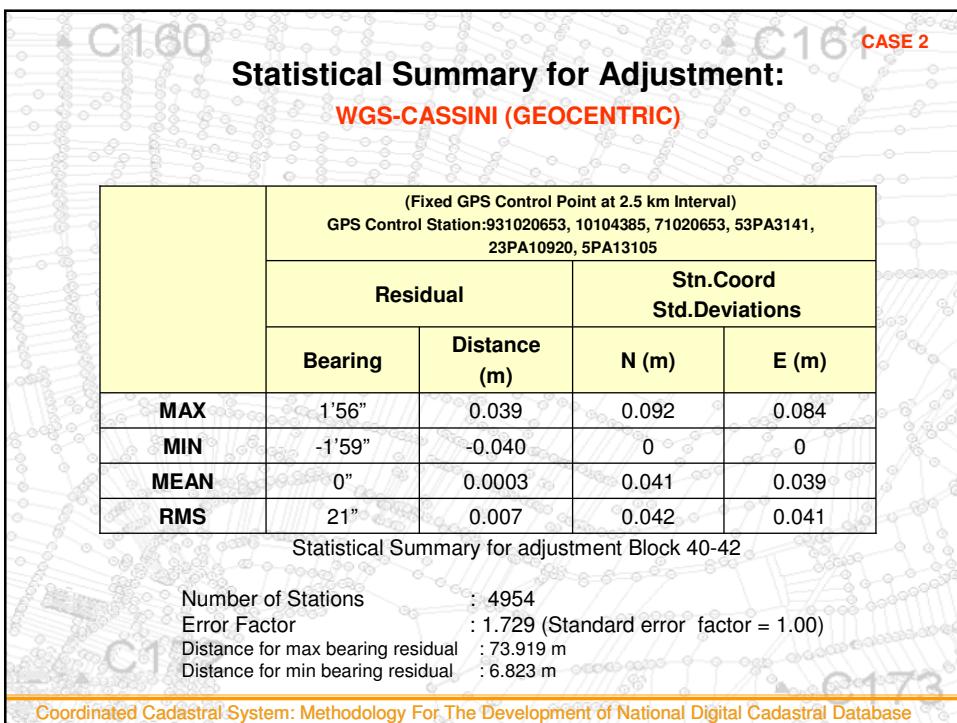
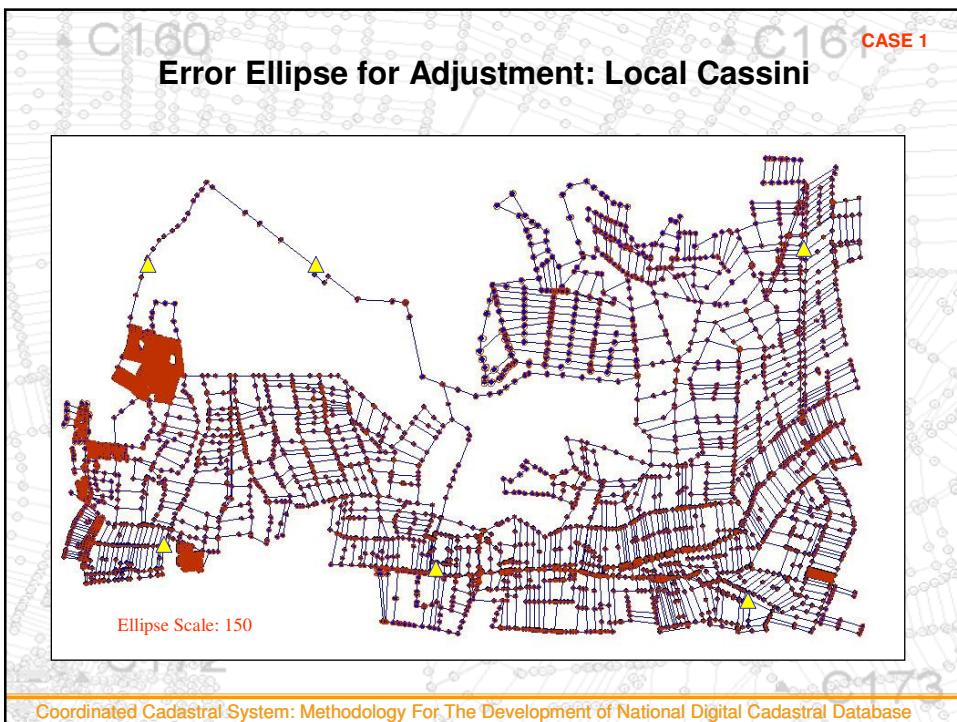




