FLEXURAL STRENGTH OF FIBRE REINFORCED CONCRETE UNDER ELEVATED TEMPERATURE

CHEONG LIP SING

A project report submitted in partial fulfillment of the requirements for the award of the degree of Master of Engineering (Civil – Structure)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > JANUARY 2015

Specially dedicated to my beloved wife, family and friends.

ACKNOWLEDGEMENT

Firstly, I would like to express my deepest gratitude to my family for their continuous support and encouragement, especially my beloved wife, Eileen Lee, who has been patient and understanding throughout the course of my studies.

I would also like to thank my supervisor, Dr. Izni Syahrizal bin Ibrahim, for his efforts and continuous guidance, advice and support, which has been a major contribution to the completion of this study.

Finally, my appreciation to all the wonderful friends I have made through the course of my study, whose friendship I truly cherish.

ABSTRACT

The study observes and examines the effects of elevated temperature on the fibre reinforced concrete's ability to withstand flexure loading. The fibres used are steel, synthetic and kenaf fibre. Addition of fibres into reinforced concrete may increase the ductility and cracking resistance. To explore the effects of elevated temperature, a furnace was used to heat the prepared specimens to 400°C, 600°C and 800°C. Steel fibre reinforced concrete was found to maintain some amount of flexure capacity even at 800°C, due to the high melting point, 1300°C, of the steel fibres. However, not all properties of concrete may improve with the addition of fibres. High temperature that may be due to structure usage or accidents is a concern that has to be considered when choosing the right material for reinforcing the concrete matrix. By understanding the effects of elevated temperature on the various types of fibre reinforced concrete, a better choice of fibres can be made for different structural usage.

ABSTRAK

Kajian ini memerhati dan mengkaji kesan suhu tinggi pada gentian diperkukuhkan keupayaan konkrit untuk menahan lenturan loading. Gentian yang digunakan adalah keluli, sintetik dan gentian kenaf. Penambahan gentian ke dalam konkrit bertetulang boleh meningkatkan kemuluran dan retak rintangan. Untuk meninjau kesan suhu tinggi, gentian telah digunakan untuk memanaskan spesimen bersedia untuk 400°C, 600°C dan 800°C. Gentian keluli konkrit bertetulang didapati mengekalkan sedikit kapasiti lenturan walaupun pada 800°C, disebabkan takat lebur yang tinggi, 1300°C, pada gentian keluli. Walau bagaimanapun, tidak semua sifatsifat konkrit boleh bertambah baik dengan penambahan gentian. Suhu tinggi yang mungkin disebabkan oleh penggunaan struktur atau kemalangan adalah satu kebimbangan yang perlu dipertimbangkan apabila memilih bahan yang tepat untuk mengukuhkan matriks konkrit. Dengan memahami kesan suhu tinggi mengenai pelbagai jenis gentian konkrit bertetulang, pilihan gentian yang lebih baik boleh dibuat bagi penggunaan struktur yang berbeza.

LIST OF CONTENTS

CHAPTER			TITLE	PAGE	
	DECLARATION DEDICATION			ii	
				iii	
	ACK	ACKNOWLEDGEMENT ABSTRACT			
	ABS				
	ABS	vi			
	LIST	r of cc	ONTENTS	vii	
	LIST	Г OF ТА	BLES	ix	
	LIST OF FIGURES			Х	
	LIST	Г <mark>О</mark> F АР	PENDICES	xiii	
1	INTRODUCTION			1	
	1.1	Gener	al	1	
	1.2	Proble	em Statement	2	
	1.3	Objec	tives	2	
	1.4	Scope		3	
2	LITERATURE REVIEW			4	
	2.1	Reinfo	orced Concrete	4	
	2.2	Fibre	Reinforced Concrete	5	
		2.2.1	Steel Fibre	9	
		2.2.2	Synthethic Fibre	11	
		2.2.3	Kenaf Fibre (Natural Fibre)	13	
	2.3	Concr	ete Under Elevated Temperatu	re 15	

	2.4	Fibre	Reinforced Concrete Under	
		Eleva	ted Temperature	19
3	MET	rhodo	LOGY	22
	3.1	Mater	rials Preparation	22
	3.2	Samp	les Preparation	26
		3.2.1	Mixing, Casting and Curing	26
		3.2.2	Expose Samples to Elevated	
			Temperature	30
	3.3	Samp	les Testing Preparation	37
		3.3.1	Cube Test	37
		3.3.2	Flexure Test	38
4	ANA	LYSIS		39
	4.1	Slum	p Test	39
	4.2	Cube	Test	40
	4.3	Flexu	re Test	44
5	DISC	CUSSIO	N	54
	5.1	Comp	parison to Estimated Findings	54
	5.2	Comp	pare to Control	54
6	CON	ICLUSI	ON	56
REFER	ENCES			58
APPEN	DIX			61

