

LANDSLIDE RISK ASSESSMENT ANALYSIS USING MONTE CARLO
SIMULATION

MOHD KAMAL IKHWAN BIN ZULKEFLY

A project report is submitted in fulfillment of the requirements for the award for the
degree of Master of Civil Engineering (Geotechnic)

Faculty of Civil Engineering
Universiti Teknologi Malaysia

AUGUST 2012

*Dedicated to
my beloved family, friends, and lectures
for your
Love...
Patience...
Encouragement...
and Help...*

ACKNOWLEDGEMENT

First of all, I would like to express my highest gratitude to Allah S.W.T for His guidance and blessings for me to complete the project report. In preparing this project report, I was in contact with many people, researchers, and academicians. They have contributed towards my understanding and thoughts. In particular, I wish to express my sincere appreciation to my main project report supervisor, Dr. Hisham bin Mohamad, for encouragement, guidance, critics and friendship. I am also very thankful to my co-supervisors Prof Aminaton Marto for her guidance, advices and motivation. Without their continued support and interest, this project report would not have been the same as presented here.

My sincere appreciation also extends to all my colleagues and others who have provided assistance at various occasions. Their views and tips are useful indeed. Unfortunately, it is not possible to list all of them in this limited space. Last but not least, thank you to my beloved parents and family for their encouragement and moral support. And also thanks to all of my friends who were involved directly or indirectly in completing this project report.

ABSTRAK

Kebanyakan analisis kecerunan menggunakan pendekatan metod tradisional iaitu menggunakan reka bentuk nilai purata parameter. Oleh itu, kaedah ini menghasilkan anggaran nilai tunggal untuk menganggarkan kestabilan cerun. Pendekatan faktor keselamatan tidak boleh digunakan untuk mengira kebarangkalian kegagalan atau tahap risiko yang berkaitan dengan situasi reka bentuk disebabkan keadaan tanah yang tidak pasti. Ketepatan analisis kestabilan tahap kecerunan boleh dikira menggunakan Analisis Kebarangkalian. Metod analitikal menggunakan maklumat tentang taburan kebarangkalian ciri-ciri kecerunan untuk menentukan analisis output taburan kebarangkalian. Objektif kajian ini adalah untuk menentukan kebarangkalian kegagalan kecerunan berdasarkan bidang dan kesan kajian parameter statik dalam kegagalan hasil pengiraan. Simulasi Monte Carlo adalah berguna untuk memodelkan satu sifat yang tidak boleh disampel atau diukur secara langsung, tetapi boleh diungkapkan sebagai fungsi matematik yang boleh dijadikan contoh. Faktor keselamatan juga sesuai dengan keadaan ini. Oleh yang demikian, metodologi kajian ini telah dipilih berdasarkan jurnal dan kertas kerja yang lepas. Data diperolehi daripada kes kajian kecerunan di Majlis Peperiksaan Malaysia, Selayang, Selangor, berdasarkan laporan makmal dan disimulasikan menggunakan perisian SLOPE/W. Hasil keputusan metod penentuan ialah 1.184, manakala anggaran kegagalan cerun ialah 9.49%. Ini adalah disebabkan ketidakpastian bahan yang terkandung dalam tanah. Kajian ini juga mendapati bahawa, lebih tinggi ketidakpastian sifat tanah, peluang yang lebih tinggi untuk cerun akan gagal.

ABSTRACT

Most slope analyses are based on traditional method approaches which adopt average valued design parameters. Consequently, the method yields a single valued estimate of slope stability. The factor of safety approach cannot quantify the probabilities of the failure or level risk associated with a particular design situation because of the uncertainty that exists in the soil. The analysis of slope stability can be more accurately evaluated through the use of probabilistic analysis. The analytical method uses the information about the probability distribution of the slope characteristics to determine the probability distribution of the output of the analysis. The objective of the research is to determine the probabilities of slope failure based on a given field study and study the effect of statistical parameters in the result calculation of failure. Monte Carlo simulation is useful for modelling an attribute that cannot be sampled or measured directly, but can be expressed as a mathematical function of properties that can be sampled. Factor of safety also fits this situation. The methodology of this research is based on previous relevant papers and journals. The data is collected from the case study of the slope at Majlis Peperiksaan Malaysia, Selayang, Selangor based on the lab report and simulated with SLOPE/W software program. The result of the deterministic method is 1.184, while for the probability of the slope failure is equal to 9.49%. This is due to low uncertainty in the soil properties. The study also found that the higher the uncertainty of the soil properties, the higher the chance the slope will fail.

