# Improvement of Structural Stability for Multi –Storey Building by Using Composite Column and Cable Connection

Yashar Arvin (U16353051)

A project report submitted in partial fulfilment of the requirements for the award of the degree of Master of Engineering (Civil-Structure)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > April 2010

... In loving appreciation of my dear family and friends ... ...May God shower his blessing on you...

...Love you forever...

# Acknowledgement

I would like to thank all my friends and people who has given the cooperation to me in writing this project report. I am sincerely grateful to my supervisor, Dr. Suhaimi Abu Bakar for his continuous support and guidance to set a high standard for the conduct of this study and his valuable suggestions confer to complete this project report.

### Abstract

Two methods for the improvement of structural stability of the building are introduced. These methods ensure that the structural building is safe, reliable and economy with the production of small deflection against seismic loading and any other external effect. The application of composite column as flexible material or non-rigid part of the frame structure is introduced and capable to absorb energy comes from the action and reaction of loading. The second approach in this research is by introducing active spring or prestressed cable connections in order to reduce the instability of structural building. The effect of P-delta on two methods stated is studied by performing non-linear analysis by using SAP2000 software on the building and compared the analysis results to conventional structural building with rigid connection. The results from SAP2000 shows that the proposed composite column and structural building with prestressed cable connections under loading compared with the deflection induced by conventional building with rigid connection. This finding proves that the proposed methods for building construction can increase the stability of the structural building.

### Abstrak

Dua kaedah untuk pembaikan kestabilan struktur bangunan diperkenalkan. Kaedah ini menjamin struktur bangunan yang selamat, boleh dipercayai dan ekonomi dengan penghasilan pesongan yang sedikit terhadap pembebanan seismik dan lain-lain kesan luaran. Penggunaan tiang komposit sebagai bahan fleksibel atau bahagian tidak tegar dalam struktur kerangka diperkenalkan dan berkebolehan untuk menyerap tenaga daripada tindakan dan tindakbalas pembebanan. Penggunaan kedua dalam kajian ini tertumpu kepada pengenalan spring aktif atau sambungan kabel prategasan bagi tujuan pengurangan ketakstabilan struktur bangunan. Kesan P-delta terhadap kedua-dua kaedah yang dinyatakan dikaji dengan mengadakan analisis tak linear menggunakan perisian SAP2000 ke atas bangunan dan membandingkan keputusan analisis dengan struktur bangunan bersambungan tegar yang biasa. Keputusan daripada SAP2000 menunjukkan struktur bangunan dengan pembinaan tiang komposit yang dicadangkan dan juga sambungan kabel prategasan boleh menghasilan pesongan yang kecil di bawah pembebanan berbanding dengan pesongan yang dihasilkan oleh bangunan biasa dengan sambungan tegar. Penemuan ini membuktikan kaedah yang dicadangkan bagi pembinaan bangunan boleh meningkatkan kestabilan struktur bangunan tersebut.

# Table of Contents

CHAPTI	ER TITLE	PAGE
	Declaration	ii
	Dedication	iii
	Acknowledgements	iv
	Abstract	v
	Abstrak	vi
	Table of Contents	vii
	List of Tables	Х
	List of Figures	xii
1	Introduction	
1.1	General	1
1.2	Statement of Problem	2
1.3	Aim and Objective	3
1.4	Scope of project	4
1.5	Organization of This Report	5
2	Literature Review	
2.1	General	6
2.2	Existing Methods for Improvement of Structural Stability	8
2.2.1	Steel Plate Shear Walls	8
2.2.1.1	Strip Model	9
2.2.2	Steel Bracing Model	10

	٠	٠	٠	
V	1	1	1	

CHAPTER	TITLE	PAGE
2.3	Composite Column	10
2.4	Stability and the Effective Length for Column	12
2.5	Existing Analytical Modeling	12
2.5.1	Linear Centerline Model	13
2.5.2	Elastic Model with Panel Zone	13
2.5.3	Nonlinear Centerline Model	14
2.5.4	Nonlinear Model with Panel Zone	15
2.6	Analytical Modeling of Panel Zone	17
2.7	Analytical Modeling of Beam-connection	17
2.8	Active Control Cable	19
2.9	Isolation System for Vibration Stay Cables Control	21
3	Analysis and Modeling of Proposed Composite column	
3.1	Composite Columns	23
3.2	Advantage of Using Composite Material	25
3.3	Disadvantages of Using Composite Column	26
3.4	Flexible Column by Using Steel and Concrete Components without Cable	26
3.4.1	Data Collection	27
3.4.2	Data Analysis	28
3.4.2.1	First Analysis	28
3.4.2.1.1	Critical Lateral Load and Deflection (P-Delta effect)	28
3.4.2.1.2	Critical Compression Load and 20% of Critical Lateral Load, Deflection	37
3.4.2.2	Second Analysis	54
3.5	Flexible Column by Using Steel and Concrete Components with Cable(Prestress Composite Column)	68
3.6	Discussion of results	82