Statistical Summary for Adjustment: WGS-MRT-RSO-CASSINI				CASE 1		
	(Fixed GPS Control Point at 2.5 km Interval) GPS Control Station: 931020653, 10104385, 71020653, 53PA3141, 23PA10920, 5PA13105				C161 C173	
	Residual		Stn.Coord Std.Deviations			
	Bearing	Distance (m)	N (m)	E (m)		
	MAX	1'54"	0.039	0.092	0.084	
	MIN	-2'00"	-0.040	0	0	
	MEAN	0"	0	0.041	0.039	
	RMS	21"	0.013	0.042	0.041	

Statistical Summary for adjustment Block 40-42

Number of Stations	: 4954
Error Factor	: 1.729 (Standard error factor = 1.00)
Distance for max bearing residual	: 73.919 m
Distance for min bearing residual	: 42.714 m

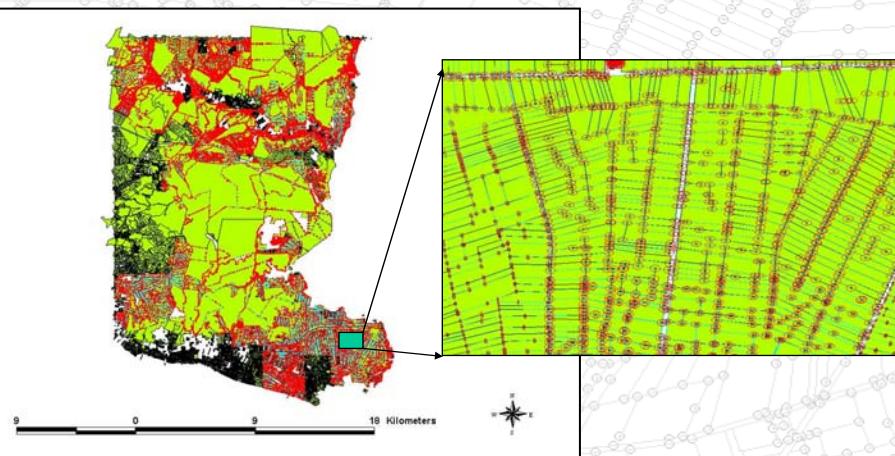


Error Ellipse for Adjustment: Geocentric Cassini



Coordinated Cadastral System: Methodology For The Development of National Digital Cadastral Database

COMBINED ADJUSTMENT RESULTS OF MELAKA CADASTRAL NETWORK



Total of Adjusted Boundary Marks: 62411

Coordinated Cadastral System: Methodology For The Development of National Digital Cadastral Database

Statistics of the Melaka CCS Adjustment

Blk No.	No. of Stn.	No. of Obs.	No. of Unknowns	No. of Redundant Obs.	No. of GPS Stn.	Error Factor
Blk 1-6	7643	20832	15258	5574	13	1.8
Blk 7-8	3384	8318	6756	1562	6	1.4
Blk 11-12	3741	11007	7470	3537	6	1.7
Blk 15-16	5595	16263	11178	5085	6	1.7
Blk 17	2728	8074	5448	2626	4	1.7
Blk 18	3229	8228	6452	1776	3	1.7
Blk 20-27	3608	9870	7198	2672	9	1.7
Blk 30	1546	4044	3084	960	4	1.7
Blk 32-33	1997	5132	3978	1154	7	1.6
Blk 34-37	8139	24251	16256	7995	11	1.7
Blk 40-42	4954	14405	9896	4506	6	1.7
Blk 43	4510	12699	9010	3689	5	1.8
Blk 45	3698	9583	7392	2191	4	1.8
Blk 47	3034	8388	6062	2326	4	1.7

