DESIGN OF MECHANICALLY STABILIZED EARTH WALL – REINFORCED EARTH WALL

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Dedicated

То

Almighty GOD

То

My beloved husband and family,

Employer,

Father and mother,

Brother and sisters,

Friends

For your love and support

ACKNOWLEDGEMENT

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ABSTRACT

Mechanically Stabilized Earth (MSE) Walls are internally stabilized fill walls that are constructed using alternating layers of compacted soil and reinforcement such as geotextiles, metallic strips or rods of metal. Among the types of MSE Wall, Reinforced Earth (RE) Wall is commonly used in the construction industries nowadays. A reinforced soil should be stable overturning, sliding and bearing capacity and also respect to the internal stability. The main objective of this study is to investigate the factors that influence in the designing of reinforced earth wall through the input of case studies of project at Jalan Tumang, Fasa 1, Segamat, Johor Darul Takzim using spreadsheet. The result will be compared with field data to obtain the best solution of RE wall design for both safety and economic factors.

ABSTRAK

Tembok penstabilan tanah Mekanikal (MSE) merupakan tembok penstabilan dalaman yang dibina dengan menggunakan beberapa lapisan tanah terpadat dan melibatkan penggunaan tetulang seperti geotextiles, jalur logam atau rod logam. Antara jenis Tembok MSE yang digunakan ialah tembok tanah bertetulang biasanya digunakan dalam industri pembinaan pada masa kini. Sebuah tembok tanah bertetulang perlu dianalisis dengan kestabilan luaran seperti kestabilan dari keterbalikan, gelongsor dan keupayaan galas serta juga perlu mengambilkira keperluan semakan keatas kestabilan dalaman. Objektif utama kajian ini adalah untuk menyiasat faktor keselamatan yang mempengaruhi rekabentuk tembok tanah bertetulang melalui input kajian kes projek Jalan Tumang, Fasa 1, Segamat, Johor menggunakan spreadsheet yang direkabentuk. Hasil yang diperolehi akan dibandingkan dengan data lapangan bagi memperolehi penyelesaian yang terbaik keatas rekabentuk tembok tanah bertetulang bagi kedua-dua faktor.

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LIST OF SYMBOLS

$\sigma_{\rm v}$	-	Vertical pressure
$\sigma_{\rm h}$	-	Horizontal pressure
γ	-	Effective unit weight of the soil
Z	-	Depth
K ₀	-	At rest earth pressure coefficient.
Н	-	Height of wall
P_0	-	At rest total pressure
Ka	-	Active earth pressure coefficient
$\mathbf{K}_{\mathbf{p}}$	-	Passive earth pressure coefficient
q	-	Surcharge unit area
Ø'	-	Soil friction angle
P_a	-	Total active pressure
P_p	-	Total passive pressure
W	-	Weight of soil
b	-	Strip breadth
t	-	Strip thickness
$FOS_{(sliding)}$	-	Factor of safety against sliding
$\sum F_{R'}$	-	Sum of horizontal resisting forces
$\sum F_d$	-	Sum of horizontal driving forces
$FOS_{(overturning)}$	-	Factor of safety against overturning
$\sum M_{\rm P}$	-	Sum of the moment of forces tending to overturn
		about point
$\sum M_{\alpha}$	-	Sum of the moment of forces tending to resist
		overturn about point

$FOS_{(bearing \ capacity)}$	-	Factor of safety against bearing capacity
$N_{\gamma,}N_{c,}N_{q}$	-	Bearing capacity factors
FOS_B	-	Factor of safety against tie break
$f_{\mathcal{Y}}$	-	Yield or breaking strength of the material
S_{v}	-	Vertical spacing of reinforcement
S_H	-	Horizontal spacing of reinforcement
FOS_p	-	Factor of safety against tie pullout
${\it extsf{/}}_{\mu}$	-	Angle of friction of soil - strip
L	-	Tie length

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CHAPTER 1

INTRODUCTION

1.1 Background of the Study

Reinforced Earth Wall is very significant and commonly used in construction industry especially in infrastructure projects. At present, the mechanically stabilized earth walls are probably the most used particularly for roadwork where deep cuts or hill side road locations require retaining wall to hold the earth in place.

The mechanically reinforced earth wall uses the principle of placing reinforcing into the backfill using devices such as metal strips and rods, geotextile strips and sheets and grids or wire grids (Figure 1.1). The three basic components of mechanically reinforced earth wall are:

- 1. The earth fills usually select granular material with less than 15 percent passing the 200 sieve.
- Reinforcement strips or rods of metal, strips or sheets of geotextiles, wire grids, or chain link fencing or geogrids fastened to the facing unit and extending into the backfill some distance.
- Facing units not necessary but usually used to maintain appearance and to avoid soil erosion between the reinforcements.



