TELECOMMUNICATION TOWERS ASSESSMENT SYSTEM IN CONSIDERATION OF EARTHQUAKES EFFECTS IN MALAYSIA

ZAWIYAH BINTI ABDUL RAZAK

UNIVERSITI TEKNOLOGI MALAYSIA

TELECOMMUNICATION TOWERS ASSESSMENT SYSTEM IN CONSIDERATION OF EARTHQUAKES EFFECTS IN MALAYSIA

ZAWIYAH BINTI ABDUL RAZAK

A dissertation submitted in partial fulfilment of the requirements for the award of the degree of Doctor of Engineering (Technology and Construction Management)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > AUGUST 2013

DEDICATION

Undertaking a study of this nature cannot be sustained without the support of the dearly people in my life. I dedicate this work firstly to the following two persons who have touched my life in significant ways, more than they'll ever know – my beloved parents, Tn. Haji Abdul Razak B. Mohamed Ali and Pn. Hajah Nooriah Bt. Mohamed Basir; for their simplicity and thrift, ALLAH bless them with good health. Secondly, to my boy Zubair, who have been very patience, cherish my life and have helped in various ways, my heartfelt thanks. Also to all my brothers, sisters and families for their invaluable support and understanding, for their forbearance over the past few years, may ALLAH bless them with self-fulfilment, good cheer and happy lives.

ACKNOWLEDGEMENT

I wish to thank the following persons without whose efficient assistance this thesis would not be in its readable form as it is now: especially for my research supervisors, Associate Professor Dr. Arham Bin Abdullah and Professor Dr Azlan Bin Adnan for their assistance, guide, comments and valuable suggestions, the management of Telekom Malaysia Berhad especially to the Vice President of Group Human Capital, Dr Zainal Bin Abu Zarim, Vice President of Support Business, Tn. Haji Zam Ariffin Bin Ismail, management of Business Support Property Operations especially to Tn. Haji Mohd Nasir Bin Abdullah, staffs of Project Implementation (Civil) unit, Property Operations Region 1 Central, especially Mr Yahya Bin Ibrahim, Property Management especially to Mr. Ir Rozaidi Bin Ramli and those from Malaysian Research and Education Network (MYREN). I am also indebted to Yayasan Telekom (YTM) for funding my Eng.D.

I would also like to thank my mentor Associate Professor Dr Kazutsuna Yamaji of National Institute of Informatics, Tokyo, Associate Professor Dr Yoshiaki Kasahara of Kyushu University, Japan, friends for their great support and encouragement. Special thanks to my fellow postgraduate friends Mr. Mohammad Reza Vafaee, Mr. Behzad Bayat and Mr SK Muiz Bin SK Abd Razak for their great assistance and hardship in my modeling works and who have been very patient in helping me throughout the work.

My sincere appreciation also extends to all my colleagues and others who have provide assistance at various occasions until the end of my study.

Thank you. May ALLAH bless all of you.

ABSTRACT

The main objectives of the study are to produce standard visual inspection procedure for telecommunication tower structures, secondly to evaluate the structural integrity of the existing towers and finally, to develop Telecommunication Tower Assessment System (TTAS). In order to produce the standard visual inspection procedure a meticulous strategy was implemented. Five phases of work included planning, site survey, development, evaluation and finally the application phase were carried out. Moreover, the integrity of existing towers was evaluated through seismic analysis of four (4) legged self-supporting steel towers with different geometrical cross sections and variable heights using International Building Code (IBC2000) and Eurocode (EC8) and SAP 2000 software. Seismic base shears, maximum joint displacements, and axial force of critical elements were calculated and were compared with allowable values. Besides that, a map locating all the towers was also produced on the seismic map considering 500 year return period for both Peninsular and East Malaysia. Both TTAS and map was later evaluated, verified and validated by subject matter experts. The study has developed a complete telecommunication tower's assessment system that includes types of damages and severity inflicted to the towers and also matters related to it. This classification will make assessment easier and standardised for all inspection works carried out. With the aid of the seismic map, Telekom Malaysia will be able to further define priorities and establish programmes to apply available resources to the most critical towers nationwide. In short, the outcomes of this research helps to promote a uniform standard of practice among tower owners and related parties besides assessing relevant authorities to be more prepared and alert in terms of emergency management and hazard-preparedness due to the effect of earthquake events.

ABSTRAK

Objektif utama kajian adalah untuk menghasilkan prosedur setara pemeriksaan bagi struktur menara telekomunikasi, keduanya untuk menilai keupayaan dan kekukuhan struktur sediada, dan akhir sekali untuk membangunkan satu Sistem Penilaian Menara Telekomunikasi (SPMT). Bagi menghasilkan prosidur tersebut, strategi yang teliti telah diimplentasikan. Lima fasa merangkumi perancangan, kajian lapangan pembangunan, penilaian dan akhirnya fasa aplikasi telah dilakukan. Kekukuhan menara sediada turut dikaji melalui analisa seismik terhadap menara besi berkaki empat (4) dengan berbagai keratan geometri dan ketinggian menggunakan International Building Code (IBC 2000) dan Eurocode (EC8) serta perisian SAP2000. Ricihan tapak seismik, perubahan sambungan maksima dan daya paksian bagi elemen kritikal telah dikira dan dibandingkan dengan nilai yang dibenarkan. Sebuah peta seismik yang mengandungi semua menara telah dihasilkan yang mengambilkira ulangan gempa selama 500 tahun bagi Semenanjung dan Malaysia Timur. Kedua-dua SPMT dan peta yang dihasilkan telah dinilai, sahkan dan dimuktamadkan oleh pakar bidang berkaitan. Kajian ini berjaya membangunkan satu sistem penilaian penyelengaraan yang menyeluruh termasuk mengenalpasti jenis-jenis dan tahap kerosakan yang dialami oleh menara telekomunikasi dan perkara yang berkaitan dengannya. Klasifikasi ini akan membuat penilaian struktur lebih mudah dan setara. Peta seismik yang dihasilkan akan memudahkan pihak Telekom Malaysia menetapkan keutamaan dan mewujudkan program untuk menyediakan peruntukan bagi menara yang kritikal di seluruh negara. Dalam ertikata lain, kajian ini telah berjaya membangunkan satu sistem setara yang diharap dapat membantu menyelaraskan pemeriksaan antara pemilik menara dan pihak lain yang terbabit serta membantu pihak berkuasa dalam membuat persiapan pengurusan kecemasan dan persediaan menghadapi kesan akibat kejadian gempa bumi.

LIST OF CONTENTS

СНАРТ	ER	TITLE	PAGE
	DE	CLARATION	ii
	DEI	DICATION	iii
	AC	KNOWLEDGEMENT	iv
	ABS	STRACT	V
	ABS	STRAK	vi
	LIS	T OF CONTENTS	vii
	LIS	T OF TABLES	xii
	LIS	T OF FIGURES	XX
	LIS	T OF SYMBOLS	xxiii
	LIS	T OF ABBREVIATIONS	xxvi
	LIS	T OF APPENDIX	xxviii
1	INT	RODUCTION	1
	1.1	Introduction	1
	1.2	Problem Statement	9
	1.3	Aim of Research	11
	1.4	Objectives of Research	11
	1.5	Scope of Research	12
	1.6	Significance of the Research	19
	1.7	Contribution of the Research	20
	1.8	Organisation of Thesis	21
2	LITI	ERATURE REVIEW	23
	2.1	Introduction of Telecommunciation	23

vii

	٠	٠	٠
V	1	1	1

2.2	Telelcommunciation Infrastructure and Society	24
	2.2.1 Economic Impact	26
2.3	Telekom Malaysia Berhad	28
	2.3.1 National Broadband Initiatives	29
	2.3.2 ASIA Submarine Cable Express Cable	
	System	32
2.4	Importance of Telecommunication Facilities	36
2.5	Importance of Telecommunication Facilities During	
	Disaster	44
2.6	Organising for Effective Disaster &	
	Communication	47
	2.6.1 Malaysian Emergency Response System	
	(MERS 999) Concept	48
2.7	Telecommunciation Steel Tower Structures	49
	2.7.1 Types of Tower	50
	2.7.2 Foundation Types of Self Supporting	
	Telecommunication Tower	54
	2.7.3 Tower Member Classification	55
	2.7.4 Tower Loadings	59
2.8	Maintenance of Telecommunciation Structures	60
2.9	Maintenance Culture in Malaysia	62
2.10	Earthquakes	66
	2.10.1 Power of Earthquakes	69
	2.10.2 Effects of Earthquakes	71
2.11	Disaster Preparation	77
2.12	Seismic Hazard Map	79
2.13	Structural Failures	82
2.14	Tower Analysis	84
2.15	Concluding Remarks	88
THE	DRETICAL BACKGROUND	90
3.1	Introduction	90
	3.1.1 Seismic Design Requirements for Non Building	91

3

3.2	Earth	quake Desi	ign Data	91
	3.2.1	Structur	al Analysis Procedure Selection for	
		IBC/AS	CE	91
	3.2.2	Seismic	Design Requirement	99
	3.2.3	Structur	e Analysis Procedure Selection for	
		Eucocoo	de (EC8)	103
	3.2.4	Structur	e Analysis Procedure Selection Using	
		ANSI/T	IA-222G Standards	111
	3.2.5	Design	Spectral Response Acceleration	113
	3.2.6	Seismic	Analysis Procedure	114
		3.2.6.1	Total Seismic Shear	115
		3.2.6.2	Vertical Distribution of Seismic Forces	117
		3.2.6.3	Determination of Seismic Forces	118
		3.2.6.4	Base Shear Contributed by Each Mode	119
		3.2.6.5	Limit State Deformations	120
3.3	Conclu	uding Rem	arks	121
RESE	EARCH	METHO	DOLOGY	122
4.1	Introdu	uction		122
4.2	Data C	Collection	Method	124
	4.2.1	Primary	Sources	124
	4.2.2	Secondar	ry Sources	125
	4.2.3	Internet	Sources	125
4.3	Resear	ch Methoo	dology Phases	125
	4.3.1	Phase 1-	Planning	125
	4.3.2	Phase 2-	Site Survey	126
	4.3.3	Phase 3-	Development	127
		4.3.3.1	Tower Assessment System	128
		4.3.3.2	Mapping of Telecommunication	
			Towers	128
		4.3.3.3	Tower Modeling	128
		4.3.3.4	Tower Analysis	129
	4.3.4	Phase 4-	Evaluation	129
		4.3.4.1	Tower Assessment System	129

4

ix

		4.3.4.2 Mapping of	Tower	130
		4.3.4.3 Tower Mode	eling and Analysis	130
		4.3.5 Results and Conclusio	n	131
	4.4	Concluding Remarks		131
5	MOL	EL DEVELOPMENT		133
	5.1	Introduction		133
	5.2	Formulation of Tower Assess	ment List	133
	5.3	Ratings of Tower Assessment	Damages	135
		5.3.1 Condition Rating Sys	tem for components of	
		a Structure		136
		5.3.2 Condition Rating Sys	tem for Structures	137
	5.4	Guideline for Clarifying Seven	rity of Damage	137
	5.5	Tower Inspection Procedures		140
	5.6	Types of Assessment Forms		147
	5.7	Mapping for Telecommunciat	ion Tower	
		Structures		149
		5.7.1 Towers for Seismic M	lapping	149
		5.7.2 Procedure for Seismic	c Mapping of Towers	150
	5.8	Output on Seismic Mapping o	fTelecommunication	
		Towers in Malaysia		154
	5.9	Tower Modeling		157
		5.9.1 Outcome of Tower Me	odeling	157
		5.9.2 Validation of the Tow	er Model	160
	5.10	Concluding Remarks		168
ſ	5.10 D.D.C			100
6	KES	LIS AND DISCUSSIONS		172

