

EXTENT OF TRAFFIC SHOCKWAVE PROPAGATION INDUCED BY
MIDBLOCK U-TURN FACILITIES

NURUL FARHANA BINTI JAILANI

UNIVERSITI TEKNOLOGI MALAYSIA

EXTENT OF TRAFFIC SHOCKWAVE PROPAGATION INDUCED BY
MIDBLOCK U-TURN FACILITIES

NURUL FARHANA BINTI JAILANI

A project report submitted in partial fulfilment of the
requirements for the award of the degree of
Master of Engineering (Civil – Transportation and Highway)

Faculty of Civil Engineering
Universiti Teknologi Malaysia

JANUARY 2013

To my beloved father, mother and family

ACKNOWLEDGMENTS

Firsts and foremost, I am grateful to Allah SWT for the knowledge, guidance, patience and strength He bestowed for me in the fulfilment of my thesis.

I would like to express my warmest appreciation to my honourable project supervisor, Assoc. Prof. Dr. Johnnie Ben-Edigbe to his guidance, motivation, encouragement, critics and valuable advice throughout every step of my study. Without his continued support, this thesis would not have been the same presented here.

My special thanks to Pn. Raha binti Abd Rahman for her advice, critics and guidance for me in finishing this project. I am very grateful to all lecturers who had given me knowledge and guidance during my study, FKA staff members for their help and assistance.

Million thanks for MAP classmates and best friends who showed their concern and support all the way. My heartfelt thanks to my family members who have love me unconditionally, stood by me all this while and support me especially mom and dad.

ABSTRACT

The aim of study is to determine the extent of traffic shockwave propagations induced by midblock U-turn facilities. Based on the hypothesis that shockwave would result from U-turning at midblock facility, 24 hour data were collected which is volume and speed at two sites in FT005 Skudai – Pontian highway, Malaysia. The two sites are denoted as site A and site B. The study of highway capacity is the most important part in the identification of road performance. Any change made at the roadway for example installing a midblock U-turn facilities would result in highway capacity loss. There are mainly three causes of capacity loss which are due to the road condition itself, due to ambient and due to the traffic operation. When capacity loss happen, it would lead to traffic shockwaves. Traffic shockwave is a motion of propagation of a change in density and flow. When shockwaves happen, it would lead to reduction of roadway level of service. Speed, flow and density for with and without midblock U-turn facility are estimated using Greenshields Model. All data was collected using Automatic Traffic Counters (ATC). Shockwave velocity propagation with and without midblock U-turn facility were estimated by determining and comparing the capacity and density of roadway. Direct empirical capacity estimation methods are use to estimate capacity of roadway. Results shows that there is significant shockwave velocity propagation happen on the exit of the U-turn midblock facility which is about -35.1 km/hr. While a minor shockwave happens at the approaching of the U-turn. This study concluded that midblock U-turn facilities will induce traffic shockwave that may lead to road accidents.

ABSTRAK

Tujuan kajian adalah untuk menentukan tahap gelombang kejutan trafik berlaku disebabkan oleh blok kemudahan pusingan-U. Berdasarkan pada hipotesis bahawa gelombang kejutan trafik akan berlaku disebabkan blok kemudahan pusingan-U, 24 jam data telah dikumpul iaitu isipadu dan kelajuan pada dua tapak di FT005 lebuh raya Skudai – Pontian, Malaysia. Tapak ditandakan sebagai tapak A dan tapak B. Kajian kapasiti lebuh raya adalah bahagian yang paling penting dalam pengenalpastian prestasi jalan raya. Sebarang perubahan yang dibuat di jalan, contohnya memasang satu blok kemudahan pusingan-U, akan mengakibatkan kehilangan kapasiti. Terdapat tiga punca kehilangan kapasiti iaitu disebabkan oleh keadaan jalan raya itu sendiri, persekitaran dan operasi trafik. Apabila kehilangan kapasiti berlaku, ia akan membawa kepada gelombang kejutan trafik. Gelombang kejutan trafik adalah perubahan dalam ketumpatan dan aliran. Apabila gelombang kejutan trafik berlaku, ia akan membawa kepada pengurangan tahap perkhidmatan jalan. Kelajuan, aliran, dan ketumpatan untuk dengan adanya kemudahan pusingan-U dan tanpanya dianggarkan menggunakan Model Greenshields. Semua data telah diambil menggunakan cerapan traffic automatik (ATC).

