DETERMINATION OF OPTIMUM CONCENTRATION OF LIME SLURRY FOR SOIL STABILISATION

MUHAMMAD SOFIAN ABDULLAH

A project report submitted of the fulfillment of the requirements for the award of the degree of Master of Engineering (Civil- Geotechnic)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > **APRIL, 2006**

To my beloved family, Rafidah and Muhammad Hussien

ACKNOWLEDGEMENT

I wish to express my sincere appreciation to my thesis supervisor, Associate Professor Dr. Khairul Anuar Kassim, for encouragement guidance, critics and friendship during preparing this thesis. Without his continued support and interest, this thesis would not have presented here.

My sincere appreciation also extends to all the staff of Geotechnical Laboratory, Faculty of Civil Engineering, UTM, especially for Mr. Abdul Samad, Mr. Zulkifli, who provided assistance at various occasions. Their views and tips are useful indeed. Unfortunately, it is not possible to list all of them in this limited space. I am also grateful to all my family members especially my mother, my wife and my son for their understanding and encourage me all the time to complete this thesis.

I am also indebted to the librarians at Universiti Teknologi Malaysia (UTM) for their assistance in supplying the relevant literatures and lastly my sincere appreciation extent to all the people direct or indirect participates to complete this thesis.

ABSTRACT

Lime has been used as active additive in soil stabilisation for the past 5000 years ago. The Pyramids of Shersi in Tibet were built using compacted mixtures of clay and lime. Lime powder is normally spread on the ground using mechanical means before mixing with the soil. The problem arise from lime mixing is dusting. The introduction of lime slurry method for soil stabilisation is intended to solve the problem. This research is focused on determination of optimum concentration of the lime slurry for soil stabilisation. Classification test on the natural soil was conducted to determine the suitability of soil to be treated with lime. Suitability of lime test also conducted to ensure the quality of lime used is acceptable. The compaction characteristic of soil treated with lime slurry posses the same behaviour as demonstrated by soil treated with dry lime where the optimum moisture content increases and the maximum dry density decreases. Unconfined compressive test (UCT) was conducted on soil treated with different range of lime concentration, ranging from 10 to 40 percent of weight of water and cured for 7 to 28 days. Based on UCT, it was illustrated that the strength development before 14 days is less significant. This could be due to the process of modification. However, after 14 days the strength increases rapidly. This phenomenon could be related to the process of stabilisation. The optimum concentration of lime slurry to stabilise the soil is 20 percent of weight of water, which is equivalent to 3.63 percent of dry lime.

Keyword: Soil stabilisation, lime slurry, unconfined compressive test (UCT)

ABSTRAK

Penggunaan kapur sebagai agen penstabilan tanah telah digunakan lebih 5000 tahun dahulu. Piramid Shersi di Tibet dibina menggunakan campuran kapur dan tanah yang dimampatkan. Serbuk kapur biasanyan diserakkan di atas tanah menggunakan alatan mekanikal sebelum kerja-kerja percampuran kapur dengan tanah. Masalah yang timbul daripada percampuran ini ialah habuk. Pengenalan kepada kaedah larutan kapur untuk kerja-kerja penstabilan tanah diperlukan untuk menyelesaikan masalah ini. Kajian ini memfokuskan kepada penentuan kepekatan larutan kapur yang optima untuk penstabilan tanah. Ujian pengkelasan dijalankan ke atas tanah asal untuk menentukan kesesuaian tanah tersebut untuk di rawat dengan kapur. Ujian kesesuaian kapur juga dijalankan untuk menentukan kualiti kapur yang digunakan. Ciri-ciri lengkung pemadatan tanah yang dirawat menggunakan larutan kapur menunjukkan sifat yang sama dengan tanah yang dirawat menggunakan kapur kering dimana kandungan lembapan optima meningkat dan ketumpatan kering maksima menurun. Ujian mampatan tak terkurung (UCT) dijalankan ke atas tanah yang di rawat menggunakan larutan kapur yang berbeza dalam lingkungan 10 hingga 40 peratus daripada berat air dan diawet selama 7 hingga 28 hari. Berdasarkan kepada ujian UCT, kekuatan tanah yang diawet kurang daripada 14 hari tidak begitu signifikan. Ianya disebabkan oleh proses modifikasi. Walaubagaimanapun, selepas 14 hari diawet berlaku peningkatan kekuatan tanah secara mendadak. Situasi ini disebabkan oleh proses penstabilan tanah. Kepekatan larutan kapur yang optima ialah 20 peratus daripada berat air dimana ianya bersamaan dengan 3.63 peratus kapur kering.