6.1	Introduction		172
6.2	Results	s on Analysis	172
	6.2.1	Joint Displacement Analysis	173
	6.2.2	Base Shear Reaction Analysis	193
6.3	Results	s and Discussion on Joint Displacements	
	and Ba	se Shear Reaction Analysis Output	210

	6.4	Analysis of Axial Forces for Modeled Towers	229
	6.5	Statistical Analysis on Questionnaires	236
		6.5.1 Summary on Questionnaires Findings	237
	6.6	Concluding Remarks	239
7	CO	NCLUSION AND RECOMMENDATIONS	241
	7.1	Introduction	241
	7.2	Conclusions	242
	7.3	Limitations of the Research	244
	7.4	Recommendations	246

REFERENCES	249
Appendix A-E	264 - 313

xi

LIST OF TABLES

TABLE NO.TITLE		PAGE
1.1	Magnitude and return period of earthquake events	4
1.2	Heights of towers for modeling and analysis	13
1.3	500 year return period based on CIDB for Peninsular	
	Malaysia and PWD for East Malaysia	14
1.4	Seismic zones in Malaysia	14
1.5	Peak ground accelerations for tower analysis	15
1.6	Heights of towers for modeling and analysis	16
1.7	Ground types in European Standards	17
1.8	Ground types in International Building Code	18
2.1	Summary of news channels	28
2.2	Summary of advertisements medium	28
2.3	High Speed Broadband project rollup Telekom Malaysia	32
2.4	The 10 largest and deadliest earthquakes since 1990	38
2.5	Earthquake events that have effects on Malaysia	40
2.6	Summary of TM 4 legged self supporting steel towers	
	until December, 2010 for TM	61
2.7	Summary of 4 legged self supporting steel tower's	
	height category for TM	61
2.8	List of Tower Inspector's for TM	62
2.9	The Modified Mercalli Intensity scale	69
2.10	The deadliest tsunami in history from 1755 to 2011	74
3.1	Occupancy category of buildings and other structures	92
3.2	Seismic coefficient for non-building structures not similar	
	to buildings	94
3.3	Importance factors by risk category of buildings and other	

	structures for snow, ice and earthquake loads	96
3.4	Seismic coefficient for Malaysia	96
3.5	Ground Types in International Building Code	97
3.6	Coefficient for upperlimit on calculated period	98
3.7	Value of approximate period parameters C_s and x	98
3.8	Allowable story drift, Δ_a	103
3.9	Importance classes for towers, masts and chimney	104
3.10	Ground Acceleration for Tower Analysis	104
3.11	Ground Types in European Standard	105
3.12	Values of parameters describing the recommended Type 2	
	elastic response spectra	108
3.13	Classification of structures	112
3.14	Structure irregularities	113
3.15	Seismic analysis procedure methods	114
4.1	Location of towers selected for case study	127
5.1	General definition of rating system for structures	137
5.2	Criteria for classification of severity of damage for Tower	
	Assessment System	138
5.3	Towers color code on seismic mapping for TM towers	150
5.4	Conversion sample of coordinates for mapping	151
5.5	Natural period and frequency for modeled towers	162
5.6	Seismic load pattern input for ground type A	165
5.7	Seismic load pattern input for ground type B	166
5.8	Seismic load pattern input for ground type C	167
5.9	Seismic load pattern input for ground type D	167
5.10	Summary of types of damages for Tower Assessment	169
6.1	Shapes of modeled towers	214
6.2	Displacement limitations for Zone 1:PGA0.04:	
	ground type A	216
6.3	Displacement limitations for Zone 1:PGA0.04:	
	ground type B	217
6.4	Displacement limitations for Zone 1:PGA0.04:	
	ground type C	217

6.5	Displacement limitations for Zone 1:PGA0.04:	
	ground type D	218
6.6	Displacement limitations for Zone 1:PGA0.06:	
	ground type A	218
6.7	Displacement limitations for Zone 1:PGA0.06:	
	ground type B	219
6.8	Displacement limitations for Zone 1:PGA0.06:	
	ground type C	219
6.9	Displacement limitations for Zone 1:PGA0.06:	
	ground type D	220
6.10	Displacement limitations for Zone 1:PGA0.08:	
	ground type A	220
6.11	Displacement limitations for Zone 1:PGA0.08:	
	ground type B	221
6.12	Displacement limitations for Zone 1:PGA0.08:	
	ground type C	221
6.13	Displacement limitations for Zone 1:PGA0.08:	
	ground type D	232
6.14	Displacement limitations for Zone 2:PGA0.08:	
	ground type A	232
6.15	Displacement limitations for Zone 2:PGA0.08:	
	ground type B	233
6.16	Displacement limitations for Zone 2:PGA0.08:	
	ground type C	233
6.17	Displacement limitations for Zone 2:PGA0.08:	
	ground type D	234
6.18	Displacement limitations for Zone 2:PGA0.10:	
	ground type A	234
6.19	Displacement limitations for Zone 2:PGA0.10:	
	ground type B	225
6.20	Displacement limitations for Zone 2:PGA0.10:	
	ground type C	225
6.21	Displacement limitations for Zone 2:PGA0.10:	

xiv

	ground type D	226
6.22	Displacement limitations for Zone 2:PGA0.12:	
	ground type A	226
6.23	Displacement limitations for Zone 2:PGA0.12:	
	ground type B	227
6.24	Displacement limitations for Zone 2:PGA0.12:	
	ground type C	227
6.25	Displacement limitations for Zone 2:PGA0.12:	
	ground type D	228
6.26	Summary of tower assessment attributes	237
6.27	Summary of types of damages	237
A.1	Sample of Cassini Coordinates	264
B.1	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.04, ground type A	265
B.2	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.04, ground type B	265
B.3	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.04, ground type C	266
B.4	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.04, ground type D	266
B.5	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.06, ground type A	266
B.6	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.06, ground type B	267
B.7	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.06, ground type C	267
B.8	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.06, ground type D	267
B.9	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.08, ground type A	268
B.10	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.08, ground type B	268
B.11	Results of tower analysis for joint displacement in Zone 1	

XV

	PGA 0.08, ground type C	268
B.12	Results of tower analysis for joint displacement in Zone 1	
	PGA 0.08, ground type D	269
B.13	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.08, ground type A	269
B.14	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.08, ground type B	270
B.15	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.08, ground type C	270
B.16	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.08, ground type D	271
B.17	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.10, ground type A	271
B.18	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.10, ground type B	272
B.19	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.10, ground type C	272
B.20	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.10, ground type D	273
B.21	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.12, ground type A	273
B.22	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.12, ground type B	274
B.23	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.12, ground type C	274
B.24	Results of tower analysis for joint displacement in Zone 2	
	PGA 0.12, ground type D	275
C.1	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.04, ground type A	276
C.2	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.04, ground type B	276
C.3	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.04, ground type C	277

xvi

C.4	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.04, ground type D	277
C.5	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type A	278
C.6	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type B	278
C.7	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type C	279
C.8	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type D	279
C.9	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type A	280
C.10	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type B	280
C.11	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type C	281
C.12	Results of tower analysis for base shear reactions in Zone 1	
	PGA 0.06, ground type D	281
C.13	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.08, ground type A	282
C.14	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.08, ground type B	282
C.15	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.08, ground type C	283
C.16	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.08, ground type D	283
C.17	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.10, ground type A	284
C.18	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.10, ground type B	284
C.19	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.10, ground type C	285
C.20	Results of tower analysis for base shear reactions in Zone 2	

xviii

	PGA 0.10, ground type D	285
C.21	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.12, ground type A	286
C.22	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.12, ground type B	286
C.23	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.12, ground type C	287
C.24	Results of tower analysis for base shear reactions in Zone 2	
	PGA 0.12, ground type D	287
D.1	Results of tower analysis for axial forces for T30 in Zone 1	
	PGA 0.08, ground type A	288
D.2	Results of tower analysis for axial forces for T30 in Zone 1	
	PGA 0.08, ground type D	288
D.3	Results of tower analysis for axial forces for T30 in Zone 2	
	PGA 0.12, ground type A	289
D.4	Results of tower analysis for axial forces for T30 in Zone 2	
	PGA 0.12, ground type D	289
D.5	Results of tower analysis for axial forces for T45 in Zone 1	
	PGA 0.08, ground type A	289
D.6	Results of tower analysis for axial forces for T45 in Zone 1	
	PGA 0.08, ground type D	290
D.7	Results of tower analysis for axial forces for T45 in Zone 2	
	PGA 0.12, ground type A	291
D.8	Results of tower analysis for axial forces for T45 in Zone 2	
	PGA 0.12, ground type D	291
D.9	Results of tower analysis for axial forces for T90a in Zone 1	
	PGA 0.08, ground type A	292
D.10	Results of tower analysis for axial forces for T90a in Zone 1	
	PGA 0.08, ground type D	292
D.11	Results of tower analysis for axial forces for T90a in Zone 2	
	PGA 0.12, ground type A	293
D.12	Results of tower analysis for axial forces for T90a in Zone 2	
	PGA 0.12, ground type D	293

D.13	Results of tower analysis for axial forces for T90b in Zone 1	
	PGA 0.08, ground type A	293
D.14	Results of tower analysis for axial forces for T90b in Zone 1	
	PGA 0.08, ground type D	294
D.15	Results of tower analysis for axial forces for T90b in Zone 2	
	PGA 0.12, ground type A	295
D.16	Results of tower analysis for axial forces for T90b in Zone 2	
	PGA 0.12, ground type D	295
D.17	Results of tower analysis for axial forces for T120 in Zone 1	
	PGA 0.08, ground type A	296
D.18	Results of tower analysis for axial forces for T120 in Zone 1	
	PGA 0.08, ground type D	297
D.19	Results of tower analysis for axial forces for T120 in Zone 2	
	PGA 0.12, ground type A	297
D.20	Results of tower analysis for axial forces for T120 in Zone 2	
	PGA 0.12, ground type D	298
D.21	Results of tower analysis for axial forces for T140 in Zone 1	
	PGA 0.08, ground type A	299
D.22	Results of tower analysis for axial forces for T140 in Zone 1	
	PGA 0.08, ground type D	299
D.23	Results of tower analysis for axial forces for T140 in Zone 2	
	PGA 0.12, ground type A	300
D.24	Results of tower analysis for axial forces for T140 in Zone 2	
	PGA 0.12, ground type D	301

LIST OF FIGURES

FIGURE NO	O. TITLE			
1.1	National Broadband Initiatives Vision - New Economic Model	2		
1.2	Location of plates	3		
1.3	Collapsed of tower structure after an earthquake event			
	in Indonesia	7		
1.4	Research objectives and methodology	12		
2.1	Technology trends in communication	26		
2.2	Highspeed Broadband landscape for Malaysia	30		
2.3	System Configuration of Asia Submarine Cable Express	34		
2.4	Typical Telecommunication tower	36		
2.5	Worldwide casualities from earthquakes	39		
2.6	MERS 999 solution offerings	49		
2.7	4 legged tower-lattice	50		
2.8	Guyed mast-lattice	51		
2.9	4 legged guyed mast-lattice	51		
2.10	Guyed monopole	52		
2.11	Telecommunication monopole	53		
2.12	Aesthetic monopole	53		
2.13	Lamp pole	54		
2.14	Footing base of tower	55		
2.15	Leg member of tower	56		
2.16	Horizontal member of tower	56		
2.17	Plan bracing of tower	57		
2.18	Diagonal bracing of tower	57		
2.19	Redundant member of Tower	58		
2.20	Hip member of tower	58		