TABLE OF CONTENT

CHAPTER	TITLE	PAGE
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDEMENTS	iv
	ABSTRACT	v
	ABSTRAK	vii
	TABLE OF CONTENT	vii
	LIST OF TABLES	x
	LIST OF FIGURES	xi
	LIST OF ABBREVIATIONS	
1	INTRODUCTION	
	1.1 Background of Study	1
	1.2 Problem Statement	3
	1.3 Objective of Study	4
	1.4 Scope of Study	4
	1.5 Significance of Study	4
2	LITERATURE REVIEW	
	2.1 Introduction	6
	2.2 Mechanism and Type of Landslide	7
	2.3 Landslide Behaviour	9

2.4	Risk Assessment Analysis	10
2.4.1	Risk	10
2.4.2	Risk Assessment	10
2.4.3	Risk Estimation	13
2.4.4	Landslide Risk Assessment	16
2.5	Conventional Method	18
2.5.1	Type of Conventional Method	19
2.5.2	Limitation Conventional Method	20
2.6	Probability	22
2.7	Monte Carlo Simulation	22
2.7.1	History of Monte Carlo Simulation	23
2.7.2	The Concept	24
2.7.3	The Application in Geotechnical Engineering	25
3	METHODOLOGY	
3.1	Introduction	28
3.2	General Methodology	29
3.3	The Research Methodology	31
3.4	The Case Study	33
3.5	Determination of the Parameter of the Soil	35
3.5.1	Determination of the Soil Properties	36
	3.5.1.1 Cohesion, Angle of Friction and Unit Weight of the Soil	36
3.5.2	Determination of Statistical Parameter of The Soil	37
3.6	Deterministic Analysis	37
3.7	Probabilistic Analysis	38
3.8	Variable of the Analysis	38
3.9	SLOPE/W	38
3.9.1	Analysis Setting	39
3.9.2	Input Setting	41
3.9.3	The Output	42

4	RESULT AND ANALYSIS	
4.1	Introduction	44
4.2	Soil Parameter	45
4.2.1	Soil Properties	45
4.2.2	Statistical Parameter of The Soil	47
4.3	Soil Layout	48
4.4	Deterministic Analysis	50
4.5	Probability Analysis	51
4.5.1	Variable Study	53
5	CONCLUSION AND RECOMMENDATIONS	
5.1	Problem	58
5.2	Recommendation	59
5.3	Conclusion	60
	REFERENCES	61
	Appendix A-B	64-75

LIST OF TABLES

TABLES NO	TITLE	PAGE
2.1	List of Method and Differences	19
3.1	Type of Test and Depth of Sample	37
4.1	Representative Values of e, MC	45
4.2	Properties of the Soil	46
4.3	Statistical Parameter of the soil	47
4.4	Modification from Original Standard Deviation	53
4.5	Probability of Failure for Each Standard Deviation	53

LIST OF FIGURES

FIGURE NO	TITLE	PAGE
1.1	Landslide Event at Bukit Antarabangsa Area	2
1.2	Landslide Event at Hidayah Madrasah Al-Taqwa in FELCRA Semungkis	2
2.1	Landslide Types	8
2.2	The Flow Chart of the of Stages in the Risk Assessment Process	12
2.3	Example of Risk Matrix	15
2.4	Uncertainty of The Soil Properties	21
2.5	Innate Variability and Spatial Average	21
2.6	The Concept of Monte Carlo Simulation	24
3.1	General Monte Carlo Simulation Approach	30
3.2	Research Flow Chart	32
3.3	Location of the Study Area	33
3.4	Aerial Mapping of the Study Area	33
3.5	Slope Area and Location	34
3.6	Slope Cut Geometry E-E'	35
3.7	The Soil Profile and location of Each Borehole.	36

3.8	The Setting Tab	39
3.9	The Slip Surface Tab	40
3.10	The FOS Distribution Tab	40
3.11	KeyIn Materials Window	41
3.12	Parameter Details Window	41
3.13	Probability Density Function of the Factor of Safety	42
3.14	Probability Distribution Function of the Factor of Safety	42
3.15	The Probability of Failure	43
4.1	The Soil Profile	49
4.2	The Soil Profile with Grid and Radius Option	50
4.3	Failure Critical Slip Surface on the Case Study	51
4.4	Probability Density Function (Case Study)	52
4.5	Probability Distribution Function (Case Study)	52
4.6	Probability Density Function (OSD -0.5)	54
4.7	Probability Distribution Function (OSD -0.5)	54
4.8	Probability Density Function (OSD + 1)	55
4.9	Probability Distribution Function (OSD + 1)	55
4.10	Probability Density Function (OSD + 2)	56
4.11	Probability Distribution Function (OSD + 2)	56
4.12	Effect of Variation to the Probability of Failure	57
5.1	Uncertainty in Soil Property Estimates	59