-	
	**
	v
	•

#### CHAPTER

## TITLE

4	Analysis and Modeling of multi-storey building with Cable Connection	
4.1	Cable Connection	85
4.2	Spherical Link	86
4.3	Circular Support with Cable	87
4.4	Mechanism of Cable Connection	87
4.5	Advantage of Using Cable Connection	88
4.6	Disadvantages of Using Cable Connection	88
4.7	Presume Multi-Storey Building Properties	89
4.8	Data Analysis for Frame	90
4.8.1	P-Delta Analysis for Frame without Prestress Cable(First Analysis)	90
4.8.2	P-Delta Analysis for Frame with Proposed Prestress Cables in Connection(Second Analysis)	97
4.8.3	Third Analysis	105
4.9	Rigid Connection	112
4.10	Discussion of the Analysis Results	120
5	Conclusions	
5.1	Composite Column	122
5.2	Cable Connection	123
6	References	124

# List of Tables

TITLE

Table NO.

Table 3.1	Critical Lateral Load Steel Section (ST275 UC305x305x240)	36
Table 3.2	Critical Lateral Load Concrete Section (355x321)	36
Table 3.3	Critical Lateral Load Composite Section(Steel 305x305x240 Top, Bottom ,Concrete 355x321 8#18 at middle)	37
Table 3.4	Critical (Compression and 20% Lateral Load ) Steel Section (ST275 UC305x305x240)	53
Table 3.5	Critical (Compression and 20% Lateral Load ) Concrete Section (355x321 8#18)	53
Table 3.6	Critical (Compression and 20% Lateral Load )Composite Section (Steel 305x305x240top, bottom ,Concrete 355x321 at middle)	53
Table 3.7	Maximum Compression Load and 20% Critical Lateral Load ,Steel Section (ST275 UC305x305x240)	67
Table 3.8	Maximum Compression Load and 20% Critical Lateral Load , Composite Section (Steel 305x305x240top, bottom and Concrete 355x321 at middle Without Cable)	67
Table 3.9	Cable Strain for Composite Section (Steel 305x305x240 Top, Bottom and Concrete 355x321 at middle With Cable)	68
Table 3.10	Maximum Compression Load and 20% Critical Lateral Load , Composite Section With Cable	78

## Table NO.TITLE

xi

Table 3.11	Deflection for Steel Section (UC 305x305x240)Under Maximum Compression Load ,20% Critical Lateral Load	82
Table 3.12	Comparing Lateral Deflection for Steel Section (305x305x240), Composite Column Without Cable and With Cable	84
Table 4.1	Columns (UC305x305x240) Under Axial Forces (First Analysis)	97
Table 4.2	Beams (UB305x165x54) Under Axial Forces (First Analysis)	97
Table 4.3	Columns (UC305x305x240) Under Gravity Load with Using Cable in Connection (Second Analysis)	104
Table 4.4	Beam (UB305x165x54) Under Gravity Load with Using Cable in Connection (Second Analysis)	104
Table 4.5	Columns (UC305x305x240)Under Gravity Load and Lateral Load with Cable Connection (Third Analysis)	112
Table 4.6	Beam (UB305x165x54) Under Gravity Load and Lateral Load with Cable Connection (Third Analysis)	112
Table 4.7	Column (UC305x305x240) Under Gravity Load and Rigid Connection	120
Table 4.8	Beam (UB305x165x54) Under Gravity Load and Rigid Connection	120