XX

2.21	Top platform and rest platform (botton view)	59
2.22	Rest platform, cage, cable ladder and cables	59
2.23	Hop or caged ladder	60
2.24	Microwave antenna, dish	60
2.25	High rise structures in metro Kuala Lumpur	65
2.26	Devastation of March 11, 2012 Fukushima Earthquake	68
3.1	Story drift determination	101
3.2	Shape of the elastic response spectrum	108
3.3	Recommended Type 2 elastic response spectra for ground	
	Types A to E (5% damping)	109
4.1	Research methodology	123
5.1	Different levels of TM tower maintenance procedure	145
5.2	Organisation of tower assessment program at headquater's	
	level	146
5.3	Suggested Organisational setup of tower assessment	
	program at regional level	146
5.4	Workflow for tower assessment list verification	148
5.5	Conversion of coordinates to Cassini System	151
5.6	PGA map for 500 Year Retun Period based on CIDB	
	for Peninsular Malaysia	152
5.7	PGA map for 500 Year Return Period based on PWD	
	for East Malaysia	153
5.8	Seismic mapping for telecommunication towers for 500	
	Year Return Period for Peninsular Malaysia	154
5.9	Seismic mapping for telecommunication towers for 500	
	Year Return Map for East Malaysia	155
5.10	Simplified work flow for developing the TM	
	Telecommunication towers seismic mapping	156
5.11	Model of T30 tower	157
5.12	Model of T45 tower	158
5.13	Model of T90a tower	158
5.14	Model of T90b tower	159
5.15	Model of T120 tower	159
5.16	Model of T140 tower	160

xxi

5.17	Line drawing of tower models	161
6.1	Joint displacement of towers in Zone 1, 0.04 gals	173
6.2	Joint displacement of towers in Zone 1, 0.06 gals	177
6.3	Joint displacement of towers in Zone 1, 0.08 gals	180
6.4	Joint displacement of towers in Zone 2, 0.08 gals	183
6.5	Joint displacement of towers in Zone 2, 0.10 gals	187
6.6	Joint displacement of towers in Zone 2, 0.12 gals	190
6.7	Base shear reactions of towers in Zone 1, 0.04 gals	193
6.8	Base shear reactions of towers in Zone 1, 0.06 gals	196
6.9	Base shear reactions of towers in Zone 1, 0.08 gals	198
6.10	Base shear reactions of towers in Zone 2, 0.08 gals	201
6.11	Base shear reactions of towers in Zone 2, 0.10 gals	204
6.12	Base shear reactions of towers in Zone 2, 0.12 gals	207
6.13	Weight of towers (kN) against tower height (m)	211
6.14	Height of towers (m) against Natural period (sec)	212
6.15	Axial forces for towers in Zone 1, 0.08 gals in ground	
	Type A	230
6.16	Axial forces for towers in Zone 1, 0.08 gals in ground	
	Type D	231
6.17	Axial forces for towers in Zone 2, 0.12 gals in ground	
	Type A	233
6.18	Axial forces for towers in Zone 2, 0.12 gals in ground	
	Type D	235

xxii

LIST OF SYMBOLS

Design ground acceleration ag _ В Ratio of shear demand or lower bound factor for the horizontal _ design spectrum Deflection amplification factor C_d _ C_s -Seismic response coefficient C_u _ Upper limit on calculated period C_{vx} _ Vertical distribution vector D -Dead load of structure and appurtenances, excluding guy assemblies D_{g} Dead load of guy assemblies; - D_1 Weight of ice due to factored ice thickness; _ Е Earthquake load _ F Seismic force _ Fa Acceleration-based site coefficient based on site class and spectral response acceleration at short periods. F_v Velocity-based site coefficient based on site class and spectral response _ acceleration at short periods. Structural height h_n I_e Seismic importance factor _ Ii Ice importance factor thickness I_i _ I_s Snow importance factor _ Ice importance factor thickness I_w Iw -Total mass of the building т _ \overline{N} Standard penetration resistance _ P_x -The vertical design load at and above level x Behaviour factor qβ The lower bound factor for the horizontal design spectrum _ R -Response modification factor

S	-	Soil factor
S_1	-	1 Seconds spectral acceleration
S_{D1}	-	Design spectral response acceleration at a period of 1.0 second
S _{DS}	-	Design spectral response acceleration parameter in the short period
		range
$S_d(T)$) –	Design spectrum
$S_E(T)$) -	The elastic response spectrum
S_S	-	0.2 Seconds spectral acceleration
\overline{S}_u	-	Soil undrained shear strength
Т	-	Vibration period of a linear single-degree-of-freedom system
Ta	-	Fundamental period
T_1	-	Load effects due to temperature
T_1	-	Fundamental period of vibration of the building for lateral motion in
		the directional considered
T_B	-	The lower limit of the period of the constant spectral acceleration
		branch
T_C	-	The upper limit of the period of the constant spectral acceleration
		branch
T_D	-	The value of defining the beginning of the constant displacement
		response range of the spectrum
V	-	Total design lateral force or seismic base shear of the structure
V_x	-	Seismic shear force acting between level x and x-1
W	-	Effective seismic weight
Wo	-	Wind load without ice;
W_1	-	Concurrent wind load with factored ice thickness.
Ŋ	-	Damping correction factor with reference value of $\eta=1$ for
		5% viscous damping
Ω	-	System Overstrength
θ	-	Stability coefficient
θ_{max}	-	Maximum stability coefficient
Δ	-	The design story drift
Δ_{a}	-	The allowable story drift, Δ_a
λ	-	The correction factor

 δ_x - Deflection at level x

LIST OF ABBREVIATIONS

AAG	-	Asia America Gateway
AASHTO	-	American Association of State Highway and Transportation
		Officials
AEIC	-	ASEAN Earthquake Information Centre
AGCA	-	Associated General Contractors of America (AGCA)
ALI	-	Automatic Location Identification
AM	-	Amplitude modulation
ASE	-	Asia Submarine Cable Express Cable System
ATC	-	Applied Technology Council
CATV	-	Consumer Access Television
CIDB	-	Construction Industry and Development Board
DWDM	-	Dense Wavelength Division Multiplexing
EC	-	European code
EENA	-	European Emergency Number Association
EIA	-	Electronic Industry Association
EN	-	European standards
ETP	-	Economic Transformation Program
FM	-	Frequency modulation
GIS	-	Geographical information system
GNP	-	Gross National Product
GTP	-	Government Transformation program
HERP	-	Headquarters for Earthquake Research Promotion
HSBB	-	High Speed Broad Band
IAEA	-	International Atomic Energy Association
IBC	-	International Building Code
ICC	-	International Code Council
IP	-	Internet protocol

Internet Protocol Television
International Telecommunication Union
Japan Seismic Hazard Information Station
Japan Seismic Centre for Earthquake
Jabatan Telekom Malaysia
Japan-United States Cable Network
Malaysian Communication and Multimedia Commission
Malaysian Emergency Response System
Modified Mercalli
Malaysian Metrological Services
New Economic Model
National Emergency Number Association
Next Generation Network

NGN _ Next Generation Net **NKRAs** National Key Result Areas -

IPTV

ITU

JSHIS

JSCE

JTM

JUCN

MCMC

MERS

MM

MMS

NEM

NENA

-

_

-

-

_

-

-

_

_

-

_

_

- NBI National Broadband Initiatives _
- NRCC National Research Council Canada -

- NTT Nippon Telegraph and Telephone _
- NTTComm -Nippon Telegraph and Telecom Communications
- PCS Personal Communication Service _
- Peak ground acceleration PGA -
- PSP _ Philippine Sea plate
- PWD Public Works Department -
- SMW3 South-East Asia-Middle-Western Europe 3 -
- TeAMS Telekom Asset Management System -
- TIA Telecommunciation Industry Association -
- ΤM Telekom Malaysia Berhad -
- TTAS Telecommunication Towers Assessment System _
- TNB Tenaga Nasional Berhad -
- UBC Uniform Building Code -
- UHF Ultra high frequency _
- USGS United States Geogical Survey -
- VHF -Very High Frequency
- WHO World Health Organisation -
- World wide web www _

xxviii

LIST OF APPENDICES

APPEN	NDIX TITLE	PAGE	
А	Sample of Cassini coordinates for TM towers	264	
В	Results of tower analysis for joint displacements	265	
С	Results of tower analysis for base shear reactions	276	
D	Results of tower analysis for axial forces	288	
Е	Survey questionnaires	302	

CHAPTER 1

INTRODUCTION

1.1 Introduction

Since its independence over the past 55 years, in parallel with the strong emergence of the telecommunication industry a large number of self supporting towers have been erected throughout Malaysia. With the divergence of the high speed broadband projects initiated by the Malaysian government in mid 2008, more telecommunication towers are being and erected to cater for the country's needs. Furthermore with the Government Transformation Programme (GTP) which has been launched by the Prime Minister in 2010 is an ambitious, broad based initiative aimed at addressing key areas of concern to the people while supporting Malaysia's transformation into a developed and high-income nation as per Vision 2020 have been actively participated by everyone in the country.

The GTP is aligned to the New Economic Model (NEM) and the Tenth Malaysia Plan (10MP) and should be viewed together with these initiatives as part of one cohesive effort to transform Malaysia into a progressive, harmonious and high-income nation by 2020 (PEMANDU, 2010). The roadmap of the National Broadband Initiatives (NBI) is illustrated in Figure 1.1.



Figure 1.1 National Broadband Initiatives Vision – New Economic Model (PEMANDU, 2010)

With this National Key Result Areas (NKRAs) targets, more telecommunication towers will need to be erected throughout the country. These specifically light and slender tower structures are particularly sensitive to the environmental loads and also to ground movements. Cantuniar (2011) has expressed that globally the need for speed is becoming pronounced and urgent. Consumer appetite for on the move data consumption via mobile broadband shows no signs of being sated in the face of smart phone and tablet growth. Home and business demand for high-speed internet access is also growing, while the scrutiny on broadband providers is under from watchdogs, home regulators and the commission continues.

Being the country's biggest telecommunication infrastructure planner and service provider, Telekom Malaysia (TM) has to keep up at par with the current needs and trends not only locally but globally besides maintaining the efficiency of its facilities. Telecommunication plays an important role in our daily life. It is primarily concerned with people. Any telecommunications administration must be

judged not by its equipment, but on how well it meets the needs and the aspirations of the people it serves. Zamzairani (2011) has pointed out that one of the driving forces for people at TM is that whatever they do there is of prime importance to the country. The feeling of wanting to contribute to the nation's development is what matters most.

Malaysia is situated on the southern edge of the Eurasian plate. It is close to the two most seismically active plate boundaries, the inter plate boundary between the Indo-Australian and Eurasian plates on the west and the inter-plate boundary between the Eurasian and Philippines plates on the east as seen in Figure 1.2.



Figure 1.2 Location of plates (Adnan. A, 2006)

Major earthquake originating from these plate boundaries has been felt in Malaysia. Peninsular Malaysia is classified as a seismically stable area. As reported in the ASEAN Earthquake Information Center (2009) there have been no known local earthquakes so far except for those occurred in reservoirs such as those occurring at the Kenyir Dam area in Terengganu between 1984 to 1986. However the west of peninsular Malaysia is affected by tremors originating from large

Sumatran earthquakes on the average of 1.5 to 2 tremors a year with maximum intensity of V observed base on the Modified Mercalli (MM) scale. Several possible active faults have been delineated and the local earthquakes in East Malaysia appear to be related to some of them. Based on earthquakes with body wave magnitude of 4 and above located within a radius of 450 kilometer of the island during the period 1976 to 1990, the return periods for different magnitudes were found to be as tabulated in Table 1.1 below.

Table 1.1: Magnitude and Return Period of Earthquakes Events (ASEANEarthquake Information Centre, 2009).

Magnitude(Richter)	4.0	5.0	5.5	6.0	6.5	7.0
Return Period (year)	0.3	1.01	2.07	3.95	7.52	14.30

East Malaysia is classified as a moderately active in seismicity mainly in Sabah. In addition to the local earthquakes, East Malaysia is also experiencing tremors originating from large earthquakes located over southern Philippines and in the Straits of Makassar, Sulu Sea and Celebes. These areas have experienced earthquake origin with magnitudes of up to 5.8 on the Richter scale. Some of these resulted in some damages on properties. The maximum intensity observed so far was VI on the MM scale.

