MODELING OF SHEAR BOND IN COMPOSITE SLAB USING INTERFACE ELEMENT

TOH YIK FUNG

A project report submitted in partial fulfilment of the requirements for the award of the degree of Master of Engineering (Civil-Structure)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > JUNE 2006

In loving appreciation of my dear family and friends May God shower his blessing on you. Love you forever...

ACKNOWLEDGEMENT

I would like to thank all the parties who have given the co-operation to me in writing this project report. I am sincerely greatful to my supervisor, Dr. Redzuan Abdullah for his continuous support and guidance in this project. He has set a high standard for the conduct of this study and his valuable suggestions and guidance have provided me the motivation needed to complete this project report.

I would like to thank the staffs in Faculty of Civil Engineering for their help in the completion of this project.

I thank my family and friends for their supports and encouragement. Their encouragement provided the often-needed motivation and inspirations for me to push through the hard times. I would also like to acknowledge the contributions of those who have helped either directly or indirectly in the completion of this project.

ABSTRACT

Composite slabs utilizing cold-formed steel decks are widely used in construction industries. The behavior of the slabs is governed by shear bond between concrete and the steel deck. Modeling the shear bond correctly is critical in a numerical study of composite slab. This paper pertains with finite element study to model the shear bond using an interface element available in LUSAS. The interface element normally used for modeling delamination of plane and crack propagation of a composite material was utilized to study the shear bond behavior in composite slab. Experimental data from published literatures was used to verify the finite element analysis results. The results show that with proper assignment of material parameters to the interface elements, the shear bond in the composite slab can be modeled correctly.

ABSTRAK

Penggunaan dek keluli dalam papak rencam amat luas digunakan dalam industri pembinaan. Kelakuan papak rencam bergantung kepada kekuatan interaksi di antara konkrit dan dek keluli. Permodelan interaksi antara konkrit dengan dek keluli adalah kritikal dalam kajian 'numerical' papak rencam. Kertas kerja ini membincangkan permodelan interaksi di antara konkrit dan dek keluli dengan menggunakan "interface element" dalam LUSAS dan analisis papak rencam dengan kaedah unsur terhingga. Dalam projek ini, "interface element" yang biasanya digunakan untuk pemodelan dua lapisan nipis dan kewujudan retak dalam bahan rencam telah digunakan untuk memodelkan interaksi konkrit dan dek keluli dalam papak rencam. Data ujikaji daripada laporan pengkaji yang lepas telah digunakan untuk mengesahkan hasil analisis dengan kaedah unsur terhingga. Hasil kajian ini menunjukkan bahawa dengan menggunakan nilai cirri yang sesuai untuk "interface element", kekuatan interaksi papak rencam boleh dimodelkan dengan tepat.

TABLE OF CONTENTS

DECLARATION	ii
DEDICATION	iii
ACKNOWLEDGEMENT	iv
ABSTRACT	V
ABSTRAK	vi
TABLE OF CONTENT	vii
LIST OF TABLES	ix
LIST OF FIGURES	X
LIST OF SYMBOLS	xii

CHAPTER	TITLE	PAGE

INTRODUCTION 1

1.1	General	1
1.2	Objectives	2
1.3	Scope of Works	2
1.4	Organization of This Report	3

2	LITE	LITERATURE REVIEW		
	2.1	Composite Slab Behavior	4	
	2.2	Modeling of Composite Slabs	7	

2.3	Composite Delamination using Interface Elements		
	in Lusas Modeller		9
	2.3.1	.3.1 Fracture Modes	
	2.3.2	Interface Material Properties	10

3 FE MODELING AND ANALYSIS

3.1	Introduction		12
3.2	Development of the FE model		13
	3.2.1	3.2.1 Structural Model	
	3.2.2 Concrete Properties3.2.3 Steel Properties3.2.4 Interface Element Properties		17
			18
			19
		3.2.4.1 Fracture Energy	20
		3.2.4.2 Relative Displacemen	21
		3.2.4.3 Initiation Stress	22
	3.2.5	Nonlinear Analysis Control	23

4 **RESULTS AND DISCUSSIONS**

4.1	Introduction	26
4.2	Analysis Results	26
4.3	Initiation Stress Versus Slenderness	36
4.4	Load Versus Slenderness	37
4.5	Discussions	38

5 CONCLUSIONS AND RECOMMENDATIONS

5.1	Conclusions	41
5.2	Recommendations	42

REFERENCES

LIST OF TABLES

TABLE NO.