# LIST OF FIGURES

FIGURE N	O. TITLE	PAGE
Figure 2.1	Strip model as shear wall	9
Figure 2.2	H Section Steel-Concrete Composite Column	11
Figure 2.3	Scissors Model	14
Figure 2.4	Nonlinear Centerline Model	15
Figure 2.5	Shi Model	16
Figure 2.6	Krawinkler Model	16
Figure 2.7	Moment – Rotation Behavior of Panel Zone	17
Figure 2.8	Simple Analytical Modeling of the Moment Connection	18
Figure 2.9	Brittle Fracture in Welded Part	18
Figure 2.10	Analytical Modeling of Moment Connection	19
Figure 2.11	Stay Cable With Using Damper For Isolation System	22
Figure 3.1.	Proposed Composite Column with Pretension Cable	24
Figure 3.2	Lateral Deflection with Lateral Load	28
Figure 3.3	Critical lateral load and deflection for L=3000(steel section)	29
Figure 3.4	Critical lateral load and deflection for L=5000(steel section)	29
Figure 3.5	Critical lateral load and deflection for L=7000(steel section)	30
Figure 3.6	Critical lateral load and deflection for L=3000(Concrete Section	) 30
Figure 3.7	Critical lateral load and deflection for L=5000(Concrete Section	) 31
Figure 3.8	Critical lateral load and deflection for L=7000(Concrete Section	) 31
Figure 3.9	Composite Section, L=3000 mm Steel section, Top	32
Figure 3.10	Composite Section, L=3000 mm Concrete section, Middle	32
Figure 3.11	Composite Section, L=3000 mm Steel section, Bottom	33

## FIGURE NO.

## TITLE

Figure 3.12	Composite Section, L=5m Steel section, Top	33
Figure 3.13	Composite Section, L=5m Concrete section, Middle	34
Figure 3.14	Composite Section, L=5m Steel section, Bottom	34
Figure 3.15	Composite Section, L=7m Steel section, Top	35
Figure 3.16	Composite Section, L=7m Concrete section, Middle	35
Figure 3.17	Composite Section, L=7m Steel section, Bottom	36
Figure 3.18	Critical(Compression ,20% lateral load),L=3m steel section	38
Figure 3.19	Critical(Compression, 20% lateral load),L=5m steel section	39
Figure 3.20	Critical(Compression, 20% lateral load),L=7m steel section	40
Figure 3.21	Critical(Compression, 20% lateral load),L=3m Concrete Section	41
Figure 3.22	Critical(Compression, 20% lateral load),L=5m Concrete Section	42
Figure 3.23	Critical(Compression, 20% lateral load),L=7m Concrete Section	43
Figure 3.24	Critical(Compression ,20% lateral load),L=3m Composite,Top	44
Figure 3.25	Critical(Compression ,20% lateral load),L=3m Composite,Middle	45
Figure 3.26	Critical(Compression ,20% lateral load),L=3m Composite ,Bottom	46
Figure 3.27	Critical(Compression ,20% lateral load),L=5m Composite,Top	47
Figure 3.28	Critical(Compression ,20% lateral load),L=5m Composite,Middle	48
Figure 3.29	Critical(Compression ,20% lateral load),L=5m Composite,Bottom	49
Figure 3.30	Critical(Compression ,20% lateral load),L=7m Composite,Top	50
Figure 3.31	Critical(Compression ,20% lateral load),L=7m Composite,Middle	51
Figure 3.32	Critical(Compression ,20% lateral load),L=7m Composite,Bottom	52
Figure 3.33	Maximum Compression Load and 20% Critical lateral load,L=3m steel section	55
Figure 3.34	Maximum Compression Load and 20% Critical lateral load,L=5m steel section	56
Figure 3.35	Maximum Compression Load and 20% Critical lateral load,L=7m steel section	57

#### FIGURE NO. TITLE PAGE Figure 3.36 Maximum Compression Load and 20% Critical Lateral Load, 58 L=3m Composite Section, Top part Figure 3.37 Maximum Compression Load and 20% Critical Lateral Load, 59 L=3m Composite Section, Middle part Maximum Compression Load and 20% Critical Lateral Load, 60 Figure 3.38 L=3m Composite Section .Bottom part Maximum Compression Load and 20% Critical Lateral Load, Figure 3.39 61 L=5m Composite Section .Top part Figure 3.40 Maximum Compression Load and 20% Critical Lateral Load, 62 L=5m Composite Section, Middle part Maximum Compression Load and 20% Critical Lateral Load, 63 Figure 3.41 L=5m Composite Section ,Bottom part Maximum Compression Load and 20% Critical Lateral Load, 64 Figure 3.42 L=7m Composite Section, Top part Maximum Compression Load and 20% Critical Lateral Load, 65 Figure 3.43 L=7m Composite Section ,Middle part Maximum Compression Load and 20% Critical Lateral Figure 3.44 66 Load,L=7m Composite Section ,Bottom part Maximum Compression Load and 20% Critical Lateral Figure 3.45 69 Load,L=3m Composite Section ,Top part (With Cable) Maximum Compression Load and 20% Critical Lateral 70 Figure 3.46 Load,L=3m Composite Section ,Middle part(With Cable) Maximum Compression Load and 20% Critical Lateral 71 Figure 3.47 Load,L=3m Composite Section ,Bottom part (With Cable) Maximum Compression Load and 20% Critical Lateral Figure 3.48 72 Load,L=5m Composite Section ,Top part (With Cable) Maximum Compression Load and 20% Critical Lateral 73 Figure 3.49 Load,L=5m Composite Section ,Middle part (With Cable) Maximum Compression Load and 20% Critical Lateral 74 Figure 3.50 Load,L=5m Composite Section ,Bottom part (With Cable) Figure 3.51 Maximum Compression Load and 20% Critical Lateral 75 Load,L=7m Composite Section ,Top part (With Cable) Figure 3.52 Maximum Compression Load and 20% Critical Lateral 76 Load,L=7m Composite Section ,Middle part (With Cable)