Tjia (1983) mentioned that in Sabah and Sarawak historical and instrumental seismicity recorded the presence of several earthquake epicenters that reflect their present-day tectonic setting. Lim (1985) claimed that on-shore Sabah, the epicenters mark earthquakes of moderate magnitudes that are mostly found in and close to, the Dent and Semporna Peninsulas, where they demarcate a broad zone of mainly shallow foci between the Sulu Trench and Sulu Volcanic Arc. Lim (1986) and Raj (1996) mentioned that in the west of Sabah and Sarawak the epicenters in the South China Sea may represent renewed fault movements, whilst other epicenters particularly in Sarawak show no clear relationship with the tectonic setting. Due to its strategic location, Malaysia is generally spared from any major active seismic

activities. However, when earthquakes occur in neighbouring countries, the effects can be felt locally even though the epicenter of the earthquake is hundreds of kilometers away. Again Lim (1977) and Godwin (1992) both claimed that substantial damage to buildings have been reported on July 26, 1976 and on May 26, 1991 in Tawau, Lahad Datu and Ranau, Sabah, respectively. While Adnan et al. (2005) in their study which included several items such as the tectonic setting of Sumatra, location, mechanism and size of the recent earthquake and also analysis of ground at bedrock for Penang and Kuala Lumpur using several appropriate attenuation relationships has shown that the Sumatra Earthquake did have some effect to the Malaysian Peninsular. In 2007, the inhabitants of Bukit Tinggi, Pahang have experienced tremors due to minor movements from the earth. Although there were no reports of major structural damage, the incident has raised several questions. As reported in the Jurutera Bulletin (2008); one of the major concerns is this; "Are existing high rise buildings in Malaysia able to withstand such tremors and should future developments be designed for seismic effects?" Again, on September 30, 2009, another earthquake measuring 7.6 Magnitude location of epicenter at 60 km southwest of Padang Sumatera, followed by subsequent moderate magnitude 6.6 quake occurred at the same spot about 20 minutes later have caused great fear to our locals especially in the Peninsular Malaysia (USGS). Thousands of workers in several cities in the country fled their high rise offices and homes as tremors shook the buildings. Reports on calls and complaints by the public were made to the Fire Department in most of the states due to the tremors felt and the concern of their safety on their dwellings and offices. These were documented in the Executive Report on Typhoon and Earthquake Disaster (2009) by the Fire Department of Malaysia.

Similar question arose indicating one of the major concerns in regards to the existing buildings and structures in the country being able to withstand earthquake events in future. If not, what are the actions and necessary steps to be taken to mitigate this disaster? After the 2004 tsunami disaster that struck Aceh, the Malaysian government under the National Security Council has taken early initiatives to look into the impact of earthquake events originating from our neighbours. The Standard Operating Procedure (SOP) for Managing Earthquake

Disaster (2007) which rules out the guidelines and the responsibilities of all relevant departments and agencies in handling and managing such disaster has been drafted. This is to ensure that the operations will runs smoothly and systematically in facing such events. Further discussions on this are made in the proceeding chapters.

Important buildings and sensitive structures such as telecommunication towers are among the most crucial to be looked upon. This is because tower structures play an important role in enabling communication during disaster to be broadcasted to the public without any failure. Lomnitz (1974) mentioned that it is important to be noted that earthquakes do not need to be of large magnitude to produce severe damage, because the degree of damage depends not only on the physical size of an earthquake but also on other factors such as where and when an earthquake occurs, the population density in the area and secondary related events such as fire.

Managing telecommunication structures is of utmost importance to ensure that services are uninterruptable and can be delivered during these hard times. Communication needs during disaster are unique and critical. It becomes more crucial when disaster such as flood, typhoon, hurricane and earthquakes events happen. According to Kramer (1992), in a major emergency caused by an earthquake it is likely that telephone lines may be down, other alarm and telecommunications facilities are adversely affected, and a vast increase in the work load imposed upon personnel and equipment in the control centre. One distinguishing characteristics is the dramatic increase in the number of people who must make use and communicate among them. The malfunction of the communication facilities immediately after an earthquake struck other countries should be a lesson learned especially for telecommunication service providers.

Telecommunication towers are categorised among the tallest man-made structures and can be found standing high on every part of the globe with different heights and purposes. McClure (1999) quoted a survey of the earthquake performance of communication structures that summarised documented reports of 16 instances of structural damage related to seven important earthquakes between 1949 to 1998, none of which were a direct threat to life safety. However, several towers may have been damaged or have become unserviceable without having collapsed or suffered damage visible from the ground during post earthquake inspections. Many strong earthquakes have happened since then and more damage has been reported as more telecommunication equipment is deployed worldwide. Indonesia, the country that lies in the Ring of Fire area has witnessed many of its telecommunication towers failed during earthquakes events. This can be seen in Figure 1.3.



Figure 1.3 Collapsed of tower structure after an earthquake event in Indonesia (PT. XL Axiata Tbk., Indonesia 2011).

Also Bahme et al. (1992) highlighted that in any disaster scenario quality decisions require the communication of timely, valid, and usable information among a very large number of individuals and agencies during this event. Information that needs to be communicated generally involves guidance, direction, requests for assistance, status reports on the incident and updates on resources and operations. According to Faridafshin et al. (2008) the preservation of serviceable communication infrastructure as critical links of communication or post disaster networks is essential in the event of an earthquake. The January, 1995 Kobe earthquake is a good example where communication facilities malfunction has given a big impact where

this event was said to have prevented local governments from knowing the level and the scope of casualties caused by the disaster. Their poor reaction has increased the number of the fatalities affected (Smith, 2007). The same scenario happened in most of the world's earthquake prone countries like Indonesia, Haiti, New Zealand and others. The failure of communication has worsened the disaster effects to the victim. As has been reported in the Mainichi Shimbun (2011) in the March 11, 2011 Tohoku earthquake, communications were badly broken, with cell phone service largely knocked out; many residents had to rely on the small number of surviving pay phones. Undoubtedly, the preservation of serviceable communication infrastructure as critical links of communication or post disaster networks is essential in the event of a severe earthquake and this issue requires attention in not only the seismic-prone regions of the world but also areas that felt earthquake effects. This can no longer be compromised.

Telecommunication towers are exposed to numerous environmental loads and imposed live loads which increase from time to time and also due to rugged site locations. These can cause a reduction in overall strength and will lead to eventual failure of the towers. Bai et al. (2010) who conducted a study on transmission towers and power lines have proven that seismic responses are amplified to these structures when considering the local site effect. Telecommunication towers are also expected to have experienced the same effects on its structures. As mentioned by Bahme (1992), successful communication in information management for disaster control requires a good organisation that includes proper managing of its structures that will not affect transmission or news broadcasting during disaster.

Since TM possesses many high structures including hundreds of telecommunication towers nationwide, therefore it is of vital importance to understand and monitor the safety of all its structures. Assessing the condition of the structures by taking earthquake effects into consideration is deemed necessary to determine its structural integrity, safety and reliability.

1.2 Problem Statement

Currently towers are evaluated only through visual observation and inspections. The major problem with visual inspection is the inherent variability that occurs naturally when subjective observations are carried out without proper guidance. Telecommunication tower evaluation method may vary according to personal judgment. Thus, large uncertainties exist in the interpretation of inspection data. Besides there is no such standardised assessment system yet in TM or other operators in managing its telecommunication towers nationwide. If there were, the inspection requirement would differ from one operator to another and would not cover all aspects of the tower structures including its surrounding conditions.

Local authorities too like the Kuala Lumpur City Hall and also major players in the construction industry like the Public Works Department (PWD) and also Construction Industry and Development Board (CIDB) do not have such guidelines yet especially in inspection of their building structures. Different agencies have different guidelines to be followed but not implemented as an act by the government. In the event of an earthquake, the Fire Department is called upon to check and ensure the safety of the building before tenants are allowed to go back to their homes or offices. The fireman could not check the connections as they are covered. They don't have the expertise in this field to properly justify the safety of the structures and rely on other parties for evaluation. By the time the evaluation is completed, it might already be too late for taking any preventive measures?

To date there is no specific study or research that has been made or carried out in TM or in the country specifically related to the issues and areas addressed. As for TM's practices, only normal visual inspections have been carried out in its maintenance program for its tower structures and if defects are encountered it will only be rectified based on request and urgency. Specifically, earthquake related scenario has not been considered and nothing has been carried out so far.

As mentioned earlier, Malaysia is now being exposed to earthquake effects though it is located far away from its epicenter. As has been reported in the Executive Report on Typhoon and Earthquake Disaster (2009), that Malaysia emerged relatively unscratched from the September 2009 disaster with only a dozen buildings reporting cracks compared to 2600 buildings damaged or destroyed in Padang including thousands trapped under collapsed buildings. Since our buildings are generally not designed to withstand earthquake loads, it is better to prevent huge losses to assets and life rather than 'curing' it later. According to Kramer (1992) disaster such as earthquake will affect the economy and development of the nation, destroying means of production, distribution, and transportation of commercial products, and disruption of communications and public utility services. This is noticeably true. The triple catastrophe in Fukushima 2011 has proven this and causing Japan to face a downturn in its economic. As reported in the International Business Times (2012), exports fell 9.3 percent because of the disruption of supplychain processes due to Thai floods, slower growth in China's economy and a weakening euro that made Japanese products more expensive. Imports fell 9.8 percent because of increased demand for energy imports in a country whose nuclear power production levels have yet to recover from the devastating tsunami and earthquakes. Prior to 2011, Japan had not run a trade deficit in 31 years.

In The Street, Drainage and Building Act (1984) it is clearly stated that buildings or structures reaching the age of five to twelve years need a thorough inspection to determine its durability, integrity besides its safety for occupants. Besides that clause 5.1 and Clause 5.2, 'Actions After Construction' as stated in the Guide for Construction of Towers and Telecommunication Broadcasting Structures Systems Under Local Authority (Ministry of Housing and Local Government, 2002) clearly states the "requirements that needed to be complied by developers or networks services provider to ensure that all structures after erection need to be properly maintained but also to ensure the safety of the property and also the public at large. Scheduled inspection should be carried out regularly but not less than once in every twelve months after the date of certificate of fitness have being issued by the local authority".

In Malaysia, most of the existing telecommunication towers were erected way back before the 1950's especially for those structures meant for carrying microwave purposes. The British and Japanese erected it earlier during their occupation in Malaya and all are still operational today. During those days, the towers are designed only to cater for the normal loads and no additional factors or specific considerations of seismic events were considered. Generally these tower structures are more than ten years of age while some reaching almost sixty years of service life and no thorough assessment have been carried out on them. Most of these structures are located on highland areas which are also suspected to have experienced some effects due to earthquake events and movements of faults lines besides ground motions. Due to their tall and tapered shape and being exposed to environmental factors daily, it is of outmost importance to investigate the safety, reliability and structural integrity of these tower by taking into consideration earthquake effects in the country.

1.3 Aim of Research

The main aim of this research work is to develop a telecommunication tower assessment system by considering earthquakes effects in Malaysia which can serve as a national guide for all tower owners and operators in the country, thus helps to promote a uniform standard of practice among various parties.

1.4 Objectives of Research

To achieve such aim the following objectives have been considered for the research work:

- i. To produce standard visual inspection procedures for telecommunication tower structures by considering earthquake effects.
- ii. To evaluate the structural integrity of the existing telecommunication tower structures due to the effects of earthquakes.

iii. To develop a Telecommunication Tower Assessment System by considering earthquakes effects in the country.

To achieve the above objectives, a specific research methodology has been carried out and explained in detail in Chapter 4 of this thesis and also as simplified in Figure 1.4 below.



Figure 1.4 Research objectives and methodology

1.5 Scope of Research

The focus of this research has been narrowed down to only on structural related issues for assessment on four (4) legged self-supporting steel towers. Due to the huge numbers of four (4) legged self-supporting steel towers type being erected,

it is most important to prioritise due to its importance in the telecommunication purposes. Several steel telecommunication towers of different height categories have been selected for this purpose. The research work includes investigating, accessing, analysing and evaluating, including modeling the selected towers located in various seismic zones in the country.