LIST OF APPENDIX

APPENDIX	TITLE	PAGE
A	Boreholes Log	64
B	Laboratory Test Results	75

CHAPTER 1

INTRODUCTION

1.1 Background of the Study

The tragic events of Bukit Antarabangsa in 2008 are one of landslide disaster that happened in Malaysia four years ago. Unfortunately, there was another tragic incident that happened last year at Hidayah Madrasah Al-Taqwa in FELCRA Semungkis. On 21st May 2011, the landslide disaster happened near the Madrasah that killed 16 people, where 15 of the victims are children. The tragic incident had prompted the government and public awareness about the importance of the stability of slope design in reducing the risk of failure of which result landslide event. Hence, it is importance that the current practices of slope stability need to be updated and revised to reduce the occurrence of the tragedy.

Nowadays, most of the slope stability designed based on the traditional slope stability method. For this traditional slope stability method, it still used single values estimate for each variables in the slope stability analysis. The shear strength, slope geometry, external load and pore water pressure are assigned as the parameter in the design. The result of the analysis would be the slope stand still or collapse, which is the factor of safety.



Figure 1.1: Landslide Event at Bukit Antarabangsa Area



Figure 1.2: Landslide Event at Hidayah Madrasah Al-Taqwa in FELCRA Semungkis

The ability of geotechnical engineer to accurate model slope performance is compromised by a variety factor. These may be broadly classified as theoretical and practical consideration. The net result of this consideration is the exact behavior of slope cannot be accurately predicted. Hence, the engineer resorts the factor of safety to reduce a risk of landslide. However, the factor of safety cannot quantify the probabilities of failure, level of risk and must be associated with particular design situation.

1.2 Problems of statement

Most of the slope stabilization is based on conventional or deterministic approach when the set of single value design are adopted and set of single value of factor of safety is obtained. However, the inherent variability of the characteristic that affected slope stability is a probabilistic rather than a deterministic situation. Due to the presence of uncertainty that not included in the normal factor of safety calculation, the factor of safety could not quantify the probability of failure or level of risk that associated with particular design situation.

The analysis of the slope stability can be evaluated accurately through the use of the probabilistic analysis. In addition, the analytical methods use the information about the probability distribution of the slope characteristic to determine the probability distribution of the output of the analysis. Knowledge of the probability distribution results allowed the engineer to assess the probability of failure slope stability. Due to that, it is needed to facilitate the adoption of probability analysis in slope design practice among the engineers.

1.3 Objectives of the study

The objectives of the study are:-

- To determine the probabilities of slope failure based on a field study given
- To study the effect of statistical parameters from the failure calculation

1.4 Scope of the study

This research is focuses on:-

- The SLOPE/W that will use as simulation program.
- The Majlis Peperiksaan Malaysia (MPM) that will be used for the field study.
- The analysis will focus on the slope cut.
- The MSE wall and the presence of the boulder excluded from analysis.
- The uncertainty that included in the shear strength of the soil which are Cohesion, angle of friction and unit weight of the soil.
- The deterministic method that will focus only on Morgenstern-Price method.

1.5 Significance of the study

The limitation of this research are the excluding presence of MSE wall and boulder. Besides that, the deterministic method only focus on Morgenstern-Price method. MSE wall and boulder exclude from the analysis due to lack of skill and knowledge in inserting the presence MSE wall and boulder on the SLOPE/W program and also because of the short duration period of research to learn it.

The deterministic method only focuses on Morgenstern-Price method because the method only suited to both circular and non-circular problems. Besides that, it is theoretical and more accurate than Bishop's method. The method is similar to Spencer's method except it allows for a variable interslice force angle on every slice and instead of theoretically, but more mathematically correct.

From the research, we are able to understand on how risk assessment on probabilistic approach through Monte Carlo has done. The probability of failure will help the engineer to design accurately the stability of the slope. With the change of the statistical parameter of the soil, the uncertainty effect can be identified on the stabilization of the slope thus can help the engineer to select the best method in stabilizing the slope.

