



# Rheological and Morphological Characterization of Cup Lump Rubber-Modified Bitumen with Evotherm Additive

Suleiman Abdulrahman<sup>1,2</sup> · Fayez Alanazi<sup>3</sup> · Mohd Rosli Hainin<sup>1,4</sup> · Mohammed Albuaymi<sup>5</sup> · Hani Alanazi<sup>5</sup> · Musa Adamu<sup>6</sup> · Abdelhalim Azam<sup>3,7</sup>

Received: 11 June 2022 / Accepted: 15 February 2023 / Published online: 8 March 2023  
© King Fahd University of Petroleum & Minerals 2023

## Abstract

Natural rubber (NR) in the form of “cup lump” is used to significantly enhance the thermal stability and elasticity of bitumen. Despite these benefits, the paving industry has raised concerns about its increased energy use and carbon emissions when applied in hot mix asphalt (HMA). Warm mix asphalt (WMA) was invented to reduce this negative effect. In WMA, an additive is first added to the bitumen, which acts as a surfactant and allows the production and compaction of asphalt mixture at temperatures up to 50 °C less. As a result, the amount of energy consumed and carbon dioxide emissions during asphalt production are considerably reduced. In this study, CLR-modified bitumen (CMB) was blended with five percentages of Evotherm warm mix additive (0.3, 0.4, 0.5, 0.6, and 0.75%), and the properties were examined. According to the findings, the Evotherm modification lowered the viscosity of the binder by 26% and the contact angle by 6°, while the binder’s crack resistance at low temperatures marginally improves. Quantitative analysis from Fourier transform infrared spectra revealed a reduction in C=C stretch, C–O stretch, and C–H absorbance in response to the addition of Evotherm. Also, atomic force microscopy (AFM) scan shows an increase in the number of catana phases with a separation of peri- and para-phases. Only CMB with 0.75% Evotherm possesses 100% aggregate coating with sufficient air void; hence, it was selected as the optimum to be used in producing the warm mix asphalt mixture. Overall, the mixing temperature of the CMB can be lowered by 40 °C.

**Keywords** Warm mix asphalt · Natural rubber · Cup lump · Polymer-modified bitumen · Wettability · Evotherm

## 1 Introduction

The aggregate content of hot mix asphalt (HMA) is around 95% and the bitumen content is approximately 5%. Despite the fact that bitumen makes up such a small fraction of the total, it dictates the flexible pavement’s performance.

✉ Suleiman Abdulrahman  
asuleiman3@live.utm.my

✉ Fayez Alanazi  
fkalanazi@ju.edu.sa

Mohd Rosli Hainin  
roslihainin@ump.edu.my

Mohammed Albuaymi  
malbuaymi@mu.edu.sa

Hani Alanazi  
hm.alanazi@mu.edu.sa

Musa Adamu  
madamu@psu.edu.sa

Abdelhalim Azam  
amazam@ju.edu.eg; Abdelhalim.azam@mans.edu.eg

<sup>1</sup> School of Civil Engineering, University Teknologi Malaysia, Johor Bahru, 81300 Johor, Malaysia

<sup>2</sup> Department of Civil Engineering, Kebbi State University of Science and Technology Aliero, Kebbi State, Aliero, Nigeria

<sup>3</sup> Department of Civil Engineering, College of Engineering, Jouf University, Sakaka 72388, Saudi Arabia

<sup>4</sup> Civil Engineering Department, College of Engineering, University Malaysia Pahang, Pekan, Malaysia

<sup>5</sup> Department of Civil and Environmental Engineering, College of Engineering, Majmaah University, Al-Majmaah 11952, Saudi Arabia

<sup>6</sup> Engineering Management Department, College of Engineering, Prince Sultan University, 11586 Riyadh, Saudi Arabia

<sup>7</sup> Public Works Engineering Department, Faculty of Engineering, Mansoura University, Mansoura 35516, Egypt