Scopes of work are listed below:

i) Types

4 legged Self Supporting steel towers were selected. Several towers of different heights that are categorised in the medium rise and high rise structures were selected.

The medium rise category was considered when the height of the tower lies within 19.81 meters and 73.15 meters while for the high rise category was considered when the height is above 73.15 meters (UBC,1994). No low rise category was carried out since tower under this height are located on building roof tops.

For medium rise category, two towers of height 30 meters and 45 meters were selected for modeling and analysis, while for high rise category four towers of height 90 meters, 120 meters and 140 meters were selected. The categories of the towers are tabulated in Table 1.2.

Tower Categories	Height of Tower (meter)
Lower Rise (< 19.81 meter)	Nil (No tower in this category)
	30
Medium Rise ($19.81 \le H \le 73.15$)	45
	90
High Rise (H> 73.15)	120
	140

Table 1.2: Heights of Towers for Modeling and Analysis

The selected towers are located in the various seismic zones as indicated in the CIDB 500 Years Return Period map for Peninsular and the PWD 500 Years Return Period for East Malaysia map as tabulated in Table1.3. This table shows that for Peninsular Malaysia, the maximum of peak ground acceleration for a Year Return of 500 years is less than 100 gals while for East Malaysia is less than 120 gals and in various seismic zones. This follows accordingly as in seismic map in Figure 5.6 and 5.7 in Chapter 5.

 Table 1.3:
 500 Year Return Period Based on CIDB for Peninsular Malaysia and PWD for East Malaysia

500 Year Return Period Based on CIDB for Peninsular Malaysia	Seismic Zone	500 Year Return Period Based on PWD for East Malaysia	Seismic Zone
20 - 40 gals	0	60 - 80 gals	1
40 - 60 gals	1	80 - 100 gals	2A
60 - 80 gals	1	100 - 120 gals	2B
80 - 100 gals	2A	-	

iii) Seismic zone

The study was carried out in various zones. The zones are Zone 1, Zone 2A and Zone 2B. Table 1.4 summarises the seismic zones identified in Malaysia ranging from Zone 0 to Zone 4 (Refer Table 1.4).

 Table 1.4:
 Seismic Zones in Malaysia

Peak Ground Acceleration (gals)	Seismic Zone	Seismic Zone Factor (Z)
0 - 40	0	0.0
41 - 80	1	0.075
81 - 100	2A	0.15
101 - 150	2B	0.20
151 - 300	3	0.30
301 - 500	4	0.40

iv) Peak Ground Acceleration

In the design codes like IBC and Euro Code, Design Base Earthquake (DBE) is considered to have a return period of 475 years. Therefore, following these codes, if we want to design structures the DBE must be in reference for the calculation of the earthquake load.

The 500 year return period is preferably to be used in the research work is because in Malaysia the DBE is also the commonly used return period for the design of structures against earthquake loads.

Various values of peak ground acceleration were selected for the purpose of analysis. Since the study was carried out in the 500 year return period, three (3) ground accelerations in Zone 1 and three (3) in Zone 2. (Refer Table 1.5) This table shows the peak ground acceleration (pga) that has been selected and categorised meant for tower analysis. The zones are breakdown into various pga's within each dedicated range to enable a more specific pga value to be used for analysis purposes.

No	Seismic Zone	Peak Ground Acceleration (gals)
1	Zone 1	0.04
2	Zone 1	0.06
3	Zone 1	0.08
4	Zone 2	0.08
5	Zone 2	0.10
6	Zone 2	0.12

 Table 1.5: Peak Ground Acceleration for Tower Analysis

(v) Selection of Analysed Towers

From structural point of view, since the study is concerning on the seismic behavior of 4 legged self-supporting towers and the scope carried out includes the different tower height ranges in the medium and high rise category; not the geometry of the towers.

For this study, two numbers of 4 legged self supporting steel tower from medium rise category and 4 numbers from high rise category are selected. There are six (6) numbers of towers in the respective category to be modeled in four (4) types of ground conditions and analysed on the different seismic zones.

These tower samples that are studied cover all of the current existing heights of towers in the country. They are anlaysed in different types of parameters such as the different seismic zones, peak ground acceleration and also ground types. These samples of tower are enough for the scope of the study on seismic behavior of the towers considering earthquake effects in Malaysia.

The summary is as tabulated in Table 1.6 below. The total numbers of analyses carried out were ninety six (96).

Tower Category	Tower Id	Height of Tower (meter)
Medium Rise	Т30	30
Weddulli Kise	T45	45
High Rise	T90a	90
	T90b	90
	T120	120
	T140	140

Table 1.6: Heights of Towers for Modeling and Analysis

(vi) Ground Type

For this study various ground types were selected for each seismic zone for all the towers. These ground types that fall under the normal Malaysian condition were considered and used in the analysis work where the ground condition that has been applied in Euro Standards (EC8) and International Building Code (IBC) were referred. (Refer Table 1.7 for EC ground types and Table 1.8 for IBC ground types).

In EC8, Ground types A, B, C, D, and E, are described by the stratigraphic profiles and parameters given in Table 1.7 and described herein, were used to account for the influence of local ground conditions on the seismic action. This was done by additionally taking into account the influence of deep geology on the seismic action.

Ground Type	Description of stratigraphic profile	Parameters		
		vs,30 (m/s)	NSPT (blows/30cm)	cu (kPa)
А	Rock or other rock-like geological formation, including at most 5 m of weaker material at the surface.	> 800	-	-
В	Deposits of very dense sand, gravel, or very stiff clay, at least several tens of metres in thickness, characterised by a gradual increase of mechanical properties with depth.	360-800	>50	>250
С	Deep deposits of dense or mediumdense sand, gravel or stiff clay with thickness from several tens to many hundreds of metres.	180-360	15-50	70-250
D	Deposits of loose-to- medium cohesionless soil (with or without some	<180	<15	<70

Fable 1.7: Ground Typ	es in European	Standards (Star	ndard EN-1:2003)
------------------------------	----------------	-----------------	------------------

	soft cohesive layers), or			
	of			
	predominantly soft-to-			
	firm cohesive soil			
	A soil profile consisting			
	of a surface			
	alluvium layer with v_s			
	values of type C			
Б	or D and thickness			
Ľ	varying between			
	about 5 m and 20 m,			
	underlain by			
	stiffer material with $v_s >$			
	800 m/s.			
	Deposits consisting, or			
	containing a			
	layer at least 10 m			
C	thick, of soft	< 100		10.20
\mathbf{S}_1	clays/silts with a high	clays/silts with a high (indicative)		10-20
	plasticity index	× ,		
	(PI $>$ 40) and high			
	water content			
	Deposits of liquefiable			
S_2	soils, of			
	sensitive clays, or any			
	other soil profile			
	not included in types A			
	$-E \text{ or } S_1$			

 Table 1.8:
 Ground Types in International Building Code (IBC: 2009)

		AVERAGE PRO	DPERTIES IN	TOP 100 feet
SITE CLASS	PROFILE NAME	Soil shear wave velocity, \bar{v} , (ft/s)	Standard penetration resistance, \overline{N}	Soil undrained shear strength, \bar{s}_u , (psf)
А	Hard Rock	\bar{v}_{s} > 5,000	N/A	N/A
В	Rock	$2,500 \ \bar{v}_s \leq 5,000$	N/A	N/A
С	Very dense soil and soft rock	$1,200 < \bar{v}_s \leq 2,500$	$\overline{N} > 50$	$\bar{s}_u \ge 2,000$
D	Stiff soil profile	$600 \leq \bar{v}_s \leq 1,200$	$15 \leq \overline{N} \leq 50$	$1,000 \le \bar{s}_u \le 2,000$
E	Stiff soil profile	$\bar{v}_s < 600$	$\overline{N} < 15$	$\bar{s}_u < 1,000$
E	-	Any profile with mo the following charac 1. Plasticity index <i>P</i>	bre than 10 feet exteristics: I > 20,	of soil having

		2. Moisture content $w \ge 40\%$, and
		3. Undrained shear strength $\bar{s}_u < 500 \text{ psf}$
F	-	Any profile containing soils having one or more of
		the following characteristics:
		1. Soils vulnerable to potential failure or collapse
		under seismic loading such as liquefiable
		soils, quick and highly sensitive clays, collapsible
		weakly cemented soils.
		2. Peats and/or highly organic clays ($H > 10$ feet of
		peat and/or highly organic clay where
		H = thickness of soil)
		3. Very high plasticity clays ($H > 25$ feet with
		plasticity index $PI > 75$)
		4. Very thick soft/medium stiff clays ($H > 120$ feet)

1.6 Significance of the Research

This research considers earthquake effects on telecommunication tower structures mainly for the 4 legged types which are mostly found in the country. The researcher has managed to locate all the towers owned by TM on the 500 year return period seismic map. This map will enable TM to identify the location of the most critical towers in the various seismic zones that may have great exposure from earthquakes events.

The study is the pioneer in its field because this is the first time that an assessment system for 4 legged self supporting towers by considering earthquakes effects in the country is prepared. The tower assessment system consists all of the important attributes for inspection purposes, types of damages and also damage criterion not only for the tower structures but also related to the surrounding conditions.

The research is also able to determine the integrity of selected tower samples under various seismic zones in response to different peak ground accelerations and different ground types acted on it. This helps to understand the seismic behavior experienced by these towers. The developed system thus helps to promote a uniform standard of practice among various parties besides assisting the relevant authority in making preparation from an emergency-management and hazard-preparedness perspective.

1.7 Contribution of the Research

The contribution of this research is to prove that the telecommunication tower assessment system considering earthquake effects in Malaysia will help to improve the capability of the industry in facing such situation and can be seen in terms of benefits gained by both the TM Group and the nation too in addressing such issues.

In addition, the tower distribution located in the seismic map prepared for TM covers all the 4 legged self supporting towers and can be used to predict the probability of imposed damage experienced by far fields effects of earthquakes especially related to ground movements. Moreover this helps the management to further prioritise resources in their yearly budget planning for maintenance work. The impact of the research on the New Economic Model is that it will help to promote a systematic and continuous interaction between the Knowledge Triangle -Higher Education, Research in Industry and Innovation. This will further improve the capability of the industry in facing such situations and can be seen in terms of benefits gained by both the TM Group and the nation. The research will help us to understand the impact and be able to mitigate besides prevent huge losses to assets and life caused by disaster such as earthquake events which will affect the economy and development of the nation, destroying means of production, distribution, and transportation of commercial products, and disruption of communications and public utility services. It will also help to boost the economic development especially in providing the best telecommunication services and also to transform Malaysia into a competitive, knowledge-based and innovative that will drive the country towards economic prosperity.

1.8 Organisation of Thesis

The organisation of the thesis can be described briefly as follows:

- Chapter 1 is the Introduction, which explains on the background, the aim, objectives, scope of research and limitations of the research.
- Chapter 2 is on Literature Review which explains the telecommunication facilities, infrastructure and society, the company overview where the research has been carried out, matters related to policy and ethics practiced in Telekom Malaysia (TM), its responsibility in the nation's telecommunication infrastructure and development in providing services to the nation. Overview of the importance of telecommunication towers and matters on maintenance cultures related to structures in Malaysia are also discussed here. Discussion on earthquakes matters; its effects and impacts, overview of the Malaysian authorities' actions on earthquakes effects including reviews of current engineering design and construction standards are also included. The importance of seismic mapping for the country is also discussed.
- Chapter 3 presents the theoretical background of linear analysis besides earthquake consideration in tower analysis using both EC and IBC codes besides other such as American Society of Civil Engineer (ASCE), Electronic Industry Association (EIA) and Telecommunication Industry Association (TIA).
- Chapter 4 is Research Methodology which explains the methodology to complete the research besides the data collection and analysis technique used in the study.
- Chapter 5 is on Model Development which details out the development of the tower assessment list, seismic mapping of telecommunication towers in the 500 year return period map, modeling and analysis works using SAP 2000 software carried out on the samples of selected structures.