TITLE

PAGE

3.1	Composite slab geometry.	
3.2	Concrete properties (Abdullah, 2004).	18
3.3	Steel deck properties (Abdullah, 2004).	19
3.4	Mode 2 failure interface element properties.	23
4.1	Interface material properties.	33
4.2	Comparison of maximum load obtained from LUSAS	
	and average Test results.	34
4.3	Comparison of maximum displacement obtained from	
	LUSAS and average Test results.	35
4.4	Mode 2 failure interface element properties obtained by	
	Udin (2006).	39

LIST OF FIGURES

FIGURE NO.

TITLE

PAGE

Modes of failure of composite slab (Johnson, 1994). 4		
Horizontal shear failure (Abdullah, 2004).	5	
Example of shear connection devices in composite slabs.		
(a) Frictional interlock. (b) Mechanical interlock. (c) End		
Anchorage combined with mechanical interlock		
(Addullah, 2004; Eurocode 4, 2001).	6	
Three types of fracture modes.	9	
Delamination damage model.	10	
Small scale test setup in experimental work done by		
Abdullah (2004), (a) Side view. (b) Cross Section.		
(c) Isometric view.	13	
Composite slab for 3VL16-4-7.5.		
(a) One foot strip. (b) Cross section. (c) One-quarter model	.15	
Solid stress element (HX20) for concrete and steel deck		
and interface element (IS16) for shear bond.	15	
FE model. (a) Schematic side view. (b) Cross section.		
(c) Isometric view.	16	
Stress-strain curve for concrete (N. E. Shanmugam, 2002).	17	
Stress-strain curve for steel deck.	19	
Analysis results with several fracture energy values.	21	
Analysis results with different relative displacement values	.22	
Nonlinear analysis result and test result for 3VL16-4-7.5.	24	
	 Horizontal shear failure (Abdullah, 2004). Example of shear connection devices in composite slabs. (a) Frictional interlock. (b) Mechanical interlock. (c) End Anchorage combined with mechanical interlock (Addullah, 2004; Eurocode 4, 2001). Three types of fracture modes. Delamination damage model. Small scale test setup in experimental work done by Abdullah (2004), (a) Side view. (b) Cross Section. (c) Isometric view. Composite slab for 3VL16-4-7.5. (a) One foot strip. (b) Cross section. (c) One-quarter model Solid stress element (HX20) for concrete and steel deck and interface element (IS16) for shear bond. FE model. (a) Schematic side view. (b) Cross section. (c) Isometric view. Stress-strain curve for concrete (N. E. Shanmugam, 2002). Stress-strain curve for steel deck. Analysis results with several fracture energy values. Analysis results with different relative displacement values 	

3.10	Modified arc length load incrementation for a one degree	
	of freedom response (Lusas Theory Manual).	24
3.11	Test result and Lusas analysis results with and without	
	geometric nonlinearity.	25
4.1	Converting the point load in the model to uniform load	
	(Udin, 2006).	28
4.2	Comparison between FE analysis results and test data	
	conducted by Abdullah (2004).	30-32
4.3	Initiation stress versus slenderness.	36
4.4	Maximum load versus slenderness.	37
4.5	Horizontal shear stress versus slip for 3VL16 slabs	
	Calculated from small scale test data using force	
	equilibrium method (Abdullah, 2004).	40

LIST OF SYMBOLS

d	-	midspan displacement
ds	-	steel deck depth
L	-	total slab span
L _s	-	shear span
М	-	ultimate moment
Р	-	point load
t _c	-	concrete thickness
W	-	uniform load.

CHAPTER 1

INTRODUCTION

1.1 General

Composite slab is a slab system comprising of normal weight or lightweight structural concrete placed permanently over cold-formed steel deck in which the steel deck performs dual roles of acting as a form for the concrete during construction and as positive reinforcement for the slab during service ASCE (1992).

Cold-formed steel deck composite slabs become common in today construction industry due to several economic advantages. These include elimination of the use of formworks, ease in handling and placing the steel deck sheets, and the use of steel deck as the reinforcement for slab. The strength of the composite diaphragms is controlled by one of the three limit states, diagonal tension failure of the concrete, edge connector failure or shear transfer mechanism failure (Porter and Ekberg, 1977). Composite action of the slab is very much depending on the shear bond at the interface to eliminate end slip.

The interaction between the composite interfaces is very complex. The analysis and design procedures available today have to rely on test data to account the interaction parameters. Design and analysis based on test data is expansive. Numerical analysis such as by FE method can be used to replace the test data.