Halaju gelombang kejutan trafik dengan dan tanpa kemudahan pusingan-U dianggarkan dengan menentukan dan membandingkan kapasiti dan ketumpatan jalan. Empirikal langsung adalah kaedah penganggaran kapasiti yang digunakan untuk menganggar kapasiti jalan. Keputusan menunjukkan bahawa terdapat halaju gelombang kejutan trafik yang ketara berlaku pada laluan keluar kemudahan pusingan-U iaitu -35.1km/jam. Manakala, kelajuan gelombang kejutan trafik yang kecil berlaku di laluan menghampiri kemudahan pusingan-U. Kesimpulannya, blok kemudahan pusingan-U akan mendorong gelombang kejutan trafik yang membawa kepada kemalangan jalan raya.

TABLE OF CONTENTS

CHAPTER	TITLE	PAGE
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDGEMENTS	iv
	ABSTRACT	v
	ABSTRAK	vi
	TABLE OF CONTENTS	vii
	LIST OF TABLES	x
	LIST OF FIGURES	xi
	LIST OF SYMBOLS/ ABBREVIATIONS	xiii
	LIST OF APPENDICES	xv
1	INTRODUCTION	1
	1.1 Background of Study	1
	1.2 Problem of Statement	3
	1.3 Aim and Objectives	4
	1.4 Scope of Study	4

1.5	Significance of Study	5
2	LITERATURE REVIEW	6
2.1	Introduction	6
2.2	Traffic and Highway in Malaysia	6

2.3	Highway facilities	8
2.3.1	Uninterrupted Flow Facilities	8
2.3.2	Interrupted Flow Facilities	9
2.4	U-turn Facilities	10
2.5	Traffic Streams Parameters	14
2.5.1	Volume and Flow	14
2.5.2	Speed	16
2.5.3	Density	19
2.5.4	Relationship of Flow, Speed and Density	20
2.6	Passenger Car Unit	21
2.7	Greenshields Model	22
2.8	Highway Capacity	26
2.9	Traffic Shockwave	30
2.9.1	Definition of Traffic Shockwaves	30
2.9.2	Types of Traffic Shockwaves	31
2.9.3	Velocity of Shockwaves	32
3	METHODOLOGY	36
3.1	Introduction	36
3.2	Data Requirement	38
3.3	Site Selection	38
3.4	Setup of Impact Study	39

3.5	Data Collection	43
3.5.1	Data Collection Equipment	43
3.5.2	Data Collection Time	44
3.5.3	Data Extraction	45
3.6	Study Hypothesis	45
3.7	Data Analysis	46
4	FINDINGS AND DISCUSSIONS	49
4.1	Introduction	49

4.2	Site Characteristics	49
4.3	Traffic Proportion	50
4.4	Off Peak Hour and Peak Hour	51
4.5	Selected ATC's for Analysis	51
4.6	Mean Speed of Traffic Data	52
4.7	Flow, Speed and Density of Traffic	54
4.8	Speed – Density Relationship	59
4.9	Flow – Density Relationship and Capacity of Roadway	63
4.10	Shockwave Propagations	68
5	CONCLUSION AND RECOMMENDATION	72
5.1	Conclusion	72
5.2	Recommendations	73
	REFERENCES	74
	APPENDIX	77