Katakunci: Penstabilan tanah, Larutan kapur, Ujian Mampatan Tak Terkurung (UCT)

TABLE OF CONTENTS

CHAPTER	TITLE	PAGE
	THESIS STATUS VALIDATION	
	TITLE	i
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDGEMENT	iv
	ABSTRACT	V
	ABSTRAK	vi
	TABLE OF CONTENTS	vii
	LIST OF TABLES	xi
	LIST OF FIGURES	xii
	LIST OF APPENDICES	XV
1	INTRODUCTION	
	1.1 Background Study	1
	1.2 Objectives	2
	1.3 Scope of Study	2
2	LITERATURE REVIEW	
	2.1 Clay Mineral	3
	2.2 Soil Improvement	7

2.3 Compaction	7
2.3.1 Surface Compaction	7
2.3.2 Deep Compaction	8
2.4 Stabilisation	
2.4.1 Mechanical Method	9
2.4.1.1 Vibroflotation	9
2.4.2.2 Vertical Drain	10
2.4.2.3 Geotextile	10
2.4.2 Chemical Method	10
2.5 Lime	11
2.5.1 Lime Stabilisation	12
2.5.2 Mechanism of Lime Stabilisation	13
2.5.2.1 Hydration	13
2.5.2.2 Ion Exchange and Flocculation	14
2.5.2.3 Pozzolanic Reaction	14
2.5.2.4 Carbonation	15
2.5.3 Effect of Lime on The Physical Properties of	15
The Soil	
2.5.4 Solubility of Hydrated Lime	19
2.5.5 Previous Study	20
2.6 Recommended Construction Procedure	
2.6.1 Delivery	22
2.6.2 Lime Spreading	23
2.6.3 Preliminary Mixing and Watering	24
2.6.4 Mellowing Period	26
2.6.5 Final Mixing and Pulverization	26
2.6.6 Compaction	27
2.6.7 Final Curing	28
2.6.8 Advantages and Disadvantages of Different	29
Lime Applications	

METHODOLOGY

3.1 Laboratory Testing	31
3.2 Soil Testing	33
3.2.1 Specific Gravity Test	33
3.2.2 Particle Size Distribution (PSD)	34
3.2.2.1 Sieve Analysis	35
3.2.2.2 Hydrometer Analysis	35
3.2.3 Atterberg limit (Consistency of Soil)	37
3.2.3.1 Liquid Limit (LL)	38
3.2.3.2 Plastic Limit (PL)	39
3.2.3.3 Plasticity Index (PI)	40
3.2.4 Standard Proctor Test (Compaction Test)	40
3.3 Lime Testing	42
3.3.1 Initial Consumption of Lime (ICL)	42
3.3.2 Available Lime Content (ALC)	43
3.4 Lime Stabilise Soil Testing	44
3.4.1 Unconfined Compressive Test (UCT)	44

4

RESULT AND DISCUSSION

4.1 Soil Classification Test	
4.1.1 Atterberg Limit	
4.1.1.1 Liquid Limit	46
4.1.1.2 Plastic Limit	47
4.1.1.3 Plasticity Index	48
4.1.2 Specific Gravity Test	49
4.1.3 Particle Size Distribution (PSD)	50
4.2 Lime Suitability Test	
4.2.1 Initial Consumption of Lime (ICL)	51
4.2.2 Available Lime Content (ALC)	52
4.3 Standard Proctor Compaction Test	52
4.4 Unconfined Compressive Strength (UCS)	54

	4.4.1 Lime Mixing	56
5	CONCLUSION	59
REFERENCES		60
APPENDIX A	Result of Soil Classification Test	62
APPENDIX B	Result of Compaction Test	65
APPENDIX C	Result of Unconfined Compressive Test	71

LIST OF TABLES

TABLE NO.	TITLE	PAGE
2.1	Solubility of lime at different temperature (After Ng Pui Ling, 2005)	19
4.1	Summary of Data for Specific Gravity Test	49
4.2	Summary of data from ICL test	51
4.3	Compaction Test Result	53
4.4	Summary of Unconfined Compressive Strength (UCS) in kPa	56
4.5	Equivalent Dry Lime for Different Concentration of Lime Slurry	58

LIST OF FIGURES

FIGURE NO.	TITLE	PAGE
2.1	Diffuse double layer (After Braja M.Das)	6
2.2	Attraction of dipolar molecule in diffuse double layer (After Braja M.Das)	6
2.3	Reduction with time of the plasticity index due to the calcium effect (After A.Kezdi)	17
2.4	The effect of lime addition on the volume change (After A.Kezdi)	17
2.5	Figure 2.5: Typical compaction curve due to the lime addition (After A.Kezdi)	18
2.6	Typical compression strength due to lime addition (After A.Kezdi)	18
2.7	Typical compressive strength due to lime addition and curing time (After A.Kezdi)	18