Figure 1.1: The Reinforced Earth wall concept (After Vidal 1969)

As stated by British Standard Institution (BSI) (1995), the design of reinforced earth wall shall involve internal and external stability checks. The general guidance by BSI is the reinforcing strip length shall be more than 70% of the height of wall. The external stability checks of the wall are performed on sliding, overturning and bearing failure. As for internal stability design, checking is done on the tensile strength of steel strips and frictional resistance against horizontal force (Figure 1.2).

1.2 Problem Statement

Reinforced Earth is a composite material which is formed by the association of soil and tension resistant reinforcing elements. The reinforcement suppresses the normal tensile strains in the soil mass through frictional interaction. Reinforced earth wall has widely been adopted due to its economy, ease of construction and flexibility in nature. The internal stability check and external stability check need to be calculated in designing the reinforced earth wall. These conditions are based on the same limit states that apply to design the gravity wall.

The external stability comprises of checking the safety of the wall as a rigid block against overturning, sliding, bearing capacity failure and overall stability. In internal stability, the competency of the reinforcements provided at different heights is examined against tension and pull-out failures. The internal stability analysis of a reinforced earth wall requires the identification of rupture surface behind the panels of the wall and during pull-out failure, and the effective length of the reinforcement contributing to the development of frictional resistance that lies outside the wedge. Further, the earth pressure distribution behind the wall is required to design the size and type of reinforcement.

This repeating process involves many varieties, hence they have to be carried out through trial and error until the design structures fulfils the standard requirement, safe and economic. Due to the repetition works, in this study, the usage of computer program can be developing to assist the design, hence accelerate the analysis and design process.

1.3 Aim and Objectives

The aim of this study was to review the design of reinforced earth wall in terms of safety and serviceability. This study will review the most important item in highway which is reinforced earth wall. The objectives of this study are as follows:

 To analysis the factor of safety of Reinforced Earth wall in terms of internal and external stability checks. To validate with field data and structured the best solution of Reinforced Earth wall design using strip reinforcement designs.

1.4 Scope and Limitation of Study

To ensure that the study conducted will achieve the aim and objectives, the scope of study only focused on the strip reinforcement design. The Rankine's and Mohr Coulomb Theory are applied in checking the external stability of reinforced earth wall. For the internal stability, the analysis is to checking the tension failure, the stability is considered of an internal wedge of soil above each reinforcement level and the pullout failure, where the pull out capacity is checked by considering the bond length of reinforcement required belong each wedge of soil above that layer. The assumption to undertake in the program data sheet are the soil used are granular soil and there is no effect of pore water pressure. The scope of this study will focus on one case study which is the proposed slope stabilization works at Fasa 1, Jalan Tumang, Segamat, Johor Darul Takzim.

1.5 Significance of Study

From the study, the analysis and designing of reinforced earth wall can be established and it is expected to maintain the stability of slope at hillside. The most important aspect will be considered in this study is to investigate the factors that influence in the designing the reinforced earth wall and make a good comparison instead of economical and construction wise.

REFERENCES

- Aggour, M.S. & Brown, C.B. (1974). The prediction of earth pressure on retaining walls due to compaction. Ceotechnique, Vol 24, pp 489-502.
- Akroyd, TNW, (1996), 'Earth-Retaining Structures: Introduction to the Code of Practice', The Structural Engineer, vol. 74, No. 21, pp 360-364.
- Bjerrim, L. C Eide, 0. (1956). *Stability of strutted excavations in clay. Geotechnique*, Vol. 6, pp 32-47.
- Bolton, M.D., (1996), 'Geotechnical design of retaining walls', The Structural Engineer, 74, 21, 365-369.
- Bowles, J.E. (1977) Foundation Analysis and Design. McGraw Hill, New York 750p
- British Standards BS 8002 (1994), 'Code of Practice for Earth Retaining Structures', London, British Standards Institution.
- British Standards Institution, Code of Practice for Earth Retaining Structures.
- British Standards Institution, Milton Keynes, 1994, BS 8002. Foundation Engineers, Budapest, pp 373-384.
- British Standards Institution (1972). Code of Practice for the Structural Use of Concrete, CP 110:1972. British Standards Institution, London, 54 p.
- British Standards Institution (1972). Code of Practice for Foundations, CP 2004:1972. British Standards Institution, London, 158 p.