LIST OF TABLES

TABLE NO.	TITLE	PAGE
2.1	Distances between U-turns	13
2.2	Conversion Factors to pcu	21
3.1	Vehicle Classification	45
4.1	Speed statistics for ATC 02 on the first 5 minutes interval data	53
4.2	Traffic volume data for ATC 01 (Outer lane section Site A)	54
4.3	Traffic volume data for ATC 02 (Inner lane section Site A)	55
4.4	Traffic volume data for ATC 03 (Inner lane section Site B)	55
4.5	Traffic volume data for ATC 04 (Outer lane section Site B)	56
4.6	Conversion Factors to pcu	56
4.7	Flow, Speed, and Density data for ATC 01 (Site A)	57
4.8	Flow, Speed, and Density data for ATC 02 (Site A)	57
4.9	Flow, Speed, and Density data for ATC 03 (Site B)	58
4.10	Flow, Speed, and Density data for ATC 04 (Site B)	58

4.11	Traffic data of flows and densities	68
4.12	Traffic Shockwave Velocity	70

LIST OF FIGURES

FIGURE NO.	TITLE	PAGE
1.1	Midblock U-turn facilities at FT005	5
2.1	Conflict points before and after left-turn prohibition (TRB, 2003)	10
2.2	Recommended Layout for Direct U-turn	12
2.3	Special Indirect U-turn	13
2.4	The fundamental diagrams according to Greenshields; (a) Speed – Density Model, (b) Speed – Flow Model and (c) Flow – Density Model (Source: Greenshields, 1934)	23
2.5	Carriageway lanes of road section	26
2.6	Classification of roadway capacity-estimation methods (Michiel M. Minderhoud, 1997)	29
2.7	Shockwave description. (Notes: Q denotes capacity, Q_{S1} denotes positive shockwave, Q_{S2} denotes stationary shockwave, Q_{S3} denotes negative shockwaves, k_c denotes critical density, k_{s_1} denotes the corresponding density for Q_{S1} , k_{s_2} denotes the corresponding density for Q_{S2} , and k_{s_3} denotes the corresponding density for Q_{S3})	33
3.1	Flow Chart of Study	37

3.2	Federal Highway FT005, Skudai – Pontian, Johor Bahru	39
3.3	Setup of Impact Study	41
3.4	The ATC's setup at the selected site	42
3.5	Automatic Traffic Counter (ATC)	44
3.6	Speed – Density Relationship	46
4.1	Traffic Proportion	50

4.2	Location of ATC's	52
4.3	Speed – Density relationship for ATC 01 (Site A)	59
4.4	Speed – Density relationship for ATC 02 (Site A)	60
4.5	Speed – Density relationship for ATC 03 (Site B)	61
4.6	Speed – Density relationship for ATC 04 (Site B)	62
4.7	Flow – density relationship at Site A	69
4.8	Flow – density relationship at Site B	69
4.9	Traffic Shockwave Velocity on the roadway segment	71

LIST OF SYMBOLS/ ABBREVIATIONS

AADT	Annual Average Daily Traffic
ADT	Average Daily Traffic
ATC	Automatic Traffic Count
HCM	Highway Capacity Manual
HGV	Heavy Good Vehicle
hr	Hour
JKR	Jabatan Kerja Raya / Public Work Department
k	Density
k_c	Critical Density
k_j	Jam Density
km	Kilometer
p.c.u	Passenger Car Unit
Q	Capacity
q	Flow
q_{max}	Maximum Flow
R^2	Regression
SSD	Stopping Sight Distance

u	Speed
u_f	Free Flow Speed
u_c	Optimum Speed
veh	Vehicle

LIST OF APPENDICES

APPENDIX	TITLE	PAGE
A	Scheme F Classification	78
B	Sample of raw data from each counters installed for the first 5-minutes intervals	80
C	Sample of Calculation	97

CHAPTER 1

INTRODUCTION

1.1 Background of the Study

Traffic congestion problem and excessive traffic delays that is happening all around the world on urban and suburban multilane highways may due to the ineffective installation of traffic signal control system. In recent years, there has been major interest in “access management” as a new reaction to this situation. Based on TRB (2003), access management refers to the design, implementation, and management of entry and exit points between roadways and adjacent properties. Access management calls for improvements in access control, spacing, and design to preserve the functional integrity of the road system (Koepke & Levinson, 1992). One of the access management strategies is to prohibit the left turn movements at unsignalized driveways and the deterred traffic will be forced to make a U-Turn.