Note : Standard error factor is 1.0

Comparison Between Two Difference Cassini System

	BEARING RESIDUALS							
	GEODETIC				GEOCENTRIC			
	MAX	MIN	MEAN	RMS	MAX	MIN	MEAN	RMS
Blk 1-6	2'1"	-2'1"	-1"	22"	2'00"	-2'01"	0"	21"
Blk 7-8	1'46"	-1'34"	0"	14"	1'42"	-1'34"	0	14"
Blk 11-12	2'00"	-1'53"	0"	17"	1'56"	-1'54"	0"	21"
Blk 15-16	1'59"	-1'58"	-1"	16"	2'01"	2'00"	0"	17"
Blk 17	1'57"	-1'59"	-1"	20"	1'57"	-1'56"	1"	19"
Blk 18	2'00"	-1'56"	-1"	20"	1'54"	-1'56"	0"	19"
Blk 20-27	2'02"	-2'03"	-1"	21"	1'59"	-2'00"	0"	21"
Blk 30	1'57"	-2'00"	0"	20"	1' 57"	-2' 00"	0"	20"
Blk 32-33	1'46"	-1'57"	0"	20"	1'44"	-1'57"	0"	19"
Blk 34-37	2'03"	-1'59"	0"	22"	2'01"	-2'01"	0"	21"
Blk 40-42	1'54"	-2'00"	0"	21"	1'56"	-1'59"	0"	21"
Blk 43	2'01"	-2'01"	1"	25"	1'59"	-2'01"	1"	25"
Blk 45	2'00"	-1'55"	0"	28"	1'59"	-1'55"	0"	21"
Blk 47	1'54"	-1'58"	0"	24"	1'52"	-1'57"	0"	24"

Comparison Between Two Difference Cassini System

	DISTANCE RESIDUALS							
	GEODETIC				GEOCENTRIC			
	MAX	MIN	MEAN	RMS	MAX	MIN	MEAN	RMS
Blk 1-6	0.040	-0.048	0	0.008	0.040	0.040	0	0.008
Blk 7-8	0.033	-0.033	0	0.005	0.033	-0.033	0	0.005
Blk 11-12	0.040	-0.040	0	0.008	0.036	-0.036	0	0.007
Blk 15-16	0.040	-0.039	0	0.007	0.035	0.037	0	0.006
Blk 17	0.038	-0.040	0	0.009	0.039	-0.040	0	0.008
Blk 18	0.036	-0.033	0	0.007	0.046	0.038	0	0.007
Blk 20-27	0.039	-0.040	0	0.007	0.040	-0.040	0	0.007
Blk 30	0.040	-0.033	0	0.007	0.040	-0.033	0	0.007
Blk 32-33	0.040	-0.041	0	0.007	0.040	-0.040	0	0.007
Blk 34-37	0.037	-0.036	0	0.007	0.040	-0.040	0	0.008
Blk 40-42	0.039	-0.040	0	0.013	0.039	-0.040	0	0.007
Blk 43	0.040	-0.039	0	0.008	0.040	-0.039	0	0.008
Blk 45	0.039	-0.040	0	0.008	0.039	-0.039	0	0.008
Blk 47	0.038	-0.037	0	0.007	0.038	-0.037	0	0.008