Natural rubber (NR) has been used to improve bitumen performance for almost a century; in fact, the use of NR for bitumen modification was patented in England in 1840. In 1920, the United Kingdom and the United States made several attempts to construct rubberized roadways without the use of bitumen [1]. NR is a renewable, economic elastomer with outstanding thermal stability and elasticity. Its application in bitumen modification increases the rutting resistance, fatigue life, dynamic stability, resistance to moisture damage, and low-temperature cracking resistance of a pavement [2–4]. Currently, Malaysia is committed to increasing NR consumption and increasing the price of NR through road construction. More than 1000 kms of cup lump-modified asphalt (CMA) roads were built to increase NR domestic consumption. The project consumed about 42,000 metric tons of NR [5]. Cup lump rubber (CLR) is a coagulated NR that has not gone through any production processes [6]. The use of CLR-modified bitumen (CMB) in road construction gained popularity after CMA roads were constructed in Malaysia in 2015. Laboratory and field results obtained after two years of monitoring show improvements in resilience and durability, rutting resistance, and cracking performance compared to the conventional HMA [7]. Despite the CMA's excellent performance, its use in HMA raised the cost of asphalt manufacturing by 80% due to the higher amount of heating energy required during the mixing and compaction of the CMA. According to [8, 9], the high cost of rubberized asphalt pavements is due to the increased energy consumption when compared with the standard asphalt mix. The authors found that increasing the asphalt manufacturing temperature directly leads to a corresponding increase in the quantity of total energy consumed, thereby resulting in a higher asphalt production cost. Therefore, the conversion of CMA to warm mix asphalt (WMA) is anticipated to substantially lower the mixing and compaction temperatures, cost, and fume emissions without affecting the in-service performance of the CMA mix. WMA is a technology that reduces HMA production temperature by up to 50 °C by adding chemicals that reduce the viscosity of the bitumen or increase its volume, thereby facilitating aggregate coating to be achieved at a lower temperature than HMA [8–11].

Despite the several advantages of WMA over HMA, little is known about the morphology of NR-modified bitumen with warm mix additives due to earlier research focusing on HMA [12]. Asphalt binders are typically subjected to tests in one of the three categories: physical, rheological, and morphological. Physical tests are empirical procedures used to ascertain the consistency of the binder. Because bitumen is viscoelastic, rheological tests are carried out to determine the binder's resistance to flow. Morphological tests are used to analyze the interaction between bitumen and different bitumen modifiers. Morphological tests aid in explaining or understanding the bitumen's rheological results and the

mechanical performance test results of the asphalt mixture. The literature matrix presented in Table 1 examined the application of NR-modified bitumen; it confirms the lack of sufficient literature investigating the application of CLR-modified bitumen with Evotherm warm mix additive. Only Sani et al. [15, 16] analyzed the properties of NR latex-modified binders with Zycotherm warm mix additive. They discovered that NR latex-modified binders have a disjointed agglomerative structure with long voids, indicating high a moisture content that tends to lower the shear stress resistance of the binder. NR latex contains more than 40% water content, and its blending with bitumen produces foam in large quantities which generates concerns about the moisture damage susceptibility of the binder. As a result, using CLR with lower water content will be more beneficial in improving bitumen performance. Abdulrahman et al. [13, 14] investigated the physical properties and mechanical performance of cup lump rubber-modified bitumen in another study. They discovered that the WMA had outstanding rutting resistance (rut depths of less than 2 mm at 8000 load cycles), which represents a 21% improvement over the conventional HMA. In addition, moisture damage analysis revealed that the WMA has a tensile strength ratio (TSR) of 99 percent and the aggregates preserved over 90 percent of the bitumen coating. The production of CMA at a lower temperature reduced the CO<sub>2</sub> by 5 kg, resulting in a 23% reduction in the global warming potential of the asphalt mixture. Despite its enormous success, the rheology and morphology of the Evotherm-modified bitumen remained unknown. Hence, the current study intends to fill this research gap. The present study explored the properties of CLR-modified bitumen incorporating Evotherm warm mix chemical. The first step toward achieving long-term use of NR in pavement construction is determining the binder properties. Also, because of the complicated interactions that exist between the combination of bitumen, rubber, and WMA additives, characterization of the binders first is crucial before applying them at the mix design level [15]. This research becomes necessary since bitumen modification with NR increases viscosity, which necessitates greater mixing and compaction temperatures during the asphalt manufacturing process. As a result, an additional 2.5 kg of CO<sub>2</sub> per ton is produced for society, as well as CO, NO<sub>x</sub>, SO<sub>2</sub>, and other environmental contaminants. Likewise, it consumes more heating energy (0.5–1.0 L/ton) thus raising the entire asphalt production cost [16, 17]. As such, the asphalt production industry should reduce the production temperature of HMA without affecting pavement performance [18]. The outcome of this study will provide clearer knowledge on the effects of bitumen modification with Evotherm and promote WMA in the asphalt industry.