The clear advantage of prohibiting right turns at midblock driveways is an immediate increase in safety by the reduction in crashes. A study in California, United States which is applying right-hand traffic stated that by simplifying driving tasks could significantly reduce the crash rate (Vargas & Gautam, 1989). Other than

can reduce the number of conflict points between road users it also provide smoother, more reliable, and potentially higher travel speeds for arterial traffic.

As for Malaysia which has left-hand traffic, the approach is to prohibit the right turn movements by allowing the left turning movements only. This approach often coupled with provision of midblock U-turns in the downstream to accommodate the right turning traffic. This system is called the left turn U-turn (LTUT) system. The most common factors that influence the decision of this approach are the volume on the roadway and crash rates involving fatality at the site is high.

A quantitative assessment of traffic stream which is roadway capacity is based on the relationship of flow, speed and density. According to TRB (2000), the capacity of a facility is defined as the maximum hourly flow rate at which vehicles can reasonably be expected to traverse a point or section of a roadway under prevailing roadway, traffic and control conditions. By knowing the capacity of the roadway, we can do improvements to the roadway design and also traffic management.

Due to the midblock U-turn on downstream, it would create the phenomenon of backups and queuing on a highway due to a sudden reduction of the capacity of the highway which is known as a bottleneck condition. The sudden reduction in capacity could be due to a crash, reduction in the number of lanes, restricted bridge sizes, work zones, a signal turning red and so forth (Nicholas J. Garber & Hoel, 2009). Any change made on the roadway for example installing midblock U-turn facilities would result in highway capacity loss. Not only changes of roadway geometry is the only factor of highway capacity loss, the traffic conditions and also the ambient condition like dry and wet condition also contribute to highway capacity loss. When capacity loss happens, it would lead to traffic shockwaves.

Traffic Shockwave is a motion of propagation of a change in density and flow. Where traffic stream is moving at a speed in close proximity and lead vehicle driver step on the brake, if the follow up driver lose their nerves on sighting the brake lights, abrupt braking will trigger shockwaves (Johnnie Ben-Edigbe & Mashros, 2011). A driver is influenced mostly by motion of vehicles downstream of him or her. The shockwave speed is conventionally represented by the rate of change in volume and density (Lighthill & Whitham, 1955; Richards, 1956). When shockwaves happen, it would lead to reduction of roadway level of service.

1.2 Problem of Statement

Over the past few decades, there have begun considering the use of U-turn as an alternatives to direct left turn for the right-hand traffic and direct right turn for the left-hand traffic. The driving side depends on the country itself. Past studies have indicate that direct left turn maneuvers increase delay, conflicts, and crashes, and they reduce capacity and mobility in the major traffic stream and stated that the access management which providing a right turn followed by U-turns (RTUT) system gives many advantages like reducing the conflict points at the intersections.

Somehow, there are also arguments have been raised by some opponents of median modification projects, that the increase number of U-turning vehicles may result in safety and operational problems on multilane highways. Presently, there is a lack of information especially on the extant of traffic shockwave propagation induced by highway midblock U-turn facilities based on empirical research for Malaysia multilane highway condition.

1.3 Aim and Objectives

The aim of this study is to determine the extent of traffic shock wave propagations induced by highway midblock U-Turn facilities, and the objectives of this are:

- i. Estimate flow and density for road section.
- ii. Estimate and compare traffic flow for road section with and without midblock U-turn facility.
- iii. Estimate and compare density for road section with and without midblock U-turn facility.
- iv. Determine shockwave speed for road section.