Comparison Between Two Difference Cassini System

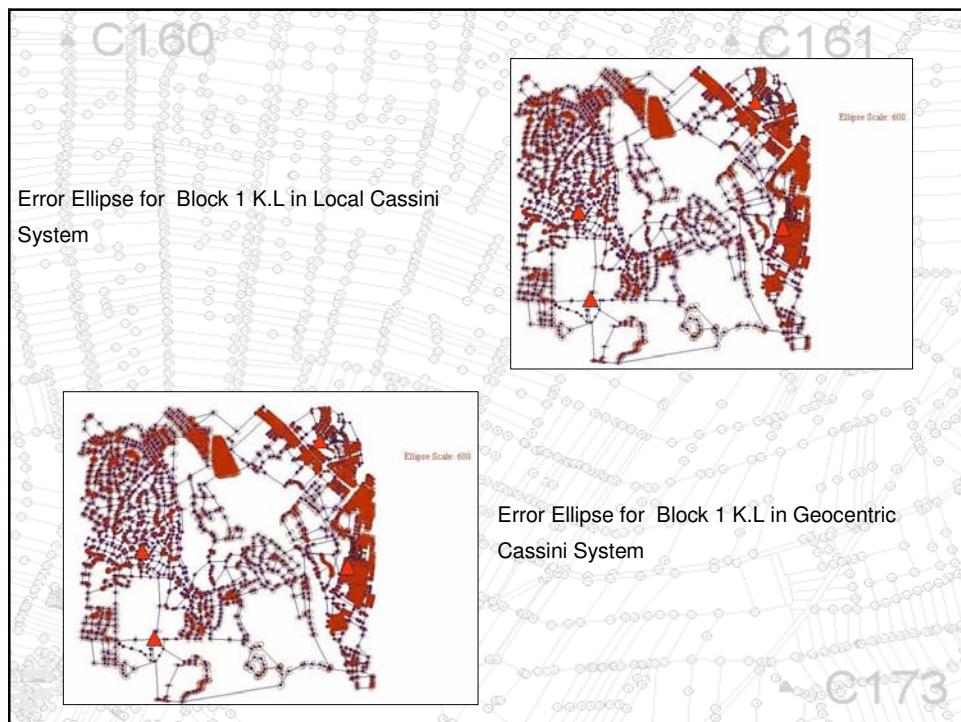
	STATION COORDINATE STANDARD DEVIATIONS NORTH/SOUTH COMPONENT							
	GEODETIC				GEOCENTRIC			
	MAX	MIN	MEAN	RMS	MAX	MIN	MEAN	RMS
Blk 1-6	0.150	0	0.070	0.075	0.140	0	0.075	0.083
Blk 7-8	0.151	0	0.076	0.084	0.140	0	0.075	0.083
Blk 11-12	0.103	0	0.044	0.048	0.101	0	0.042	0.046
Blk 15-16	0.139	0	0.065	0.069	0.141	0	0.065	0.069
Blk 17	0.092	0	0.044	0.045	0.086	0	0.039	0.040
Blk 18	0.104	0	0.056	0.059	0.104	0	0.056	0.059
Blk 20-27	0.151	0	0.059	0.062	0.139	0	0.055	0.057
Blk 30	0.120	0	0.064	0.067	0.120	0	0.064	0.067
Blk 32-33	0.150	0	0.048	0.054	0.141	0	0.048	0.053
Blk 34-37	0.141	0	0.046	0.048	0.137	0	0.046	0.049
Blk 40-42	0.092	0	0.041	0.042	0.092	0	0.041	0.042
Blk 43	0.090	0	0.043	0.044	0.090	0	0.042	0.044
Blk 45	0.081	0	0.053	0.054	0.079	0	0.053	0.054
Blk 47	0.093	0	0.044	0.047	0.092	0	0.044	0.047

Comparison Between Two Difference Cassini System

	STATION COORDINATE STANDARD DEVIATIONS EAST/WEST COMPONENT							
	GEODETIC				GEOCENTRIC			
	MAX	MIN	MEAN	RMS	MAX	MIN	MEAN	RMS
Blk 1-6	0.139	0	0.070	0.063	0.138	0	0.063	0.067
Blk 7-8	0.148	0	0.076	0.086	0.147	0	0.075	0.085
Blk 11-12	0.104	0	0.047	0.050	0.101	0	0.045	0.049
Blk 15-16	0.148	0	0.072	0.076	0.152	0	0.072	0.076
Blk 17	0.084	0	0.042	0.043	0.079	0	0.039	0.040
Blk 18	0.095	0	0.050	0.054	0.095	0	0.050	0.054
Blk 20-27	0.123	0	0.061	0.063	0.147	0	0.055	0.057
Blk 30	0.131	0	0.056	0.059	0.131	0	0.056	0.059
Blk 32-33	0.124	0	0.046	0.050	0.112	0	0.046	0.050
Blk 34-37	0.133	0	0.047	0.049	0.132	0	0.047	0.049
Blk 40-42	0.084	0	0.039	0.041	0.084	0	0.039	0.041
Blk 43	0.113	0	0.049	0.053	0.112	0	0.050	0.055
Blk 45	0.108	0	0.063	0.065	0.106	0	0.063	0.065
Blk47	0.101	0	0.046	0.050	0.101	0	0.045	0.050