**Table 1** Literature matrix summarizing the application of natural rubber-modified binder

Author	Natural rubber type	Bitumen properties			Mixture Performance				
		Physical	Rheology	Characterization	Rutting	Moisture damage	Fatigue cracking	Thermal cracking	
Abdulrahman et al. [13, 14]	Cup lump	✓	Gap to be filled by the current research		✓	✓	✓	Research Area yet to be fully explored	
Ghafar et al. [19, 20]		✗	✗	✗					
Mazlina Mustafa Kamal et al. [21]		✓	✓						
Hazoor Ansari et al. [22]		✗	✗	✗					
Azahar et al. [23–25]		✗	✗	✗		✗	✗		
Razali et al. [26]						✗	✗		
Othman et al. [5]		✗	✗			✗			
Shaffie et al. [27]						✗			
Sani et al. [28, 29]		Latex	✓	✓	✓				
Abu Bakar et al. [30]			✗	✗					
Siswanto [4]						✗			
Shafii et al. [31]			✗						
Shaffie et al. [2]						✗			
Siswanto [32]						✗			
Krishnapriya [33]			✗	✗		✗	✗		✗
Wen et al. [34]		✗	✗	✗					
Shaffie et al. [3]						✗			
Tuntiworawit et al. [6]		✗	✗	✗	✗		✗		
Tinavallie [35]		✗		✗					

✓ = With warm mix additive ✗ = Without warm mix additive

## 2 Materials and methods

### 2.1 Materials

Cup lump rubber-modified bitumen (CMB) produced in the laboratory was modified with five percentages of Evotherm in this study. Table 2 summarizes the properties of the CMB used and shows that the CMB has properties similar to PG82. The chemical Evotherm, made by Ingevity in the United States, was utilized as a surfactant warm mix additive. It

**Table 2** Properties of the cup lump rubber-modified bitumen (CMB)

Method	Test	Value
ASTM D5	Penetration at 25 °C (dmm)	36
ASTM D36	Softening point (°C)	54
ASTM D4402	Viscosity at 135 °C (Pa s)	1.9
ASTM D113	Ductility (cm)	94
ASTM D6084	Elastic recovery (%)	31



**Fig. 1** Evootherm warm mix additive used for this research

is an oily, dark liquid that is just slightly water-soluble, as shown in Fig. 1. The parameters of the Evootherm employed in this investigation are listed in Table 3.

## 2.2 Bitumen preparation

First, CMB is produced by blending 60/70 penetration grade bitumen with five percent cup lump rubber (in gel form) using a high shear mixer (at 4000 revolutions per minute) at  $160 \pm 10$  °C for two hours. More detail on the process of converting solid rubber into gel is reported in [14]. A storage stability test in accordance with the ASTM D7173 standard was conducted to examine the tendency of separation; the result shows that the CMB is stable (the difference between the top and bottom softening temperatures is 1 °C). In the second stage, the CMB is modified with 0.3, 0.4, 0.5, 0.6, and 0.75% of Evootherm using a high shear mixer at 160 °C for five minutes. The Evootherm dosage of (0.3–0.75%) and blending time were selected based on the manufacturer's recommendation. Figure 2 presents the experimental flowchart used for the study. Physical, rheological, and morphological tests were conducted to characterize and compare the properties of the modified binders at high, intermediate, and low temperatures. For each test conducted in this study, at least

three replicate samples were produced and tested, and the average value is reported. Error bars are provided to show the variability of the data obtained.

## 2.3 Experimental methods

### 2.3.1 Penetration and softening point

A penetration test is performed to determine the consistency of a binder by applying a load of 100 g from a standard needle into a mold filled with a bitumen specimen in a water bath maintained at 25 °C for 5 s [36]. A softening point test was conducted to identify the transition temperature of bitumen from a semi-solid state to a semi-liquid. At the transition temperature, the bitumen can no longer maintain its mass and begins to deform. The test was carried out in accordance with ASTM D36 [37].