1.4 Scope of Study

This study is focused on the determining the shockwave propagations of highway midblock U-turn facilities at FT005, Skudai – Pontian highway as shown in Figure 1.1. In achieving the capacity and shockwaves associated with midblock U-turn, the method covered in this study are direct empirical methods which are directed at estimation of capacity values at a specific site with traffic observations from site. The study relied on fundamental diagrams of traffic stream where flow is a function of speed multiply by density. The data collection is taken 24 hours per day for 3 weeks.

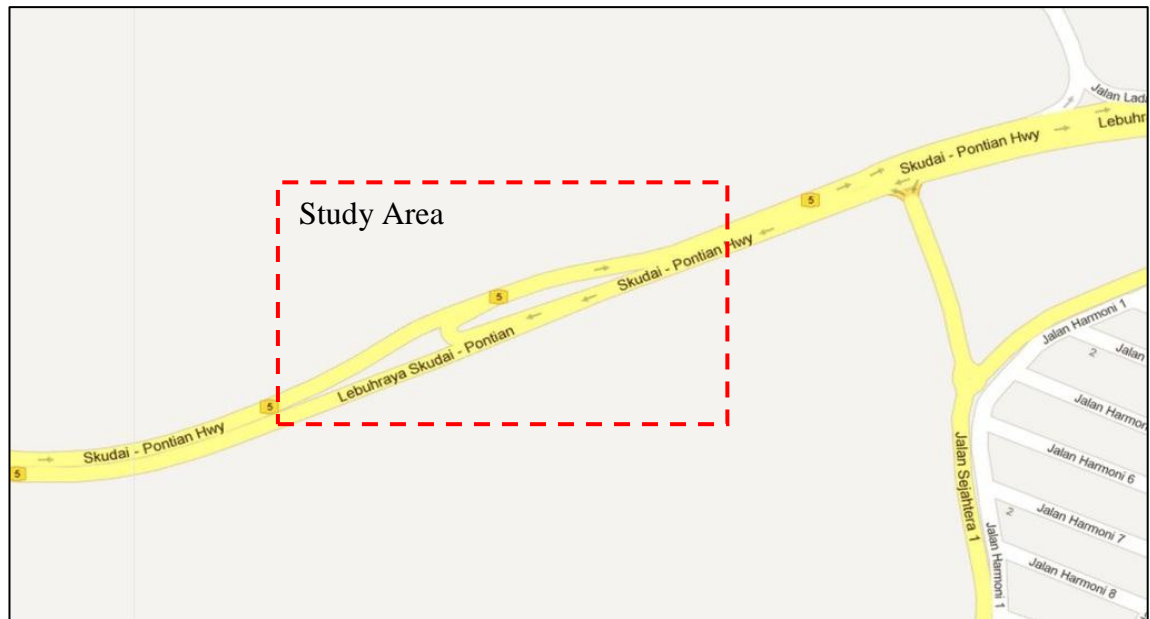


Figure 1.1: Midblock U-turn facilities at FT005

(Source: <http://maps.google.com>)

1.5 Significance of Study

The shockwave propagation at any point along a road has a major implication in traffic flow analysis, modelling, traffic engineering design and also traffic management. Shockwaves would affect the level of service of the roadway. Data obtained from this study can be used as a reference for future traffic improvements and developments on the site location. It also can be used as a basis for traffic design and timing of traffic signals if need to implement for the future. By all the improvements, it helps to ease traffic flow and to decrease the travel time therefore minimizing the travel cost.