Statistical Summary For W.P.K.L Adjustments Results (Control Stations at 500 m spacing)

	Results Statistic							
	CASE 1				CASE 2			
	GEODETIC				GEOCENTRIC			
	MAX	MIN	MEAN	RMS	MAX	MIN	MEAN	RMS
Bearing	40"	-39"	0"	6"	40"	-39"	0"	6"
Distance (m)	0.004	-0.004	0.001	0.001	0.004	-0.004	0.001	0.001
Std. Dev Coordinate (N/S) (m)	0.021	0.001	0.010	0.011	0.021	0.001	0.010	0.011
Std. Dev Coordinate (E/W) (m)	0.024	0.001	0.010	0.011	0.023	0.001	0.010	0.011



Comparison of Adjustment Statistics Between 2.5 km and 0.5 km Control Spacing (Geocentric Case)								
	Results Statistic							
	CASE 1				CASE 2			
	GEODETIC				GEOCENTRIC			
	MAX	MIN	MEAN	RMS	MAX	MIN	MEAN	RMS
Bearing	40"	-39"	0"	6"	40"	-39"	0"	6"
Distance (m)	0.004	-0.004	0.001	0.001	0.004	-0.004	0.001	0.001
Std. Dev Coordinate (N/S) (m)	0.021	0.001	0.010	0.011	0.021	0.001	0.010	0.011
Std. Dev Coordinate (E/W) (m)	0.024	0.001	0.010	0.011	0.023	0.001	0.010	0.011

	2.5 km GPS Control				0.5 km GPS Control			
	MAX	MIN	MEAN	RMS	MAX	MIN	MEAN	RMS
	Bearing	2' 01"	-2' 01"	0"	20"	40"	39"	0"
Distance (mm)	40	-40	0	7	4	-4	1	1
Std. Dev. N/S (mm)	147	0	53	56	21	1	10	11
Std. Dev. E/W (mm)	152	0	56	57	23	1	10	11

SUMMARY

- ✓ Data screening and cleaning is essential since **outliers** exist in the data input. Manual editing is needed in order to run the adjustment ~ **time consuming, tedious and challenging task**.
- ✓ Data selection and Adjustment process greatly depend on the “ cleanliness” of the data input.
- ✓ For the implementation of CCS, the adjustment progresses coherently with
 - Outliers encountered during adjustment process
 - The availability of number of software license
- ✓ The adjustment results depend on several factors:
 - I) Control station
 - ii) Accuracy of the cadastral survey
 - iii) Block size
 - iv) Number of boundary mark
 - v) Density of the cadastral lot
- ✓ Adjustment results show that the residuals and standard deviations for bearing, distance and coordinate are in tolerance.
- ✓ GPS station at 0.5km and 2.5km spacing are sufficient to be used in providing control for urban and rural cadastral networks, respectively.

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RECOMMENDATIONS

- ✓ To develop a comprehensive data integrity and management module in the existing Cadastral Management System comprising:
 - Revision of DCDB development process especially during data entry.
 - The creation of new layer in the DCDB for **Cadastral Control** and **Connection Line**
- ✓ To establish Cadastral Control Infrastructure with the following consideration:
 - GPS control in a grid format for better network geometry during adjustment process.
 - GPS station at 0.5km and 2.5 km spacing are sufficient to be used in providing control for urban and rural cadastral networks, respectively.
 - To facilitate the current GPS technologies such as Real-Time Kinematic GPS and Virtual GPS reference System in the Cadastral Control Infrastructure development process
- ✓ To further refine the present Automated Database Conversion System
- ✓ To restructure boundary mark file in order to populate the DCDB with new adjusted survey accurate coordinates

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IMPACT OF GDM2000 ON CCS IMPLEMENTATION (Technical Views)