- British Standards Institution (1978). Steel, Concrete and composite bridges -Specification for loads, BS 5400:Part 2: 1978. British Standards Institution, London, 158 p.
- British Standards Institution, Eurocode 7. Geotechnical Design: Part 1. British Standards Institution, Milton Keynes, 1995, DD ENV 1997-1.
- Brinch Hansen J., A revised and extended formula for bearing capacity. Danish Geotechnical Institute Bulletin, 1970, 28.
- Broms, B.B. (1971). Lateral Earth Pressure Due to Compaction of Cohesionless Soils. Proceedings of the 5th Budapest Conference on Soil Mechanics
- Canadian Geotechnical Society (1978). Canadian Foundation Engineering Manual, Part 4. Canadian Geotechnical Society, Ottawa, 68 p.
- Caquot, A. & Kerisel, J. (1948). Tables for the Calculation of Passive Pressure Active Pressure and Bearing Ccrpacity of Foundations. (Translated from the French by H.A. Bec, London) Gauthier - ViLlars, Paris; 120 p.
- Carder D. R. A Comparison of Embedded and Conventional Retaining Wall Design Using Eurocode 7 and Existing UK Design Methods. Transport Research Laboratory, Crowthorne, 1998, TRL Report 320.
- Cedegren, H.R. (1977). Seepage, Drainage & Flow Nets. 2nd Ed. Wiley, New York, 534 p.
- Clayton C. R Milititsky and Woods R. I, Earth Pressures and Earth Retaining Structures. Blackie Academic and Professional, London, 1993.

- Department of Transportation, Federal Highway Administration, Washington D.C. (2003). Soil Nail Walls, Geotechnical Engineering Circular No. 7, FHWA Publication No. FHWA0-IF-03-017.
- *Eurocode 7 Geotechnical design-Part 1*, DD ENV 1997-1:1995, British Standards Institution.
- Highways Agency, Backfilled Retaining Walls and Bridge Abutments. Design Manual for Roads and Bridges, Volume 2, Section 1. Highways Agency, London, 1987, BD30/87.
- Huntington, W.C. (1957). "Trial Wedge Method," Earth Pressures and Retaining Walls, John Wiley & Sons, Inc., New York, NY, pp. 73-109.
- Institution of Structural Engineers, Earth Retaining Structures. Institution of Structural Engineers, London, 1951, Civil Engineering Code of Practice No. 2 (CP2).
- Khan, I.N. and Saran, S. (2004) A Model Study on Reinforced Earth Wall. Proceedings International Conference on Bridge Engineering and Hydraulic Structures, 26- 27 July 2004, Kuala Lumpur, pp. 295-298.
- Kerisel J. and ABSE E, Active and Passive Earth Pressure Tables, 3rd edn. Balkema, Rotterdam, 1990.
- Kumar, A., and Saran, S. (2003) Closely Spaced Footings on Geo-grid-Reinforced Sand. *Journal of Geotechnical and Geoenvironmental Engineering, ASCE*, July 2003, pp. 660-664.

- Leshchinsky, D. (1985) Design Manual for Geolextile Retained Earth Walls. Research Report No. CE-85-51, Department of Civil Engineering, University of Delaware New York, DE-1976.
- Mechanically Stabilized Earth Walls and Reinforced Soil Slopes Design and Construction, FHWA Publication No. FHWA-NHI-00-043.
- Miller, E.A., and Roycroft, G.A. (2004) Seismic Performance and Deformation of Levees: Four Case Studies. *Journal of Geotechnical and Geoenvironmental Engineering*, ASCE, April, pp. 344-354.
- Muni Budhu, 2007, Soil Mechanics and Foundation, 2nd Edition
- Oliphant and Dougall, (2003) 'A Case Study on the Design of Cantilever Embedded Retaining Walls', Submitted to Geotechnique.
- Puller M. & Lee C. K. T., (1996), 'Comparative studies by calculation between design methods for embedded and braced retaining walls recommended by BS 8002: 1994 and previously used methods', Proc. Instn Civ. Engrs, Geotechnical Eng'g, 119, Jan., pp 35-48, ICE.
- Ramaswami, S.V. and Bose, G.S.C. (1989) Prototype Geogrid Reinforced Retaining Wall. Proceedings Indian Geotechnical Conference (IGC-89), Vol. 1, pp. 411-414.
- Simpson B. and Driscoll R. Eurocode 7: A Commentary. Building Research Establishment, Garston, 1998, Construction Research Communications.
- South Carolina Department of Transportation, *Bridge Design Manual*, dated April 2006.

- South Carolina Department of Transportation, *Highway Design Manual*, dated April 2003.
- Terzaghi, K. And Peck, R. B. (1967). *Soil Mechanics In Engineering Practice*, 2nd edition. John Wiley, New York, London, Sydney
- V.N.S Murthy (2003), *Geotechnical Engineering : Principles and Practices of soil mechanics and foundation Engineering;* Marcel Dekker. New York.
- Wong Weng Sing, Shim Woon Choon, Simplified Solutions for Retaining Wall Design, 2011