### 2.3.2 Ductility and elastic recovery

In the ductility test, a briquet of bitumen sample is pulled at 50 cm/min in a  $25 \pm 5$  °C water bath. The ductility value is the length the material elongates before breaking [38]. One drawback of this test is that it only examines the bitumen's capacity to stretch but not its ability to rebound. As a result, an elastic recovery test was performed in line with ASTM D6084 to assess the binders' capacity to recover after deformation [39]. Figure 3 presents an image of the prepared sample.

### 2.3.3 Viscosity

The modified binders were put through a viscosity test to guarantee that they could be blended easily at production and construction temperatures. The test was carried out using a Brookfield rotating viscometer (DV-II + Pro) and test specimens were prepared according to ASTM 4402.

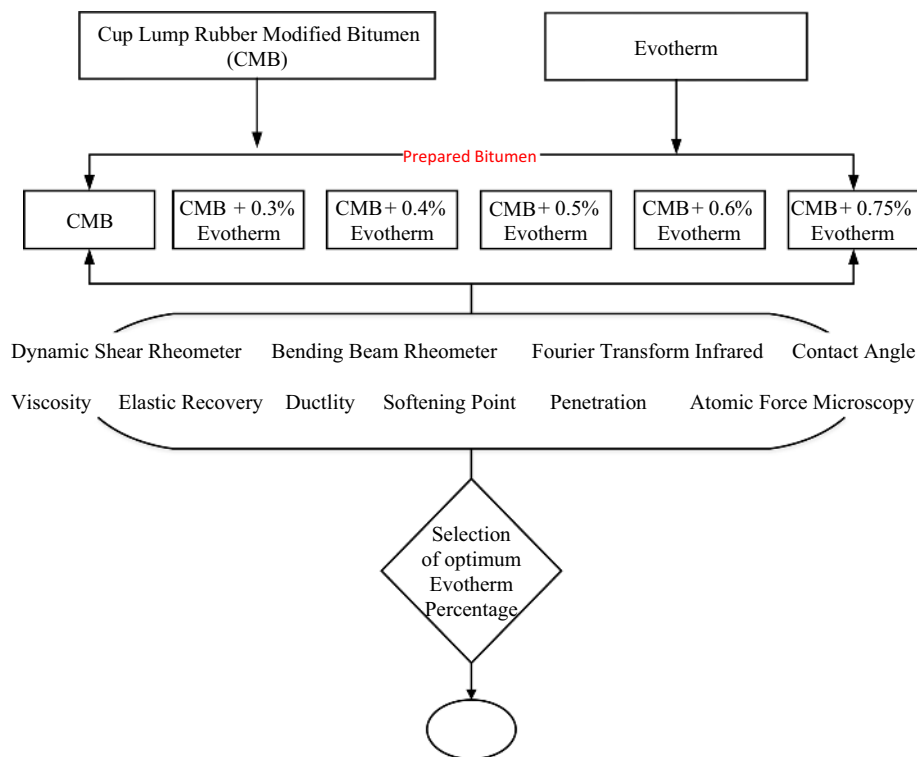
### 2.3.4 Dynamic shear rheometer

The bitumen stiffness, viscoelastic behavior, and resistance to deformation under load are determined using a dynamic shear rheometer (DSR) at 46–88 °C different pavement temperatures and a loading rate of 10 rad/s (1.59 Hz) [40, 41]. In this test, a sample of bitumen is inserted between the geometries of a HAAKE rheometer and subjected to a sinusoidal

**Table 3** Physical and chemical properties of Evootherm [14]

Physical state	Color	Odor	Specific gravity	pH values	Boiling point	Flashpoint (closed cup)
Viscous oil	Dark brown	Amine-like	1.03–1.08	10–12	> 200 °C	> 260 °C

**Fig. 2** The study’s experimental flowchart



**Fig. 3** Cup lump rubber-modified sample prepared for ductility test

strain. The applied strain’s reaction is the creation of matching sinusoidal stress, but the material’s response is out of phase with the applied strain, referred to as the phase angle ( $\delta$ ). This test revealed two properties of bitumen: rutting and fatigue resistance [42, 43].

**2.3.5 Bending beam rheometer**

The probability of crack development increases when pavement is exposed to low pavement temperatures due to the bitumen’s transition from a time-dependent ductile to a brittle behavior [44]. The creep stiffness behavior of bitumen

may be obtained using a bending beam rheometer (BBR) test. BBR test was conducted in line with the ASTM 6818 standard [45]. The *m*-value and creep stiffness of the bitumen are determined after a loading duration of 60 s [42, 46].