REFERENCES

- Alonso, E.E.(1976). *Risk Analysis of Slopes and Its Application to Slopes in Canadian Sensitive Clays*. Geotechnique. Vol.26 (3): 453-472.
- Australian Geomechanics Society (2000). Landslide risk management concepts and guidelines. Australian Geomechanics 35, 49—52.
- Bond A. and Harris A. (2008), *Decoding Eurocode 7*, Taylor & Francis Group
- Braja M. Das (1995). *Principles of Geotechnical Engineering (4th Edition)*, PWS Publisher Co.
- Bujang B.K. Huat, Gue, S. S., Faisal, A. (2004) *Tropical Residual Soils Engineering* A.A. Balkema Publishers
- Cruden, D. (1991). *A simple definition of a landslide*. Bulletin of the International Association of Engineering Geology 43, 27—29.
- Cruden, D. M. and Varnes, D. J. (1996). *Landslide types and processes*. In *Landslides: Investigation and Mitigation* (eds A. K. Turner and R. L. Schuster). Transportation Research Board, Special Report 247, National Research Council, National Academy Press, Washington DC, 36—75.
- Davidović N., Prolović, V., Stojić, D., (2010), *Modeling Of Soil Parameters Spatial Uncertainty By Geostatistics*, University of Niš, Faculty of Civil Engineering and Architecture, Serbia
- Department of the Environment (DoE) (1995). *A Guide to Risk Assessment and Risk Management for Environmental Protection*. HMSO, London.
- DGS Geotechnic (2011), *Soil Investigation Work For Slope Stability Study At Majlis Peperiksaan Malaysia*, Majlis Peperiksaan Malaysia Report
- Frank, R., Bauduin, C., Driscoll, R., Kavvadas, M., Krebs Ovesen,, N. Orr, T. And Schuppener, B., (2009) *Designers' Guide To Eurocode 7:Geotechnical Design*, Thomas Telford Limited

- Fredlund, D.G. (1984) *Analytical Methods For Slope Stability Analysis*, 4th, international symposium on landslide
- Gordon A. F. and Griffiths D. V. (2008) *Risk Assessment in Geotechnical Engineering*, John Wiley and Sons, inc
- Hoffman, P. (1999), *The Man who Loved Only Numbers*, Fourth Estate Limited
- Honjo Y. (2008) *Monte Carlo Simulation in Reliability Analysis*, Reliability-Based Design in Geotechnical Engineering, Taylor & Francis Group
- Hutchinson, S. & Bandalos, D. (1997). A guide to Monte Carlo Simulation Research for Applied Researchers. *Journal of Vocational Education Research*
- Janbu, N. (1973). *Slope Stability Computations*. Embankment Dam Engineering, Casagrande Volume, pp. 47-86.
- Jennifer L. P. (1999) *Probability Analysis of Slope Stability*, West Virginia University
- Lee, E. M. and Jones, D. K. C. (2004) *Landslide Risk Assessment*, Thomas Telford Publishing
- Harr, M.E. (1987). *Reliability Based Design in Civil Engineering*. McGraw-Hill Inc, USA.
- Liu C.N (2008) *Landslide Hazard Mapping Using Monte Carlo Simulation- A Case Study In Taiwan*, Civil Engineering Department, National Chi Nan University, No.1, University Road, Nantou, Taiwan 545, China
- Metropolis, N. (1987) *The Beginning Of The Monte Carlo Method Los Alamos Science Special Issue*
- Metropolis, N. and Ulam, S. (1949). The Monte Carlo method. *Journal of the American Statistical Association* 44:335-341.
- Morgenstern, N. R. (1991). *Limitations of stability analysis in geotechnical practice*. *Geotechnica*, 61, 5—19.
- Ng, K. S. (2005) *Reliability Analysis on The stability of Slope*, Universiti Teknologi Malaysia

Phoon K.K and. Kulhawy F. H (1999) *Characterization of geotechnical variability*,
NRC Canda

Stability Modeling with SLOPE/W 2007 Version (2008) GEO-SLOPE International
Ltd

Tobutt D.C. (1981) *Monte Carlo Simulation methods for slope stability*, Elsevier Ltd.

Yu, W. , Zijun, C. , Siu-Kui, A. (2010) ,*Efficient Monte Carlo Simulation Of
Parameter Sensitivity In Probabilistic Slope Stability Analysis*, Department
Of Building And Construction, City University Of Hong Kong, Tat Chee
Avenue, Kowloon, Hong Kong

Zalina M.D, Maizah H. A., Robiah A., Shariffah S.S.S., Fadhilah Y., Ismail M.,
(2005), *Statistic For Sciences and Engineering*.