## FIGURE NO.

## TITLE

Figure 3.53	Maximum Compression Load and 20% Critical Lateral Load,L=7m Composite Section ,Bottom part (With Cable)	77
Figure 3.54	Maximum Compression Load and 20% Critical Lateral Load from Composite Section ,L=3m Steel Section	79
Figure 3.55	Maximum Compression Load and 20% Critical Lateral Load from Composite Section ,L=5m Steel Section	80
Figure 3.56	Maximum Compression Load and 20% Critical Lateral Load from Composite Section ,L=7m Steel Section	81
Figure 4.1	Proposed Cable Connection for Multi-Storey Building	85
Figure 4.2	Proposed. Spherical Link	86
Figure 4.3.	Proposed Circular Support	87
Figure 4.4	Mechanism of cable connection	87
Figure 4.5	Proposed Cable Connection 3 Floors Frame	90
Figure 4.6	Columns (UC305x305x240) at Level 3 (First Analysis)	91
Figure 4.7	Columns (UC305x305x240)at Level 2, (First Analysis)	92
Figure 4.8	Columns (UC305x305x240)at Level 1, (First Analysis)	93
Figure 4.9	Beam (UB305x165x54)at Level 3, (First Analysis)	94
Figure 4.10	Beam (UB305x165x54) at Level 2, (First Analysis)	95
Figure 4.11	Beam (UB305x165x54) at Level 1, (First Analysis)	96
Figure 4.12	Columns (UC305x305x240) at Level 3, (Second Analysis)	98
Figure 4.13	Columns (UC305x305x240) at Level 2, (Second Analysis)	99
Figure 4.14	Columns (UC305x305x240) at Level 1, (Second Analysis)	100
Figure 4.15	Beam (UB305x165x54) at Level 3, (Second Analysis)	101
Figure 4.16	Beam (UB305x165x54) at Level 2, (Second Analysis)	102
Figure 4.17	Beam (UB305x165x54) at Level 1 , (Second Analysis)	103
Figure 4.18.	Proposed Cable Connection Frame Under Gravity Load (0.08 KN/mm) and Lateral Load(20%)	105
Figure 4.19	Columns (UC305x305x240) at Level 3, (Third Analysis)	106
Figure 4.20	Columns (UC305x305x240) at Level 2, (Third Analysis)	107
Figure 4.21	Columns (UC305x305x240) at Level $1$ , (Third Analysis)	108
Figure 4.22	Beam (UB305x165x54) at Level 3, (Third Analysis)	109

FIGURE NO.	TITLE	PAGE
Figure 4.23	Beam (UB305x165x54) at Level 2, (Third Analysis)	110
Figure 4.24	Beam (UB305x165x54) at Level 1, (Third Analysis)	111
Figure 4.25	Conventional Rigid Frame Under Gravity Load (0.08 KN/mm) and Lateral Load(20%)	113
Figure 4.26	Column (UC305x305x240) at Level 3 (Rigid Connection)	114
Figure 4.27	Column (UC305x305x240) at Level 2 (Rigid Connection)	115
Figure 4.28	Column (UC305x305x240) at Level 1 (Rigid Connection)	116
Figure 4.29	Beam (UB305x165x54) at Level 3 (Rigid Connection)	117
Figure 4.30	Beam (UB305x165x54) at Level 2 (Rigid Connection)	118
Figure 4.31	Beam (UB305x165x54) at Level 1 (Rigid Connection)	119