LIST OF TABLES

TABLE NO	TITLE	PAGE
2.1	Physical Properties of Typical Fibres. (Löfgren 2005)	8
2.2	Overview of different fibre forms.	9
2.3	Tensile properties of untreated and alkali	
	treated kenaf fibres.	14
2.4	Influence of Environmental Factors on Heated Concrete.	16
3.1	Summary of samples prepared.	26
4.1	Summary of slump height for all mixes.	39
4.2	7 days cube tests results.	41
4.3	28 days cube tests results.	42
4.4	Flexure test results for all samples.	44

LIST OF FIGURES

FIGURE NO	TITLE	PAGE
2.1	Post cracking behavior of FRC in tension. (Jansson 2008)	6
2.2	Stress-strain graph, the Effect of fibre aspect ratio on	
	the constitutive relation.	7
2.3	Pullout curve of pullout test within epoxy resin	
	combined with pictures of different stages of the test.	10
2.4	Effect of fibre volume on compressive strength.	13
2.5	Comparison between compressive strength of NSC at	
	elevated temperature with experimental data.	17
2.6	Comparison between compressive strength of high	
	strength siliceous aggregate concrete (55.2-80MPa)	
	at elevated temperature with experimental data.	17
2.7	Comparison between compressive strength of high	
	strength siliceous aggregate concrete (80-110 MPa)	
	at elevated temperature with experimental data.	18
2.8	Comparison between compressive strength of calcareous	
	aggregate concrete at elevated temperature with	
	experimental data.	18
2.9	Effect of temperature on residual compressive strength	
	of cement paste materials	19
2.10	Comparison between compressive strength proposed	
	relationship of PPFRC with experimental results.	20
2.11	Comparison between tensile strength proposed	
	relationship of PPFRC with experimental test results.	21
3.1	Steel fibre supplier specifications.	23

3.2	HE 0.75/60, 60mm steel fibres in glued bundles.	23
3.3	Macro Synthetic fibre supplier specifications.	24
3.4	Macro Synthetic fibres	24
3.5	Dry, untreated and uncut kenaf fibres.	25
3.6	Fenaf fibres cut to 60mm lengths.	25
3.7	Concrete Mixer	28
3.8	Premix truck unloading concrete onto forklift.	28
3.9	Concrete casting and curing using wet gunny sack.	29
3.10	Cube and Prism samples curing by fully submerged in	
	water.	29
3.11	Furnace and prism arrangement.	30
3.12	Steel and Kenaf FRC prism after 400°C.	31
3.13	Kenaf FRC prism after 400°C.	31
3.14	Steel FRC prism after 400°C.	31
3.15	Steel and Kenaf FRC prism after 600°C.	32
3.16	Kenaf FRC prism after 600°C.	32
3.17	Steel FRC prism after 600°C.	32
3.18	Steel and Kenaf FRC prism after 800°C.	33
3.19	Kenaf FRC prism after 800°C.	33
3.20	Steel FRC prism after 800°C.	33
3.21	Control prism sample and Synthetic FRC prism after 400°C.	34
3.22	Synthetic FRC prism after 400°C.	34
3.23	Control prism sample after 400°C.	34
3.24	Control prism sample and Synthetic FRC prism after 600°C.	35
3.25	Synthetic FRC prism after 600°C.	35
3.26	Control prism sample after 600°C.	35
3.27	Control prism sample and Synthetic FRC prism after 800°C.	36
3.28	Synthetic FRC prism after 800°C.	36
3.29	Control prism sample after 800°C.	36
3.30	Cube test machine.	37
3.31	Machine settings for Cube test. Pace rate = 2 kN/s	37
3.32	Flexure test machine.	38
3.33	Machine settings for Flexure test. Pace rate = 0.2 kN/s	38