**2.3.6 Fourier transform infrared spectroscopy**

Using attenuated total reflectance-Fourier transform infrared (ATR-FTIR) spectroscopy, the chemical alterations in the modified bitumen were examined. To investigate the oxidative aging of asphalt binders, carbonyl (C=O) and sulfoxide (S=O) absorption, as well as carbon-carbon (C=C) stretching vibrations of the aromatic ring, are all measured [43, 44]. The region covered by the peaks (from valley to valley) was used for quantitative analysis of the IR spectra using TQ Analyst software [45, 46]. Carbonyl oxidation ( $A_{1700}/A$ ), aromaticity index ( $A_{1600}/A$ ), aliphatic ( $A_{1452} + A_{1373}/A$ ), presence of a sulfoxide group ( $A_{1030}/A$ ), aromatic heteroatoms ( $A_{874} + A_{809} + A_{747}/A$ ), and lengthy chains ( $A_{720}/A_{1452} + A_{1373}/A$ ) are among them. The total area for each IR spectra is calculated using Eq. 1.

$$\Sigma A = A_{2950} + A_{2860} + A_{1700} + A_{1600} + A_{1452} + A_{1373} + A_{1030} + A_{874} + A_{809} + A_{747} + A_{720} \quad (1)$$



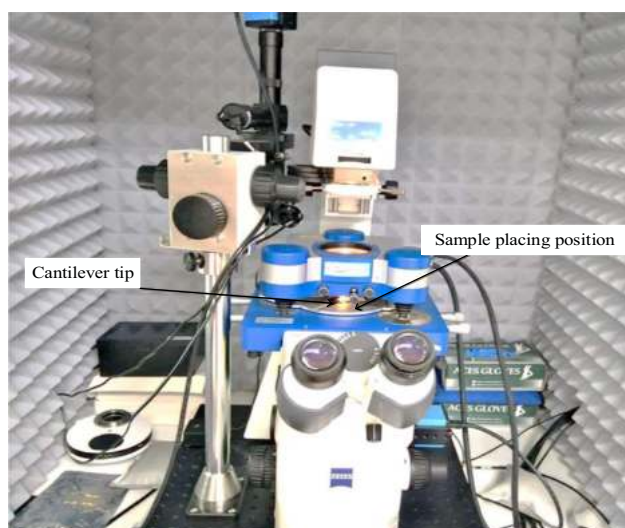


Fig. 4 Atomic force microscopy test setup

### 2.3.7 Atomic force microscopy

The atomic force microscopy (AFM) test is used to determine the forces generated between a cantilever beam's tip and the material being tested as a function of the distance between the two structures [47]. Test samples were made by heating bitumen to 135 degrees Celsius until it became fluid enough. A bitumen sample of around 2 mm diameter is dropped on a clean microscope glass slide, which is then returned to the oven at 135 °C for 5 min to provide a flat surface [48]. During testing, the system generated a two- and three-dimensional surface profiles of the bitumen specimen using the tiny cantilever tip (in a non-contact mode of operation). The phase matrix included in the tested specimen is depicted in 2D, while the three-dimensional topography (3D) offers data on the relative heights of the various characteristics present in the bitumen sample [49]. Figure 4 presents the atomic force microscopy test setup.

### 2.3.8 Contact angle

One of the major issues with WMA's durability is the possibility of moisture-induced bond breakdown. Because of the lower mixing temperature, a thin water layer forms at the aggregate—bitumen point of contact. Failures at this critical contact are most likely caused by the existence of water in the aggregates or outside the bitumen coated particle [50–52]. As a result, a robust adhesion bond connecting the aggregate and the bitumen is critical. Anything that compromises this bond will shorten the pavement's intended life span. The addition of warm mix additive to bitumen can change the binder's adhesive properties. Hence, it is important to look at the binders' ability to attract moisture at the bitumen-aggregate

interface, which tends to lower the bitumen-aggregate adhesion bond [51, 52]. Samples for the contact angle test were prepared by heating the bitumen in a conventional oven until it turns to liquid, which is then applied to a 3 cm × 5 cm piece of tiny glass. A micro-syringe needle is used to release a sessile drop of distilled water from a height of 5 mm. Then, a video contact angle measuring device (VCA optima) was employed to capture the sessile drop image, and the left and right contact angles were determined. Five replicate samples were prepared for each of the bitumen produced in the study. The repeatability of the measurement was estimated using a single test method, and the results show that the variability ranges from 1.38 to 2.59, which is within the recommended limit of  $\pm 5^\circ$  [53, 54]. Figure 5 shows the contact angle measurement process.