- Chapter 6 is on Results and Discussion that discusses the highlights of the research where the results and analysis of towers are detailed out. Statistical analyses of feedbacks on questionnaires obtained from selected respondents are also discussed in detailed at the later part of the chapter.
- Chapter 7 is on Conclusion and Recommendations which explains the significance of the research findings including recommendation or suggestion and benefit of the research for future comparative study.

REFERENCES

- ACT 133 (1984). Street, Drainage and Building Act 1984. Laws of Malaysia.Malaysia: The Commissioner of Law Revision, Malaysia, Malaysian law Journal Sdn. Bhd. and Percetakan Nasional Malaysia Bhd.
- ACT 3 (1969). Civil Aviation Act 1969. Laws of Malaysia. Malaysia: The Commissioner of Law Revision, Malaysia, Malaysian law Journal Sdn. Bhd. and Percetakan Nasional Malaysia Bhd.
- Advertising Age (2007).100 Leading National Advertisers .TNS Media Intelligence (http://adage.com/images/random/datacenter/2008/spendtrends08.pdf).
- AGCA 82 (1982), Associated General Contractors of America, Our Fractured Framework; Why America Must Rebuild, Washington D.C.
- AISC Web Site, http://www.aisc.org [cited January 2009]
- ASCE Standards [ASCE/SEI 7-10], Minimum Design Loads for Buildings and Other Structures. American Society of Civil Engineers, Reston, Virginia.
- ASCE Manual and Report on Engineering Practice No 72 (ASCE Manual 72). (1990). Design of Steel Transmission Pole Structures, 2nd. Ed., American Society of Civil Engineers, Reston, Virgina.
- ASCE Manual and Report on Engineering Practice No 74 (ASCE Manual 74). (1991). Guidelines for Electrical Transmission Line Structural Loading, American Society of Civil Engineers, Reston, Virgina.
- ASEAN Earthquake Information Centre (AEIC) Web Site, http/www.aeic.org/ [cited April 10].
- AASHTO (2002). Standard Specification for Highway Bridges. 17th ed., American Association of State Highway and Transportation Officials, Washington, DC.

- AASHTO (2004). LFRD Bridge Design Specifications. 3rd. ed., American Association of State Highway and Transportation Officials, Washington, DC.
- ATC (1981). Seismic Design Guidelines for Highway Bridges: Provisional Recommendations. Report No. ATC-32., Applied Technology Council, Redwood City, CA.
- ATC (1996). Improved Seismic Design Criteria for California Bridges. Report No. ATC-6., Applied Technology Council, Redwood City, CA.
- Alexander, G. (1975). Possible Link Between Great Quakes Studied, Los Angeles Times, April 7, 1975, Part II, p.1.
- Ali, R. The Role of Telecommunications in Economic Growth: Proposal for An Alternative Framework of Analysis. Media, Culture & Society 19(4):557-583.
- Amiri, G.G.,(1997). Seismic Sensitivity of Tall Guyed Telecommunication Towers. Ph.D Thesis, Dept. of Civil Engineering and Applied Mechanics, McGill University. Montreal, Quebec, Canada.
- Amiri, G.G., Zahedi, M.A., and Jalali, R.S., (2004). Multiple- Support Seismic Excitation of Tall Guyed Telecommunication Towers. 13th World Conference on Earthquake Engineering, Vancouver, British Colombia, Canada, August 1-6, 2004, Canadian Association for Earthquake Engineering, Paper No. 212.
- Amiri, G.G., Barkhordari, M.A., Massah,S.R., and Vafaei,M.R., (2007). Earthquake Amplification Factors for Self-Supporting 4-Legged Telecommunication Towers. World Applied Sciences Journal 2 (6): 635-643.
- Arditi, D., and Nawakorawit, M. (1999). Issues in Building Maintenance: Property Manager's Perspective. Journal of Architectural Engineering. 117-132, December 1999.
- Assi,R. and McClure,G. (2007). A Simplified Method for Seismic Analysis of Rooftop Telecommunication Towers. Canadian Journal. Civil Engineering 34: 1352-1363.
- Adnan, A., Hendriyawan and Masyhur, I, (2002). The Effect of the Latest Sumatra Earthquake to Malaysian Peninsular, Malaysian Journal of Civil Engineering, Vol. 15 No.2.
- Adnan, A., Hendriyawan, Marto, A. and Masyhur, I, (2006). Development of Seismic Hazard Map for Peninsular Malaysia. Proceeding on Malaysian Science and Technology Congress. Kuala Lumpur, Malaysia. 18-26 September.

- Bahme, C.W. (1978). Fire Officers Guide to Disaster Control. 1st. ed. Boston MA. NFPA. Pg70.
- Bahme, C.W. (1992). Fire Service and The Law. Boston MA. NFPA. No PSP-3A.
- Bai. F.L., Li, H.N. and Hao. H., (2010). Local Site Effect on Seismic Response of Coupled Transmission Tower-Line Systems. ASCE, 161.139.200.238 [accessed 12 January 2011].
- Barret, P. (1995). Facilities Management; Towards Better Practice. Oxford, U.K: Blackwell Science.
- Bilman, R. (1988). Earthquakes and Urban Growth. Nature, 336, 626-626.
- Bingel, N.G. and Niles, K.D. (2009). Assessment and Repair of Steel Tower & Steel Pole Foundations. AISC Engineering Journal., AISC, 20(10).
- Bernreuter, D.L. (1989). Seismic Hazard Characterisation of 69 Nuclear Power Plant Sites East of the Rocky Mountains, U.S. Nuclear Regulatory Commission, NUREG/CR-5250.
- Boen, T. (1978), Manual of Earthquakes Resistant Building (Housing), in Indonesia. p105.
- Bonila, M.G. (1984). Statistical Relations Among Earthquake Magnitude, Surface Rupture Length and Surface Fault Displacement. Bull. Seis. Soc. Am., 74(6), 2379-2411.
- Bolt, B.A., (1993). Earthquakes, W.H. Freeman and Company, New York.
- Borg, S.F. (1988). Earthquake Engineering. (2nd & Revised ed.) NJ: World Scientific Publishing Co Pte Ltd.
- Boulle, P.L., (1990). Will The 1990s Be A Decade of Increasingly Destructive Natural Disaster? Earthquake & Volcanoes, 22, 173-175.
- British Standard Document, DD21 : Quality Gradings for Steel Plate.
- British Standards Institution (1968). Methods for Non Destructive Testing of Plate Materials. London, BS 4336.
- British Standards Institution (1973). Methods for Radiographic Examination of Fusion Welded Butt Joints in Steel. London, BS 2600.
- British Standards Institution (1986). Guide to the Use of Nondestructive Testing Method of Test for Hardened Concrete. London, BS 1881: Part 201.
- British Standards Institution (1989). Guide to Assessment of Concrete Strength In Existing Structures. London, BS 6089.

- British Standard Institution. BS 8210: (1986). British Standard Guide to Building Maintenance Management.
- British Standard Institution. BS 3811: (1993). Glossary of Terms Used In Terotechnology.
- Brockenbrough, R.L. (1983). Consideration in the Design of Bolted Joints for Weathering Steel. AISC Engineering Journal., AISC, 20(10) pp 40-45.
- Brown, J., E.V.D. Glazier (1974). Telecommunications, Chapman and Hall Ltd, London.
- Building Research Establishment (1977). Simplified Method for Detection and Determination of Chloride in Hardened Concrete, BRE Information IS 12/77.
- Bungey, J. H (1994). The Testing of Concrete in Structures. 2nd Edition. Blackie Academic & Professional, Chapman & Hall, London.
- Campbell, K.W. (1997). Empirical Near-Source Attenuation Relationships for Horizontal and Vertical Components of Peak Ground Acceleration, Peak ground Velocity, and Pseudo-Absolute Acceleration Response Spectra. Seismol. Res. Lett., 68(1), 154-179.
- Campbell, K.W. and Bozorgnia, Y. (2003). Updated Near-Source Ground Motion (Attenuation) Relations for the Horizontal and Vertical Components of Peak Ground Acceleration and Acceleration Response Spectra. Bull. Seismol. Soc. Am., 93(1), 314-331.
- Cantuniar, N. (2011). The Need for Speed. European Communications. European Comms e-newsletter, Summer 2011. p 36. Web Site, http/www.eurocomms.com.
- Chan, K.T., Lee, R.H. K. and Burnett, J. (2001). Maintenance Performance: A Case Study of Hospitality Engineering Systems. Facilities 19(13/14):494-503.
- Chanter, B. and Swallow, P. (1996). 'Maintenance Organisation' 'Building Maintenance Management'. London: Blackwell Science.
- Chang, P. C., and Liu, S. C. (2003). Recent Research in Nondestructive Evaluation of Civil Infrastructures. Journal of Materials in Civil Engineering. Vol. 15 (3):298-304.
- Chen, W.F. and Toma, S.(1994). Advanced Analysis of Steel Frames. CRC Press, Boca Raton, Florida.
- Chen, W.F. and Duan, L., Ed. (2000). Bridge Engineering Handbook, CRC Press, Boca Raton, Florida.
- Chen, W.F and Lui, E.M. (2006). Earthquake Engineering for Structural Design. N.W: Taylor & Francis Group.

- Cheng, W. (2000). Spectrum Simulation Models for Random Ground Motions and Analysis of Long-Span Bridges Under Random Earthquake Excitations, Doctoral Dissertation, Hunan University, Changsha, China.
- CIRIA Technical Note 143. (1992). Testing Concrete in Structures. United Kingdom.
- Coenraads, R., (2006). Natural Disasters and How We Cope. Millennium House Pty Ltd, Australia.
- Concrete Society of United Kingdom (1987). Technical Report TR11 on Core Testing,
- Day, W.R., (2002). Geotechnical Earthquake Engineering Handbook. McGraw-Hill U.S.A.
- Dhir, K.R., Jones, M.R., Zheng, Li, (2002) Repair Rejuvenation and Enhancement of Concrete, Thomas Telford Publishing.
- Digital Access Index (DAI) (http://www.itu.int/ITU-D/ict/dai).
- itu.int.http://www.itu.int/ITU-D/ict/dai/. [Retrieved March, 2008]
- Dowrick, D.J., (2003). Earthquake Risk Reduction, p15-43. John Wiley & Sons, West Sussex, England.
- European Standards [EN 1998-1], Eurocode 8: Design of Structures for Earthquake Resistance- Part 1: General Rules, Seismic Actions and Rules for Buildings. Supersedes ENV 1998-1-1:1994, ENV 1998-1-2:1994, ENV 1998-1-3:1995, December (2004).
- Earthquake Engineering Research Institute Web Site, http://www.eeri.org/.
- Edward, L. (1998) A Communication Theory Perspective on Telecommunication Policy. Journal of Communication 48 (4):3-23.
- European Communications, European Comms e-newsletter. Autumn 2011. Web Site, http/www.eurocomms.com.
- Everett, L.M. and Treadway, K.W.J. (1980). Deterioration Due to Corrosion in Reinforced Concrete, Building Research Establishment.
- Executive Report on Typhoon and Earthquake Disaster, 30th September, 2009, (2009). Fire Department Headquarters, Putrajaya, Malaysia.
- Faridafshin,F. and Mc. Clure,G, (2008). Seismic Response of Tall Guyed Masts to Asynchronous Multiple-Support and Vertical Ground Motions ASCE, Journal of Structural Engineering.
- Farr, R. E., (1988), Telecommunications Traffic, Tariffs and Costs: An Introduction for Managers, Institution of Electrical Engineers Series 19, Peter Peregrinus Ltd, London.