4.1	(A) Control mix Slump. (B) Steel FRC mix Slump.			
	(C) Synthetic FRC mix Slump. (D) Kenaf FRC mix Slump.	40		
4.2	Control cube showing breaking off from crushing. (C6)	43		
4.3	Steel FRC cube shows slight breaking off from crushing			
	yet still maintains shape and form. (SF4)	43		
4.4	Synthetic FRC cube arrests breakage and maintains			
	integrity after failure. (SY6)	43		
4.5	Kenaf FRC cube maintains shape and form after failure. (SY6)	43		
4.6	Relationship of flexure stress capacity against elevated			
	temperature for different FRC.	45		
4.7	Top and Bottom view of prism after flexure test. (C1-0)	46		
4.8	Top and Bottom view of prism after flexure test. (SF2-0)	46		
4.9	Top and Bottom view of prism after flexure test. (SY1-0)	47		
4.10	Top and Bottom view of prism after flexure test. (KF1-0)	47		
4.11	Top and Bottom view of prism after flexure test. (C3-400)	48		
4.12	Top and Bottom view of prism after flexure test. (SF4-400)	48		
4.13	Top and Bottom view of prism after flexure test. (SY4-400)	49		
4.14	Top and Bottom view of prism after flexure test. (KF3-400)	49		
4.15	Top and Bottom view of prism after flexure test. (C5-600)	50		
4.16	Top and Bottom view of prism after flexure test. (SF6-600)	50		
4.17	Top and Bottom view of prism after flexure test. (SY5-600)	51		
4.18	Top and Bottom view of prism after flexure test. (KF5-600)	51		
4.19	Top and Bottom view of prism after flexure test. (C8-800)	52		
4.20	Top and Bottom view of prism after flexure test. (SF7-800)	52		
4.21	Top and Bottom view of prism after flexure test. (SY8-800)	53		
4.22	Top and Bottom view of prism after flexure test. (KF7-800)	53		

LIST OF APPENDIX

APPEND	DIX NO T	TITLE	
А	Premix design by supplier, Lafa	arge Skudai.	61

CHAPTER 1

INTRODUCTION

1.1 General

The most widely used material in the building industry is concrete, due to its ability to resist immense compression forces and resist abrasion. Concrete is a combination of cement, aggregates and water. Without any reinforcement, it has low tensile strength and cracks are easily formed, as it is quite brittle.

Before the invention of steel reinforcement, the concept and usage of fibres as a form of reinforcement has been present since ancient times, such as the use of straws in Egyptian mud blocks. There has been continuous research and development on the technology of using various fibres in construction till this day.

The addition of randomly scattered fibres throughout the concrete mix prevents cracks from expanding further. By increasing the ductility and tensile capabilities of the concrete, problems such as shrinkage cracks and cracks due to weathering can be captured. Thus, improving the durability of the concrete on a whole.

1.2 Problem Statement

Durability and robustness of a structure is huge a concern for the construction industry. Structures are designed to last much longer than a human lifespan. By understanding the factors affecting the lifespan of a building, engineers can continue to design and build long strong structures that will outlive their design expectations. A full concrete block could simply withstand intense heat without getting damaged. However, reinforced concrete would experience spalling, caused by the expansion of the steel reinforcement within the concrete when temperature increases.

Various types of fibres have been introduced to the concrete mix to improve its durability and tensile capacity. Some fibres may have higher resistance to fire, and some are easily flammable. The aim is to gain better understanding on the three main types of fibres used in the construction industry, primarily focusing on the effects of elevated temperature upon these fibre reinforced concrete (FRC) mixes.

1.3 Objectives

The main objectives of this study are as follows:

- 1. To compare the different FRC mixes under different elevated temperatures for flexure resistance.
- 2. To compare compression strength of the different FRC mixes without elevated temperature.

1.4 Scope

Fire resistivity or reaction towards heat of any given fibre is a very important information that governs the decision making of an engineer, during design and when specifying the materials used for construction. The focus of the study is to analyse the various FRC's ability to withstand flexural loads with the increase in temperature and compared to normal concrete without reinforcements.

The concern is the ability of the different fibres in retaining its original form and maintaining flexural stress performance within the concrete mix, when in contact with high heat. All samples are tested after 28 days of curing.

The prospect is for engineers to have more confidence with the limitations as well as capabilities of using the various fibres as additional reinforcement in construction.

The limitations of the study are as below:

- a) Temperature assumed to be accurate and constant throughout the sample, in accordance to the reading on furnace temperature gauge.
- b) Fibre material properties assumed to be consistent.
- c) Exclusion of cost comparison.