## 3 Results and discussion

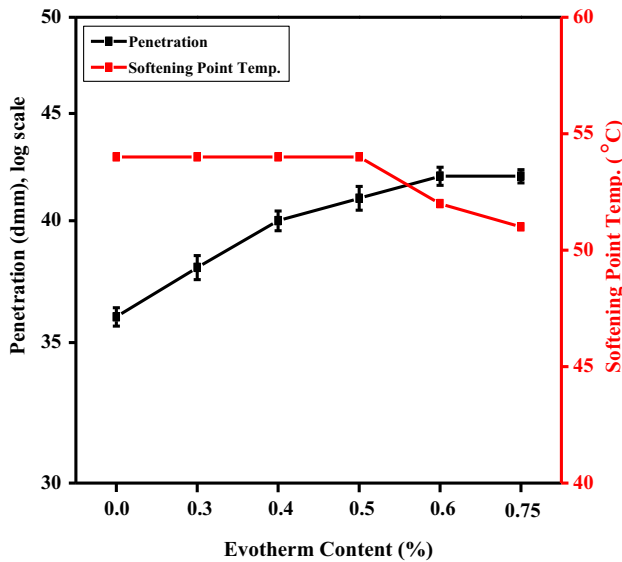
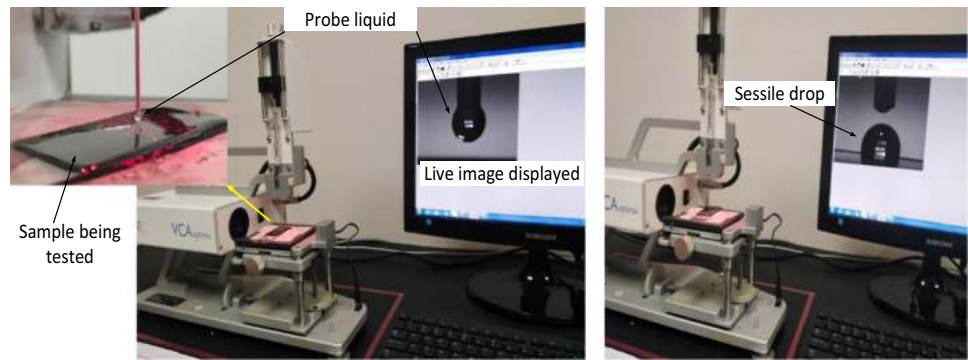
### 3.1 Penetration and softening point

The addition of 0.3–0.75% Evotherm continuously increased the penetration amount of the CMB, as depicted in Fig. 6, while a marginal decrease in softening point temperature was observed at 0.6–0.75% Evotherm content. Because the Evotherm tends to dilute the CMB, resulting in the formation of bitumen with a softer matrix, an increase in penetration value with decreasing softening point temperature is expected. This can be attributed to the lower melting and boiling point components of the Evotherm additive [55]. It was observed that the softening temperature was not affected by the addition of 0.3–0.5% Evotherm content. This is because during the blending of CMB with Evotherm, rubber particles tend to absorb the freely available Evotherm oil and cause swelling until the saturation limit is reached at 0.5% Evotherm content [56]. Furthermore, thermal analysis of NR using a differential scanning calorimeter reveals that the first endothermic peak and mass loss at 50 °C are caused by water evaporation from the NR cavities, and flowable bitumen constituents (saturates and aromatics) permeate and fill the pores left in the NR [57]. This interaction occurs at a temperature similar to the softening point testing temperature (50–60 °C); as such, Evotherm oil flows and fills the pores, hence the reason for the lack of decrease in the softening temperature of the bitumen at 0.3–0.5% Evotherm content. But adding 0.6 and 0.75% Evotherm causes the softening point temperature to decrease, signifying that the pores have reached saturation level.