REFERENCES

- Arahan Teknik (Jalan) 8/86. *A Guide on Geometric Design of Roads*. Kuala Lumpur: Public Works Department Malaysia.
- Ben-Edigbe and Ferguson (2005). *Qualitative Road Service Reduction Resulting from Pavement Distress*. Paper presented at the WIT International Conference on Urban Transport.
- Ben-Edigbe, J., & Mashros, N. (2011). Determining Impact of Bus-Stops on Roadway Capacity. *Proceedings of the ITRN 7*.
- Carter, D., Hummer, J.E., Foyle, R.S. (2005). Operational and Safety Effects of U-turns at Signalized Intersections. . *Journal of the Transportation Research Board, No. 1912*, 11-18.
- Castillo, J. M. D., & Benítez, F. G. (1995). On the functional form of the speed-density relationship—I: General theory. *Transportation Research Part B: Methodological*, 29(5), 373-389.
- Dissanayake, S., Lu, J.J., Castillo, N., Yi, P.,. (2002). Should Direct left turns from driveways be avoided? A Safety Perspective. *ITE Journal*, 72 (6), 26-29.

- Greenberg, H. (1959). An Analysis of Traffic Flow. *Operations Research*, 7(1), 79-85.
- Greenshields B. D., Bibbins J. R., Channing W. S., & H., M. H. (1935). A Study of Traffic capacity. *Transportation Research Board*, 14, 448-477.
- Public Work Department. (2011). *Malaysia Highway Capacity Manual*, Malaysia
- Koepke, F. J., & Levinson, H. L. (1992). *Access management guidelines for activity centers*, National Cooperative Highway Research Program Rep. No. 348, Transportation Research Board, National Research Council, Washington, D.C.
- Lighthill, M. J., & Whitham, G. B. (1955). On kinematic waves II: A theory of traffic flow on long crowded roads. *Proceedings of the Royal Society of London*, A(229), 317-345.
- Liu, P., Lu, J. J., & Chen, H. (2008). Safety effects of the separation distances between driveway exits and downstream U-turn locations. [doi: 10.1016/j.aap.2007.09.011]. *Accident Analysis & Prevention*, 40(2), 760-767.
- Liu Pan, P. T., Lu Jian, Cao Bing. (2009). Estimating Capacity of U-Turns at Unsignalized Intersections: Conflicting Traffic Volume, Impedance Effects, and Left-Turn Lane Capacity. *Transportation Research Record: Journal of the Transportation Research Board*, 2071, 44-51.
- May Adolf D. (1990). *Traffic Flow Fundamentals*: Prentice Hall.
- Michiel M. Minderhoud, H. B., Piet H. L. Bovy. (1997). Assessment of Roadway Capacity Estimation Methods. *Transportation Research Record* 1572, 59-67.

- Newell, G. F. (1993). A simplified theory of kinematic waves in highway traffic, part I: General theory. [doi: 10.1016/0191-2615(93)90038-C]. *Transportation Research Part B: Methodological*, 27(4), 281-287.
- Nicholas J. Garber, & Hoel, L. A. (2009). *Traffic and Highway Engineering, Fourth Edition*: Cengage Learning.
- Potts, I. B., Harwood, D.W., Torbic, D.J., Richard, K.R., Gluck, J.S., Levinson, H.S., . (2004). *Safety of U-turns at Unsignalized Median Opening*, : National Cooperative Highway Research Program, Transportation Research Board.
- Raha Abd Rahman, Ben-Edigbe, Azmi Hassan. (2012). *Extent of Traffic Kinematic Waves and Delays Caused By Midblock U-turn Facilities*. Paper presented at the Irish Transport Research Network
- Richards, P. I. (1956). Shock Waves on the Highway. *Operations Research*, 4(1), 42-51.
- Roger P. Rosess, E. S. P., William R. McShane. (2004). *Traffic Engineering*. United State of America: Pearson Prentice Hall.
- TRB. (2000). *Highway Capacity Manual*. Transportation Research Board, National Research Council, Washington, D.C.
- TRB. (2003). *Access Mangement Manual*. Transportation Research Board, National Research Council, Washington, D.C.
- Vargas, F. A., & Gautam, Y. (1989). *Problem: Roadway Safety vs. Commercial Development Access*. Paper presented at the Proc., ITE 59th Annual Meeting, San Diego, California.