2.8	Portable lime slaker for preparing lime slurry on site (After National Lime Association, 2004	22
2.9	Lime slurry application (After National Lime Association, 2004)	24
2.10	Scarification after lime spreading (After National Lime Association, 2004)	25
2.11	Steel wheel roller (After National Lime Association, 2004)	27
2.12	Prime coat emulsion for curing (After National Lime Association, 2004)	28
3.1	Laboratory testing	32
3.2	Specific Gravity Vacuum	34
3.3	A set of sieves	35
3.4	Mechanical shaker	36
3.5	Hydrometer reading	36
3.6	Atterberg Limit	37
3.7	Details of cone for liquid limit test	38
3.8	Liquid limit test (cone penetration)	39
3.9	Plastic limit test	39

3.10	Mould for Compaction Test (1L Mould)	41
3.11	Dry density / Moisture content relationship curve	41
3.12	Samples preparation	45
3.13	Unconfined Compression Test Equipment	45
4.1	Cone Penetration vs Moisture Content	47
4.2	Plasticity Chart	48
4.3	Particle Size Distribution Curve	50
4.4	Standard Proctor Compaction Curve	53
4.5	Unconfined Compressive Strength at Different Curing Time	55
4.6	Unconfined Compressive Strength at Various Lime Slurry	55
4.7	Rate of increase in Strength	57
4.8	Percentage Increase in Strength at Various Lime Content	57
4.9	Unconfined Compressive Strength (UCS) at 14 days with various addition of lime. (After Khairul Anuar Kasim and Kok Kai Chern,2004)	58

LIST OF APPENDICES

APPENDIX

TITLE

PAGE

Liquid limit Test	62
Particle Sizes Distribution	63
Specific Gravity Test	64
Compaction Test (Untreated Soil)	65
Compaction Test (10% Lime Slurry)	66
Compaction Test (20% Lime Slurry)	67
Compaction Test (30% Lime Slurry)	68
Compaction Test (40% Lime Slurry)	69
Compaction Curve For All Concentration	70
Unconfined Compressive Test (Untreated -0 day)	71
Unconfined Compressive Test (Untreated - 7 days)	72
Unconfined Compressive Test (Untreated – 14 days)	73
Unconfined Compressive Test (Untreated – 28 days)	74
Unconfined Compressive Test (10% Lime Slurry – 0 day)	75
Unconfined Compressive Test (10% Lime Slurry – 7 days)	76
Unconfined Compressive Test (10% Lime Slurry – 14 days)	77
Unconfined Compressive Test (10% Lime Slurry – 28 days)	78
Unconfined Compressive Test (20% Lime Slurry – 0 day)	79
Unconfined Compressive Test (20% Lime Slurry – 7 days)	80
	Particle Sizes Distribution Specific Gravity Test Compaction Test (Untreated Soil) Compaction Test (10% Lime Slurry) Compaction Test (20% Lime Slurry) Compaction Test (30% Lime Slurry) Compaction Test (40% Lime Slurry) Compaction Curve For All Concentration Unconfined Compressive Test (Untreated – 0 day) Unconfined Compressive Test (Untreated – 7 days) Unconfined Compressive Test (Untreated – 14 days) Unconfined Compressive Test (Untreated – 28 days) Unconfined Compressive Test (10% Lime Slurry – 0 day) Unconfined Compressive Test (10% Lime Slurry – 14 days) Unconfined Compressive Test (10% Lime Slurry – 28 days) Unconfined Compressive Test (10% Lime Slurry – 28 days)

C11	Unconfined Compressive Test (20% Lime Slurry – 14 days)	81
C12	Unconfined Compressive Test (20% Lime Slurry – 28 days)	82
C13	Unconfined Compressive Test (30% Lime Slurry – 0 day)	83
C14	Unconfined Compressive Test (30% Lime Slurry – 7 days)	84
C15	Unconfined Compressive Test (30% Lime Slurry – 14 days)	85
C16	Unconfined Compressive Test (30% Lime Slurry – 28 days)	86
C17	Unconfined Compressive Test (40% Lime Slurry – 0 day)	87
C18	Unconfined Compressive Test (40% Lime Slurry – 7 days)	88
C19	Unconfined Compressive Test (40% Lime Slurry – 14 days)	89
C20	Unconfined Compressive Test (40% Lime Slurry – 28 days)	90
C21	Stress-Strain Curve (Untreated Soil)	91
C22	Stress-Strain Curve (Treated With 10% Lime Slurry)	92
C23	Stress-Strain Curve (Treated With 20% Lime Slurry)	93
C24	Stress-Strain Curve (Treated With 30% Lime Slurry)	94
C25	Stress-Strain Curve (Treated With 40% Lime Slurry)	95
C26	Unconfined Compressive Test VS Curing Time	96
C27	Unconfined Compressive Test VS Lime Slurry Content	97
C28	Rate of Increase in Strength	98
C29	Percentage Increase in Strength at Different Lime Content	99