1.2 Objectives

The objectives of the study are:

- a) To develop a finite element model for composite slab where interface elements are used to represent the shear bond between the concrete and the steel deck.
- b) To perform non-linear FE analyses of composite slab to determine a suitable attribute of the interface element so that the element can be used to model the shear bond behavior correctly.

1.3 Scope of Works

The scopes of works for this research are:

- The FE model is in 3-Dimensional.
- To use interface elements for modeling the shear bond.
- This study is limited to simply-support slab with two-point loads only.
- Material non linearity is included in the analysis.

1.4 Organization of This Report

This project is organized in five chapters. Following Chapter 1, composite slab behavior, modeling of composite slabs by others researchers and introduction of interface element in LUSAS Modeller are described in Chapter 2. The FE modeling and analysis is presented in Chapter 3. The results of the analysis are discussed in Chapter 4. Lastly, the conclusions and recommendations are made in Chapter 5.

REFERENCES

ASCE (1992). Standard for the Structural Design of Composite Slabs. ANSI/AASCE 3-91. American Society of Civil Engineers, New York.

Bode, H., and Sauerborn, I. (1992). "Modern Design Concept for Composite Slabs with Ductile Behaviour." *Proceedings of an Engineering Foundation Conference on Composite Construction in Steel and Concrete II*, American Society of Civil Engineers, 125-141.

- Daniels, B. J. (1988). "Shear Bond Pull-out Tests for Cold-Formed-Steel Composite Slabs." ICOM-Construction Metallique, Department de Genie Civil,Ecole Polytechnique Federale de Lausanne.
- Daniels, B. J., and Crisinel, M. (1993a). "Composite Slab Behavior and Strength Analysis. Part I: Calculation Procedure." *Journal of Structural Engineering*, 119(1-4), 16-35.
- Daniels, B. J., and Crisinel, M. (1993b). "Composite Slab Behavior and Strength Analysis. Part II: Comparison With Test Results And Parametric Analysis." *Journal of Structural Engineering*, 119(1-4), 36-49.
- Eurocode 4. (2001). Design of Composite Steel and Concrete Structures Part 1.1: General Rule and Rules for Buildings. ENV 1994-1-1:1992, European Committee for Standardization, Brussels.

Johnson, R. P. (1994), Composite Structures of Steel and Concrete Volume 1Beams, Slabs, Columns and Frames for Buildings. Second Edition.

Lusas Examples Manual, Version 13, United Kingdom.

Lusas Modeller User Manual, Version 13, United Kingdom.

Lusas Theory Manual, Version 13, United Kingdom.

- Luttrell, L. D. (1987). "Flexural Strength of Composite Slabs." Composite Steel Structures-Advances, Design and Construction, Narayanan, R., Ed., Elsevier, London, 106-115.
- Nik Mat (2006). "Modeling of Shear Bond in Composite Slab Using Interface Element." Bachelor Degree Thesis, Faculty of Civil Engineering, University of Technology Malaysia, Skudai, Johor.

Porter, M. L., and Ekberg, C. E. (1977). "Behavior of Steel-Deck-Reinforced Slabs." *Journal of the Structural Division*, Volume 103, No. 3, March 1977, 663-677.

Redzuan Abdullah (2004). "Experimental Evaluation and Analytical Modeling ofShear Bond in Composite Slabs," Phd Dissertation, Virginia PolytechnicInstitute and State University, Blacksburg.

Seleim, S. S., and Schuster, R. M. (1985). "Shear-Bond Resistance of Composite Deck- Slabs." *Canadian Journal of Civil Engineering*, 12(316-323).

Shanmugam, N. E. (2002), Finite element modeling of double skin composite slabs. Department of Civil Engineering, National University of Singapore, Singapore.

Veljkovic, M. (1995). "Longitudinal Shear Capacity of Composite Slabs." NordicSteel Construction Conference '95, Malmo, Sweden.

Veljkovic, M. (1996a). "Behaviour and Resistance of Composite Slabs," Phd Thesis, Lulea University of Technology, Lulea, Sweden.

Widjaja, B. R. (1997). "Analysis and Design of Steel Deck-Concrete CompositeSlabs," Phd Dissertation, Virginia Polytechnic Institute and State University, Blacksburg.

Yam, Lloyd C. P., "Design of Composite Steel-Concrete Structures." Survey University Press, London, 1981.