xvi

**Chapter 1** 

## Introduction

#### 1.1 General

Tall building structure, as like solid cylindrical column, is a slender structure where the deformation comes from gravitational force and lateral load is critical. Small change in gravitational force or lateral load will cause large change of displacement, especially for critical members and induce structural instability and may cause collapse of the entire structure. As it is well known as P-delta effect, the stability of column as important part of the building is intended to be studied. In order to reduce instability and improve the stability of the building; two methods are introduced; by using composite columns (steel-concrete with cable-steel) and pre stress and post tension concrete beam with cable connection, both types are using cables for connection. The effect of instability of the columns is assumed for the whole structure under the same load and the deflection of the column is predicted by using P-delta type of analysis. From last finding for connection, deflection for members in rigid connection frame as P-Delta effect is smaller than semi rigid and pin connection, so rigid connection is assumed to compare with cable connection.

#### **1.2 Statement of Problem**

Engineers nowadays normally use linear analysis to analyse building structure. This type of analysis contributes small lateral displacement for the building without considering instability effect or large lateral displacement of the building.

Analysis that considers geometric nonlinearity of the building, called P-delta analysis, is also available by established software such as SAP2000 and Staad-pro. By using nonlinear analysis, engineers nowadays always assumed the connection between column and beam is perfect or rigid. The P-delta analysis conducted to the structure, by assuming the connections are rigid, contributes large lateral displacement or P-delta effect. A quite number of researchers only consider the connections are non-rigid or pin type connection to overcome instability problem of the building.

An extensive study to a new type of connection (pin or non-rigid connections) to reduce instability of the building is become necessary and helpful, particularly for the building located in the country exposed to earthquake. Effect of P-Delta as a displacement on top part of column is reduced by assuming rigid or partially rigid connection compared to pin connection. Rigid connection is not safe in the welding method and connection behavior is out of supervision, also depends on more parameters in the place of erection such as degree of weather and degree of moist.

In analysis and modeling engineer just assume the connection is rigid but how percent is rigid behavior under the loads is not clear. In tall building the displacement of frame is very important and deflection for every floor is too small but additional of displacement will cause the structure become critical and instable.

Using solid column such as steel or concrete is just assumed to transfer gravity load and lateral load from top of building to foundation. The aim of analysis column is designed safe and stable purposed column in elastic part of material behavior and maybe not economical. Rigid connection with solid material (steel or concrete) in the analysis is designed for small width of bay by limitation in high.

#### **1.3** Aim and Objective

The objectives of this study are:

To use composite column with three material components (steel - concrete - steel) by using pre stress cable at middle part (concrete) in order to study P-Delta effect.

To use special connection with pre stress cable at both direction of connection (beam to beam and column to column) to study the P-Delta effect.

Understanding of P-delta type of analysis is the first step necessary to analyse the multi-storey building.

The building made-up of rigid connection, which nowadays normally used for building connection, is analysed by using P-delta type of analysis. The analysis results is used for reference purposes. In the second step of the study, the P-delta type of analysis is used to analyse the building made-up of proposed type of connection, i.e.

flexible composite column (steel-concrete-steel) attached by strain cable at concrete part of the column to absorb the energy. This type of connection is believed capable to reduce instability effect or large lateral displacement problems (p-delta effect). The third objective is the SAP2000 software; which capable to conduct P-delta analysis is fully applied in this study.

#### **1.4 Scope of project**

Scope of this study is examination of the behavior of the structure due to P-Delta effect and proposal of suitable methods for controlling this problematic issue according to the following procedures:

1-using composite column with pre stress cable to increase stiffness of element (steel-concrete with pre stress cable, steel).

2-Using the special connection for beam and column.

3-Understanding both above scope for composed together to design long bay and high length structure such as bridge or tall building with assuming top to bottom construct by hanging the members from top and constrict cables with pre stress and post tension methods in connection and composite columns.

#### 1.5 Organization of This Report

This project is organized in five chapters; the first chapter was written about introduction and scope of this issue and following Chapter 1, chapter two was purposed as literature review about some methods for improvement of structural stability and performance of composite column behavior under lateral and compression load as P-Delta effect. The third chapter was modeling of composite column by SAP2000 to compare the results to performance of composite column and steel or concrete column. Also in chapter fourth, connection for column and beam was supposed as cable connection and modeling by SAP2000 to understanding of performance of cable connection. The chapter five was written about the performance of composite column and cable connection as conclusion and recommendation.