### 3.2 Ductility and elastic recovery

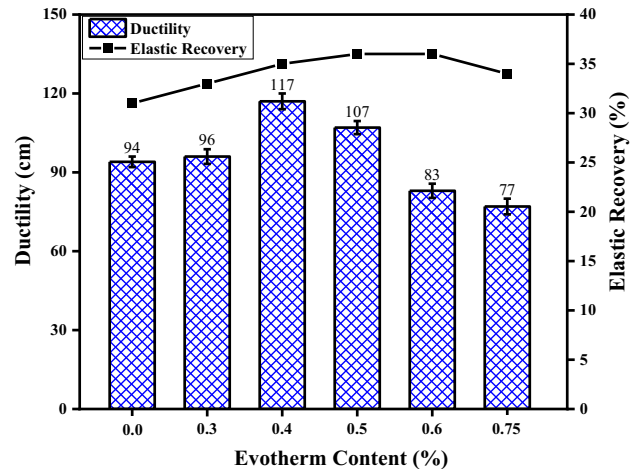
The results of the ductility and elastic recovery tests are shown in Fig. 7. First, it is worthy of mention that the base

**Fig. 5** Contact angle test setup used in measuring the affinity of the bitumen



**Fig. 6** Penetration and softening point temperature test result

60/70 PEN bitumen used in this research has a ductility value 120 cm. The addition of 5.0% CLR reduced the ductility value to 94 cm. This can be attributed to the absorption of light oils by the CLR from the bitumen as a result of the bitumen–rubber interaction during the blending process [56]. Hence, the addition of rubber leads to the production of stiff bitumen with less stretching ability. The addition of 0.3 to 0.5 percent Evotherm shows an increase in the ductility value, indicating an improvement in the stretching ability of the CMB. At 0.6 and 0.75% Evotherm content, the ductility decreased. This demonstrates that a lower Evotherm dosage increased CMB stretching ability by lowering stiffness, whereas a higher dosage reduced the stretching ability of the CMB. A similar increase in ductility with the addition of Evotherm was reported by [58]. This is because the blending of conventional bitumen with CLR stiffens the bitumen, leading to improved high-temperature performance, but renders the bitumen brittle at 25 °C testing temperature due to a reduction in cohesion [22, 56, 59]. Thus, incorporating Evotherm into the blend tends to reduce the stiffness of the



**Fig. 7** Ductility and elastic recovery result

CMB (by replenishing the light oils absorbed by the CLR) until full saturation level is attained at 0.5% Evotherm content. Similarly, the elastic recovery curve showed a tendency in which increasing Evotherm concentration improves the CMB’s recoverability until a maximum value is reached at 0.6 percent Evotherm. According to [60], a bitumen should have at least 60% elastic recovery; however, none of the tested binders met this requirement.

### 3.3 Viscosity

Figure 8 presents the viscosity test results. It demonstrates that adding 0.3 percent Evotherm to CMB reduced viscosity by 10% at 135 °C, whereas increasing Evotherm dosage to 0.4 and 0.5 percent results in a four percent additional reduction in viscosity. A cumulative reduction of 26% in viscosity was achieved at 0.75% Evotherm content, indicating that it is possible to use lower mixing and compaction temperatures for the CMB asphalt mixture. The viscosity of the CMB decreased by 11% when the testing temperature was raised to 165 °C, regardless of the Evotherm concentration. This is because the warm mix additive functions as a surfactant by lowering the surface tension between aggregate and bitumen. As such, its influence on viscosity is sometimes undetectable