- Frangopol, D.& Enright, M.P. (1999). Maintenance Planning for Deteriorating Concrete Bridges. ASCE, Journal of Structural Engineering, 125(12): 1407-1414.
- Freeman. R.L. (1989). Telecommunication System Engineering. (3rd ed) New York: John Wiley & Sons.
- Ferry-Borges, J. and Castenheta, M. (1971). Structural Safety. Laboratoria Nacional de Engenhera Civil, Lisbon.
- Figg, J.W., (1973). Methods of Measuring the Air and Water Permeability of Concrete, Magazine of Concrete Research 25(85) pp 213-219.
- Fujita, M., Schall, G. & Rackwitz, R. (1989). Adaptive Reliability Inspection Strategies For Structures Subject To Fatigue. In Proc. ICOSSar89, San Francisco, USA, pp. 1619-1626.
- Galvez C, Mc Clure, G. (1995). A Simplifield Method for Aseismic Design of Self-Supporting Lattice Telecommunication Towers. Proceedings of the 7th Canadian Conference ob Earthquake Engineering, Montreal, Canada, p. 541-548.
- Grigg, C., (1988). Design for Sustainability, A Sourcebook of Integrated Eco-logical Solutions, Earthscan Publications Ltd. London, UK.
- Grimm, C.M. (1994). The Future of The Services Delivery Process. In: K. Alexander (ed.) Facilities Management. Glasgow: CFM, University of Strathclyde.
- Gobbet, D.J and Tjia , H.D. (1973). Tectonic History. Chapter 10 in Gobbet, D.J. and Hutchinson, C.S. (Eds), Geology of the Malay Peninsula. p 305-334. Wiley-Interscience, New York.
- Guide for Construction of Towers and Telecommunication Broadcasting Structures Systems Under Local Authority, Ministry of Housing and Local Government Malaysia. (2002).
- Gurley, C., Progressive Collapse and Earthquake Resistance, Practice Periodical On Structural Design and Construction. ASCE, February, 2008.
- Gutenberg and Richter (1949). Seismicity of the Earth. Princeton NJ: Princeton University Press.
- Hathout, I. (1994). Treatment of Uncertainty in A Fuzzy Logic Expert System for Damage Assessment of Transmission Structures. A chapter in the reference book "Uncertainty Modelling and Analysis: Theory and Applications". Edited by B.M.Ayyub and M.M. Gupta, Machine Intelligence and Pattern Recognition, Elsevier Science Publishers B.V., North-Holland.

- Harkbess, E.L., Hassanain, M.A., Seismic Damage in NSW, Australia: Construction Insurance, Social, and Economic Consequences, Journal of Performance of Construction Facilities, May 2002.
- Hays, W.E., (1981). Facing Geologic and Hydrologic Hazards –Earth Science Considerations, U.S Geog. Survey Perof. Paper, Paper 1240B, 108p.
- Heck, N. (1936). Earthquake. Princeton NJ: Princeton University Press.
- Hendrik, L.R., Waverman, L. (2001). Telecommunications Infrastructure and Economic Development: A Simultaneous Approach. American Economic Review 91(4):909-923. ISSN 0002-8282 (http://worldcat.org/issn/0002-8282).
- Hiramtsu,K., Sato, Y., Akagi, H., and Tomita, S. (1989). Seismic Response Observation of Building Appendage. In Proceedings of the 9th World Conference on Earthquake Engineering, Tokyo, 2-9 August 1988. Japan Association for Earthquake Disaster Prevention, Tokyo. Vol. 6, pp.237-242.
- Hiroyuki,F., Shunishi,K., Shin,A., Nobuyuki,M., Senna,S., Kyoko, K., Toru,I., Toshihiko, O and Yuzura, H, (2006). National Seismic Hazard Maps of Japan, Japan, Vol. 81, pp. 221-232.
- Horner, R.M.W., El-Haram, M.A. and Munns, A.K. (1997). Building Maintenance Strategy: A New Management Quality Approach. Journal of Quality in Maintenance Engineering 3(4):273-280.
- International Association for Earthquake Engineering (IAEE) Commitee (1992). Earthquake Resistant Regulations: A World List-1992. Revised Edition. Prepared by the International Association for Earthquake Engineering, 1992. Approximately 1100 pages. Distributed by Gakujutsu Bunken Fukyu-Kai (Association for Science Documents Information) Oh-Okayama, 2-12-1, Meguroku, Tokyo, 152, Japan.
- International Association for Earthquake Engineering (IAEE) Committee (1986), Guidelines for Earthquakes Resistant Non-Engineered Construction, Gakujutsu Bunken Fukyu-kai, Japan, 158p.
- International Code Council. (2000). International Building Code 2000. International Code Council, International Conference of Building Officials, Whittier, CA, and others.
- Institution of Engineers Malaysia, (2005). Position Paper on Issues Related to Earthquake, IEM, Malaysia.
- Institution of Engineers Malaysia, March, 2008. Jurutera Bulletin, Malaysia.

International Association of Earthquake Engineering, Proceedings of the Tenth World Conference on Earthquake Engineering, 19-24 July, 1992, Madrid, Spain.

International Business Times, http://www.ibtimes.com/articles/ [cited February 2012]

- Ipsos MORI, (2005). I Just Text To Say I Love You (http://www.ipsosmori.com/content/polls-2000/i-just-text-to-say-i-love-you.ashx).
- Issac, (1997). Ultimate Strength. Report of Committee III.I, International Ships and Offshore Structures Congress.
- ISuara (2012). Internet for Everyone. 1TM News : June 17th. 2012, Kuala Lumpur, Malaysia.
- Japan International Cooperation Agency (JICA). (1992). Bridge Inspection, Maintenance and Rehabilitation Manual, The Study on The Maintenance and Rehabilitation of Bridges in Malaysia.
- Japan Society of Civil Engineers (JSCE). (1995). Preliminary Report on the Great Hanshin Earthquake January 17, 1995. Japan Society of Civil Engineers 1995.
- Japan Road Association (JRA). (1998). Design Specifications of Highway Bridges, Part 1 Common Part, Part II Steel Bridges, Part III Concrete Bridges, Part IV Foundations and Part V Seismic Design,1996 (in Japanese, Part V: English version July 1998).
- Japan Road Association (JRA). (2003). Design Specifications of Highway Bridges, Part 1 Common Part, Part II Steel Bridges, Part III Concrete Bridges, Part IV Foundations and Part V Seismic Design,2002 (in Japanese, Part V: English version June 2003).
- Jennings, C.W. (1994). Fault Activity Map of California and Adjacent Areas. Department of Conservation, Div. Mines and Geology, Sacramento.
- Kanai, K. (1983). Engineering Seismology, University of Tokyo Press, Tokyo.
- Kanazawa,K. and Hirata, K. (2000). Seismic Analysis for Telecommunication Towers Built on the Building. In Proceedings of the 12th World Conference on Earthquake Engineering, Auckland, New Zealand, 30 January-4 February, 2000. New Zealand Society for Earthquake Engineering, Upper Hutt, New Zealand. Paper 0534.
- Kameda, H. and Takagi, H (1981). Seismic Hazard Estimation based on Non-Poisson Earthquake Occurrences. Member Faculty Engineering, Kyoto University., v.XLIII, Pt. 3, July, Kyoto.

- Kawashima, K. (1995). Impact of Hanshin/Awaji Earthquake on Seismic Design Strengthening of Highway Bridges, Report No. TIT/EERG 95-2, Tokyo Institute of Technology. (in English).
- Kawashima, K. (1996). Design Specifications for Highway Bridges, 29th UJNR Joint Panel Meeting, May 1996. Tokyo Institute of Technology. (in English).
- Kehdr,M.A., and McClure,G (1999). Earthquake Amplification Factors for Self-Supporting Telecommunication Towers. Canadian Journal of Civil Engineering 1999; 26(2): pp. 208-215.
- Komoo,I. and Morgana, S.N.,(1999). The Kundasang Landslide Complex, Sabah (extended abstract). Journal of Nepal Geological Society, 20, 230.
- Komoo, I., Salleh, H., Tjia, H.D., Aziz, S., Tongkul, F., Jamaluddin, T.A. and Lim, C.S., (2005). Kundasang Landslide Complex: Mechanism, Socio-Economic Impact and Governance (in Malay).
- Konno, T., and Kimura, E. (1973). Earthquake Effects on Steel Structures Atop Buildings.
 In Proceedings of the 5th World Conference on Earthquake Engineering, Rome, 25-29 July 1973. Ministry of Public Works, Rome. Italy. Vol. 1,pp.184-193.
- Krinitzsky, E.L, Gould, J.P. & Edinger, P.H. (1993). Fundamentals of Earthquake Resistant Construction. New York; John Wiley & Sons, Inc.
- Kramer, W.M. and Bahme C.W. (1992). Fire Officer's Guide To Disaster Control. p.18 85 NJ: A Pennwell Publishing Company.
- Krawnikler, H. and Miranda, E. (2004). Performance-based earthquake engineering. In Earthquake Engineering: From Engineering Seismology to Performance-Based Engineering (Y. Bozorgnia and V.V. Bertero, eds). CRC Press, Boca Raton, Florida.
- Kwiatkowski V.F. (1986), Infrastructure Assets: An Assessment of User Needs and Recommendations for Financial Reporting, Ph.D. Thesis, University of Kentucky.
- Lakshmanan, N., Sattish, K., Muthumani, K., Guru, J., Gopalakrishnan, N., Experimental Investigations on the Seismic Response of a Base-Isolated Reinforced Concrete Frame Model, Journal of Performance of Constructed Facilities, ASCE, 2008.
- Lenert, E (1998). A Communication Theory Perspective on Telecommunications Policy, Journal of Communication, 48 (4):3-23.
- Lim, P.S., (1977). Earth Tremors in Eastern Sabah. Annual Report 1976, Geoglogical Survey Malaysia, National Printing Department, Kuching, Malaysia. p.220-223.

- Lim, P.S., (1985).History of Earthquake Activities in Sabah, 1897-1983. Annual Report 1983, Geog. Survey Malaysia, National Printing Department., Kuching. p 350-357.
- Lim, P.S., (1986). Seismic Activities in Sabah and Their Relationship to Regional Tectonics. Annual Report 1985, Geog. Survey Malaysia, National Printing Department., Kuching. p 465-480.
- Lim, P.S. and Godwin, P. (1992). The Ranau Earthquake Swarm, May-July, 1991, Sabah. Proc 23rd Geoglogical Survey Malaysia, No. 4, p. 163-193.
- Lin, J.L., Tsai, K.C., Miranda, E, Seismic History Analysis of Asymetric Buildings With Soil-Structure Interaction, Journal of Structural Engineering, ASCE, February 2009.
- Livingsteel, (2009). Structural Safety.(http://www.livingsteel.org/structural-safety-10)
- Lee, R. (1987). Building Maintenance Management. London: William Collins Sons.
- Lee, H.Y.H. and Scott, D. (2008). Identification of Main Aspects in The Management of Building Maintenance Operation Process. 'Surveyors Times', Hong Kong Institute of Surveyors 17(6): 37-41.
- Loosemore, M. and Hsin, Y.Y. (2001). Customer-Focused Benchmarking for Facilities Management. Facilities 19(13/14): 464-476.
- Lomnitz. C. (1994). Fundamentals of Earthquake Predictions. New York; John Wiley & Sons.
- Lomnitz. C. (1994). Global Tectonic and Earthquake Risk. New York; Elsevier.
- Luin, C.C (2008). Seismic Effects: A Threat to Local Structures? Jurutera, Institution of Engineers Malaysia Volume 3, p.6.
- Magued, M.H., Burneau, M. and Dryburgh, R.B.(1989). Evolution of Design Standards and Recorded Failures of Guyed Towers in Canada. Canadian Journal, Civil Engineering. 16, 725-732.
- Mainichi Shimbun . Devastation of March 2011 Fukushima Earthquake (2011, March 12). pg 1-3.
- Malhotra, V.M., and Carino, N.J. (Eds.), (1991). CRC Handbook on Non-Destructive Testing of Concrete. CRC Press, Florida, United States of America.
- Mikus, J. (1994). Seismic Analysis of Self-Supporting Telecommunication Towers. M. Eng. Project Report G94-10, Department of Civil Engineering and Applied Mechanics, McGill University, Montreal, Canada.