CHAPTER 1

INTRODUCTION

1.1 Background Study

Lime stabilisation is commonly resorted to in order to improve the strength and reduce the compressibility of weak clay deposits. Malaysia is one of the countries around the world that rich with limestone resources. Limestone formations are widespread in Langkawi Island, Kinta Valley, Gua Musang and Kuala Lumpur area (Khairul Anuar and Kok Kai Chern, 2004). The usage of lime as a stabilise agent is not new especially for road construction. Many researchers have studied the suitability of lime to improve soil properties such as strength and deformation behavior. Researchers have illustrated that the impact of lime addition on strength of clay soils depends on several factors. These include, soil type, curing time and method, moisture content and soil unit weight and time elapsed between mixing and compaction (Sudhakar M. Rao and P. Shivananda, 2004). In Malaysia, lime is not commonly use as a stabilise agent. It is because the lack of local research and understanding about suitability of lime stabilisation for Malaysian soil. The previous researcher found that by using 3% to 6% of dry lime, it contributed the significant increase in UCT test, ranging from 2.5 to 11 times of untreated soil (Khairul Anuar and Kok Kai Chern, 2004). Although dry lime is very useful in soil stabilisation, it always causes dusting problem and is corrosive to human skin especially quick lime. To counter this problem, lime in slurry form is suggested to stabilise the soil. 60 remolded samples with different concentration have been prepared for this study and cure for 7, 14 and 28 days. Then, unconfined compressive test is carried out on the cured samples to determine the highest shear strength. As a result, the optimum concentration of lime slurry in this study is 20% of weight of water where this percentage is equivalent to 3.63% of dry lime.

1.2 Objectives

The objectives of this research are as below:-

- To determine the optimum concentration of lime slurry for soil stabilisation
- To determine the compaction characteristic of the lime-soil mixture
- To study on strength development of the lime-soil mixture

1.3 Scope of study

These studies only focus on determination of the optimum lime slurry to be used in soil stabilisation in clayey soil. The clay will be used is kaolin clay from Tapah, Perak. Only hydrated lime slurry is used to stabilise the clayey soil in this study. All the testing for this study is conducted in laboratory by using British Standard as a reference.

REFERENCES

- Arabi M and Wild S, Property Changes Induced in Clay Soils When Using Lime Stabilization, Municipal Engineer (Institution of Civil Engineers), v 6, n 2, Apr, 1989, pg 85-99
- 2. Bell F.G and Coulthard J.M, *Stabilization of Clay Soils With Lime*, Municipal Engineer (Institution of Civil Engineers), v 7, n 3, Jun, 1990, pg 125-140
- 3. Bell F.G, *Lime Stabilization of Clay Minerals and Soils*, Engineering Geology 42 (1996), pg 223-237
- 4. Bergado, D.T, Anderson, L.R, Miura, N, and Balasundram, A.S (1994), *Soft Ground Improvement in Lowland and Other Environments*, ASCE Press, New York, U.S
- 5. Boynton R.S, Blacklock, J.R, Bulletin 331,*Lime Slurry Pressure Injection Bulletin*, National Lime Association, Arlington, A.S
- 6. British Standard Institution (1990), BS 1377, *Methods of tests for Soils*, British Standard Institution, London, UK.
- 7. British Standard Institution (1990), BS 1924, *Methods of tests for Stabilized Soils*, British Standard Institution, London, UK.
- 8. British Standard Institution (1990), BS 6463, *Methods of tests for chemical stabilizers*, British Standard Institution, London, UK.
- 9. Das B.M, Principle of Geotechnical Engineering, Fifth Edition, Brook/Cole, Thomson Learning, 2002, pg 21-39
- 10. Kassim K.A and Kok K.C (2004), *Lime Stabilized Malaysian Cohesive Soil*, Jurnal Kejuruteraan Awam 16 (1), pg 13-23
- 11. Kezdi A (1979), *Stabilized Earth Roads*, Elsevier Scientific Publishing Company, Amsterdam, Netherlands

- 12. Lawrence J S, Harry F (2000), *Building a Pad From Lime Stabilized Soil*, ProQuest Science Journals, pg. 45
- 13. National Association of Australian State Road Authorities (1986), *Guide to Stabilisation in Roadworks*, NAASRA, Sydney, Australia.
- 14. National Lime Association (2004), *Lime Stabilization and Lime Modification*, National Lime Association, USA
- 15. Rogers C.D.F, *Lime Requirement for Stabilization*, Transportation Research Record, n 1721, 2000, pg 9-18
- Sudhakar M. R and Shivananda P, Role of Curing Temperature in Progress of Lime Soil Reactions, Geotechnical and Geological Engineering (2005) 23: 79– 85
- 17. Sudhakar M. R and Shivananda P, *Compressibility Behaviour of Lime-Stabilized Clay*, Geotechnical and Geological Engineering (2005) 23: 309–319