REFERENCES

- Cement Concrete and Aggregates Australia. (2004) Concrete Basics, A Guide to Concrete Practice. pp. 50-52.
- Jan Slusarek, Aleksandra Kostrzanowska. (2010) Durability and Repair Problems of Reinforced Concrete Columnar Structures. Architecture Civil Engineering Environment, The Silesian University of Technology. pp. 85-86.
- Faisal Fouad Wafa. (1990) Properties and Application of Fibre Reinforced Concrete. JKAU: Eng. Sci., Vol.2. pp. 49-50.
- 4. R. Brown, A. Shukla and K. R. Natarajan. (2002) *Fiber Reinforcement of Concrete Structures*, pp. 10.
- L.A.P. Lourenco, J.A.O. Barros, J.G.A. Alves. (2005) Development of Fiber Reinforced Concrete of Enhanced Fire Resistance. Fiber Reinforced Concrete of Enhanced Fire Resistance for Tunnel Segments, pp. 1, 7.
- Saandeepani Vajje, Dr.N.R.Krishna Murthy. (2013) Study On Addition Of The Natural Fibers Into Concrete. International Journal Of Scientific & Technology Research Volume 2, Issue 11, pp. 203, 217-218.
- Jyrki Kullaa. (1992) Constitutive Modeling of Fiber-reinforced Concrete Under Uniaxial Tensile Loading. Rakenteiden Mekaniikka, Vol. 25, No 4, ss. 24-49, pp 38.
- 8. P.K. Mehta, P.J.M. Monteiro (2006) *Concrete: Microstructures, Properties, and Materials.*
- 9. Abid A. Shah, Y. Ribakov. (2011) *Recent Trends in Steel Fibered High-Strength Concrete*, pp. 4125.
- R.S. Olivito, F.A. Zuccarello. (2010) An Experimental Study on the Tensile Strength of Steel Fiber Reinforced Concrete, pp. 250, 255.
- Bernd Weiler, Christian Grosse. (2008) Pullout Behaviour Of Fibers In Steel Fiber Reinforced Concrete, pp. 123-125.

- 12. Hamid Pesaran Behbahani, Dr. Abdul Rahman bin Mohd. Sam. (2010) Flexural Behaviour of Steel Fiber Reinfoced Concrete Beams, pp. 45, 52.
- Anne Laning. (1992) Synthetic Fibres. Publication #C920525 The Aberdeen Group.
- Grace Concrete Products. (2011) Macro-synthetic Fibre Reinforcement. Technical Bulletin TB-1204.
- 15. Vision Paper (1998) *About the Kenaf Plant* [Online] Available From: http://www.visionpaper.com/kenaf2.html [Accessed: 10th Dec 2014]
- Everise Crimson(M) Sdn.Bhd. (2012) What is Kenaf
 [Online] Available From: http://www.kenaf-everise.com.my/Kenaf_Everise/What_is_Kenaf.html
 [Accessed: 10th Dec 2014]
- Masoud Razavi, Dr. Abdul Rahman bin Mohd. Sam. (2013) Performance of Kenaf Fibre Reinforced Concrete, pp. 67, 89.
- Y. Nitta1, J. Noda, K. Goda and W-I. Lee. (2011) Alkali-Treatment on Tensile Properties of Kenaf Long Fibers. 18th Int.Conf. on Composite Materials.
- Ribot, N.M.H., Ahmad, Z., Mustaffa, N.K. (2011). Mechanical Propertise of Kenaf Fiber Composite Using Co- Cured In-Line Fiber Joint, International Journal of Engineering Science and Technology (IJEST), pp. 3530.
- 20. Gyorgy L. Balazs, Eva Lubloy. (2012) *Reinforced Concrete Structures In and After Fire*. Concrete Structures, pp. 72.
- 21. D. J. Naus. (2005) *The Effect of Elevated Temperature on Concrete Materials and Structures-A Literature Review*, pp. 167-168
- G.A. Khoury (1996) Performance of Heated Concrete-Mechanical Properties. Contract NUC/56/3604A with Nuclear Installations Inspectorate, Imperial College.
- Farhad Aslani and Bijan Samali. (2013) Predicting the bond between concrete and reinforcing steel at elevated temperatures. Structural Engineering and Mechanics, Vol. 48, No. 5. pp. 643-660.
- P.J.E. Sullivan and R. Sharahar. (1992) The performance of Concrete at Elevated Temperatures (As Measured by the Reduction in Compressive Strength). Fire Technology 28(2). pp. 240-250.

- 25. G.M. Chen, Y.H. He, H. Yang, J.F. Chen and Y.C. Guo. (2014) *Compressive* behavior of steel fiber reinforced recycled aggregate concrete after exposure to elevated temperatures. Construction and Building Materials 71.
- Farhad Aslani and Bijan Samali. (2014) High Strength Polypropylene Fibre Reinforcement Concrete at High Temperature. Fire Technology, 50, 1229– 1247.
- Sofren Leo Suhaendi and Takashi Horiguchi. (2005) Fiber-reinforced Highstrength Concrete under Elevated Temperature—Effect of Fibers on Residual Properties. Fire Safety Science–Proceedings of the Eighth International Symposium, pp. 271-278