- Murotsu,Y., Nakayasu,H., Mori,K. and Kase, S. (1978). New Method of Relating Safety Factor to Failure Probablity in Structural Design. Advanced in Reliability and Stress Analysis, ASMR Annual Meeting, San Fransisco, CA.
- McClure, G., and Guevara, E. I. (1994). "Seismic Behavior of Tall Guyed Telecommunication Towers." Proc. IASS-ASCE Int. Symp. 1994, ASCE Structures Congress XII, ASCE, Reston, Va., 259-268.
- McClure, G. (1999). "Earthquake-resistant design of towers." Proc., Meeting of IASS Working Group 4 on Masts and Towers.
- McGuire, R.K.(1993). Practise of Earthquake Hazard Assessment. International Association of Seismology and Physics of the Earth's Interior, 284pp.
- Moehle, J.P. and Eberhard, M.O. (2000). Earthquake Damage to Bridges, Bridge Engineering Handbook. CRC Press, Boca Raton, Florida.
- Mogi,K., (1992). Earthquake Prediction Research In Japan, In Earthquake Prediction Proc. Int. Sch. Solid Earth Geophys., Erice, Italy (II Cigno, Galileo Galilei, Rome).
- Mohamad, M. (2010). A Doctor in the House –The Memoirs of Tun Dr Mahathir Mohamad. MPH, Petaling Jaya, Selangor.
- National Institute of Standards and Technology (NIST) (1995). The January 1995 Hyogoken-Nanbu (Kobe) earthquake performance: performance of structures, lifelines, and fire protection systems. NIST Special Publication 901, United States National Institute of Standards and Technology, Gaithersburg, MD, July 1996.
- "Nine-One-One and Communications Centres". (1970). A.I.A Special Interest Bulletin No 322, July.
- NRC/IRC National Research Council of Canada / Institute of Research in Construction (2005). National Building Code of Canada 2005, Ottawa, ON, Canada.

New Straits Times. Make Maintenance Culture a Way of Life. (2009, June 19) p.18.

- Online News: For Many Home Broadband Users, The Internet Is A Primary News Source (http://www.pewinternet.org/pdfs/PIP_News.and.Broadband.pdf). Pew Internet project, 2006-03-22.
- Organisation for Economic Cooperation and Development (OECD). (1983) Bridge Rehabilitation and Strengthening, Road Transport Research, Paris.
- Organisation for Economic Cooperation and Development (OECD). (1989) Durability of Road Bridges, Road Transport Research, Paris.

- Panakkat, A., Adeli, H., Recent Efforts in Earthquake Prediction (1990-2007), Natural Hazard Review, ASCE, May 2008.
- Pearson-Kirk, D., (2000). Bridge Management, Thomas Telford Publishing Ltd., London.
- PEMANDU, (2010). The Government Transformation Programme (GTP) Annual Report 2010. July 18, 2011.
- Public Works Department Malaysia, (2008). Seismic Design Guidelines for Concrete Buildings in Malaysia. (JKR20601-0184-09).
- Pierre, J.R. (1995). Damage Caused The Hanshin-Awaji (Kobe-Japan) Earthquake to Electrical and Telecommunication Networks Its Impact on the Implementation of Emergency Measures. Report RE-GEN-95-40, Hydro-Quebec, Montreal, Quebec.
- Queiroz, C., Hass, R and Cai, Y, (1994), National Economic Development and Prosperity Related To Paved Road Infrastructure, Transportation Research Record 1455, Transportation Research Board, National Research Council, Washington D.C. pp 147-152.
- Raj, J.K. (1996). Seismicity of East Malaysia. [Abstract], Annual Conference 1996, Geological Society Malaysia, Kota Kinabalu, 8&9th. June, 1996.
- Ratay, R, T. (2005). Structural Condition Assessment, USA: John Wiley & Sons. P.4.1 4.30.
- Rosaidi, M. (2001). Earthquake Monitoring in Malaysia. Seismic Risk Seminar, September 25th. Malays, Malaysia.
- Ryall, M.J., Parke, G.A.R., Harding, J.E., (2000) Bridge Management Four, Thomas Telford, London.
- Samaan, M. (2003). The Effect of Income Inequility on Mobile Phone Penetration. Boston University Honors thesis.
 - (http://dissertations.bc.edu/cgi/viewcontent?article=1016&context=ashonors)
- Sato,Y., Fuse,T., and Akagi, H. (1984). Building Appendage Seismic Design Forced Based on Observed Floor Response. In Proceedings of the 8th World Conference on Earthquake Engineering, San Fransisco, California, 21-28 July 1984. Prentice Hall Inc., Englewood Cliffs, N.J. pp. 1167-1174.
- Savage, R.J., and Hewlitt, P.C., (1977) Structural Integrity: A New NDT Method, British Society for Stress Management and Institution of Civil Engineers Joint Conference, New Castle, United Kingdom.

- Savidis, S.A., (1989). Earthquake Resistant Construction and Design. Proceedings of the International Conference on Earthquake Resistant Construction and Design, Berlin, 13-16 June 1989.
- Sackmann, V. (1996). Prediction of Natural Frequencies and Mode Shapes of Self-Supporting Lattice Telecommunication Tower. M. Eng. Project, 1996. Department of Civil Engineering and Applied Mechanics, McGill University, Montreal, Canada.
- Seeley, I.H. (1976). Building Maintenance, Macmillan, London.
- Standard Operation Procedure for Managing Earthquake Disaster, December, (2007). National Security Council, Malaysian Prime Minister's Office, Kuala Lumpur.
- Schiff, S.D. (1989). Seismic Design Studies of Low-Rise Steel Frames. PhD Thesis. Department of Civil Engineering, University of Illinois at Urbana-Champaign.
- Skjong, R. (1985). Reliability Based Optimization of Inspection Strategies. In Proc. ICOSSAR 85 Vol. III, Kobe, Japan, pp 614-618.
- Smith, B. W. (2007). Communication Structures, Thomas Telford Publishing Ltd, London.
- Sorensen, J.D. & Engelund, S. (1999) Reliability-Based Optimal Planning of Maintenance of Concrete Structures. ASCE, Case Studies in Optimal Design and Maintenance Planning of Civil Infrastructure Systems, D.Frangopol (ed.), pp 224-235.
- Spedding, A. (1987). Building Maintenance Economics and Management, E. & F. N. Spon. Volume 15, pp 111-129.
- Spencer, W. (1994). Intricate Web of Sensors Enablers Refined Quake Predictions, The New York Times, April 16, 1994.
- Stratful R.F. (1973). Half Cell Potentials and the Corrosion of Steel in Concrete, Highwway Research Record. 433 pp12-19.
- Stratful R.F., Jurkovich, W.J. and Spellman, D.L., (1974). Corrosion Testing of Bridge Decks. Transport Research Record 539.
- Steinbrugge, K.V., (1982). Earthquakes, Volcanoes and Tsunamis an Anatomy of Hazards. New York: Skandia America Group.
- Tee, A.B and Bowman, M.D. (1991). Bridge Condition Assessment Using Fuzzy Weighted Averages. Civil Engineering Systems V8 n1, March, pp 49-57.

Telecom Industry Revenue to Reach \$1.2 Trillion in 2006

(http://www.voip-magazine.com/content/view/1197/), VoIP Magazine, 2005.

- Telekom Malaysia Berhad (2008). Report for Menara Alor Setar, Civil and Structural Building Audit. GHD Perunding Sdn Bhd. Kuala Lumpur, Malaysia.
- Telekom Malaysia Berhad (2009). Facility Condition Appraisal Report for Menara Kuala Lumpur. Global Facilities Management Sdn. Bhd. Kuala Lumpur, Malaysia.
- Telekom Malaysia Berhad (2009). Integrated Facility Management Services for Menara Kuala Lumpur. AWC Facility Solutions Berhad. Kuala Lumpur, Malaysia.
- Telekom Malaysia Berhad (2009). Site Audit report for Building Facility Condition, Menara Kuala Lumpur. AWC Facility Solutions Berhad. Kuala Lumpur, Malaysia.
- Telekom Malaysia Berhad (2011). Asia Submarine Cable Express Project. 1TM News : January 17th. 2011, Kuala Lumpur, Malaysia.
- Telekom Malaysia Berhad (2011). Highspeed Broadband Landscape for Malaysia. 1TM News : 23rd. May 2011, Kuala Lumpur, Malaysia.
- The Cincinnati Enquirer, April 28, 1990.
- The Edge. Zamzairani : Management at Work. (2011, October 3rd). p.1.
- The Street, Drainage and Building Act, 1984. International Law Book Services, Malaysia.
- TIA Standards, Structural Standard for Antenna Supporting Structures and Antennas, Telecommunication Industry Association, TIA-222-G, (Revision of TIA-22-F) August (2005) (Revision of TIA-222-F) April (2007).
- Tjia, H.D. (1978). Structural Geology of Peninsular Malaysia. Proc. 3rd. Reg. Conference Geology & Mining. Res. South East Asia, 14-18 November, 1978, Bangkok, Thailand. P. 673-682.
- Tjia, H.D. (1983). Quaternary Tectonics of Sabah and Sarawak, East Malaysia. Sains Malaysiana, Vol. 12(2), p. 191-215..
- Telekom Malaysia Asset Management System, TM TeAMS Web Site, http://intra.tm.teams/ [cited January 2009].
- Unjoh, S. (2002). Design Specifications for Highway Bridges, 34th. UJNR Joint Panel Meeting, May 2002.
- Vassie, P.R. (1980). A Survey of Site Tests for Assessment of Corrosion in Reinforced Concrete. TRRL Laboratory Report 953.
- Web Site, http://earthquake.usgs.gov/ [cited June 2009].
- Wakabayashi, M., (1986). Design of Earthquake Resistant Buildings, McGraw-Hill, Paris.

- Wijanto, S. and Andriono T. (1999). Evaluation of the Seismic Performance of A 1907's L-shaped Three Storey Unreinforced Masonry Building in Indonesia, Proceeding of NZSEE Conference, Rotorua, New Zealand. pp. 103-110.
- Wijanto, S., Andriono T.and Satyarno. I. (2000). Strengthening of A 1970's L-Shaped Three Storey Unreinforced Masonry Building in Indonesia. Proceeding 12th WCEE, Auckland 2000. Paper no 1368.
- Wijanto, S., Wreksoatmodjo, S. Hardy, L and and Pendellah, A. (2001). Bengkhulu Earthquake 4th June 2001, Journal of Civil Engineering Department, Trisakti University, Vol. 01, No1, Jakarta, Indonesia. 18p.
- Will Earthquake Reach Malaysia (September 2009), http://forum.lowyat.net/topic/1805850 [cited November 2011].
- Wood,H.O. and Neumann, Fr. (1931) Modified Mercalli Intensity Scale of 1931, Bull. Seis. Soc.Am., 21, 277-283.
- Woodward, R.J. and Loe, J.A. (1981) The Inspection and Maintenance of Post-tensioned Concrete Bridges. International Conference on Gestion des Ouvrages d'Art. Bruxelles- Paris, Vol 1, pp 265-270.
- Wordsworth. P. (2001) Lees's Building Maintenance Management. 4th. Edition, Blackwell Science.
- World Housing Encyclopedia Report. (2004). Country: Indonesia. Earthquake Engineering Research Institute, the Institute Association of Earthquake Engineering.

World Telecommunication Development Report 2003

(http://www.itu.int/ITU-D/ict/publications/wtdr_03/index.html),International Telecommunication Union, 2003.