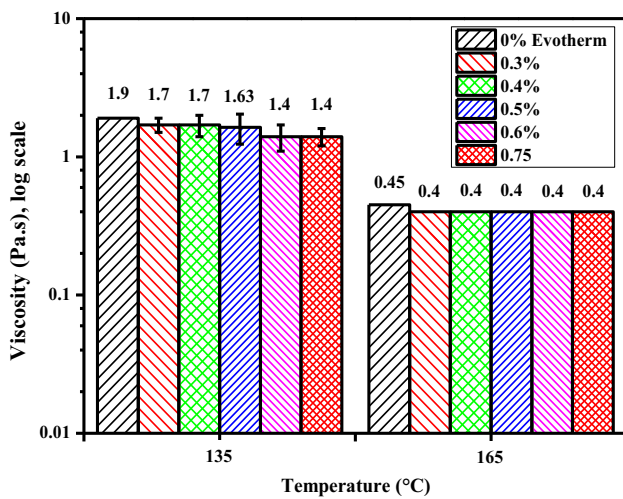


Fig. 8 Viscosity result of CMB with various percentages of Evotherm

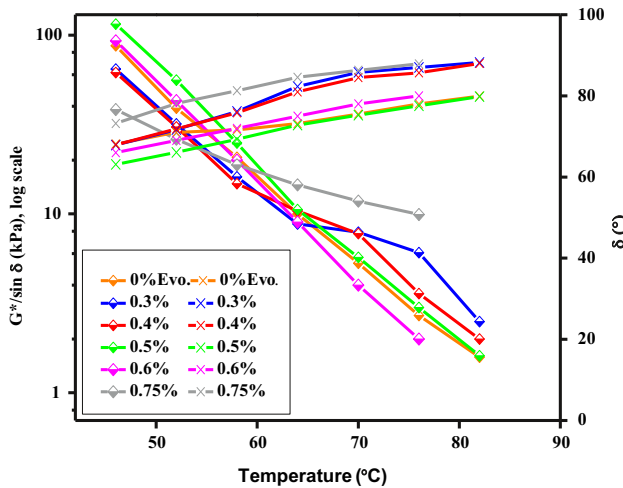


Fig. 9 Isochronal graph of rutting resistance and phase angle at a constant rate of recurrence of 1.59 Hz

at 165 °C [55, 61–63]. A similar slight decrease in viscosity was obtained by [64–66].

### 3.4 Dynamic shear rheometer

Figure 9 shows the isochronal graphs of rutting resistance and phase angles of the tested binders. The specification limit of  $|G^*|/\sin(\delta) = 1$  kPa was employed to establish the bitumen’s rutting resistance [67]. It shows that the CMB has a rutting resistance of 82 °C which is similar to the rutting resistance of the bitumen with 0.3 to 0.5% Evotherm. Meaning the addition of 0.3–0.5% Evotherm did not affect the rutting resistance of the CMB. This can be attributed to the low Evotherm dosage incorporated into the blend (3 to 5 g for every 1000 g of CMB). However, at 0.6 percent and 0.75 percent Evotherm content, rutting resistance decreased by one

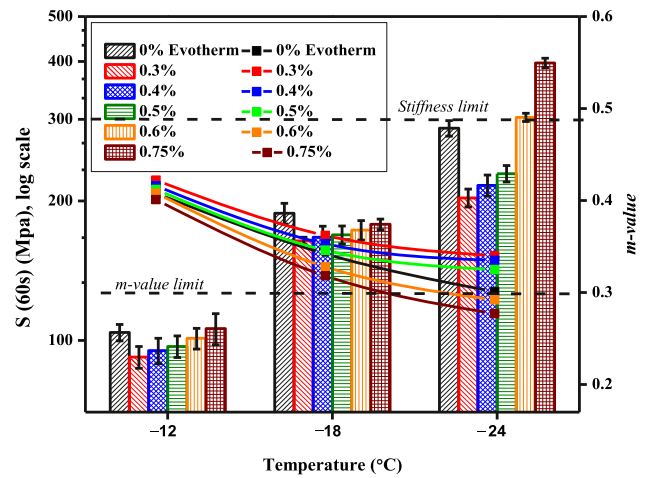


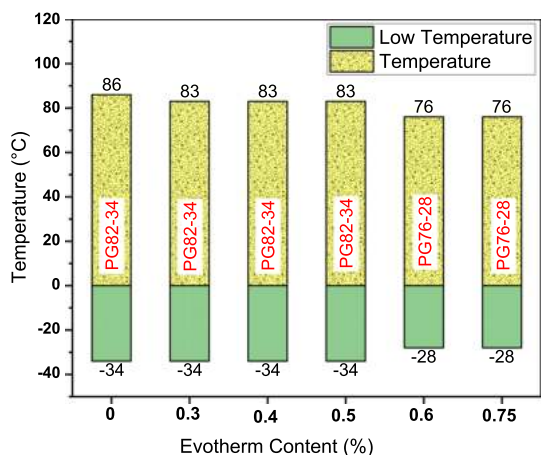
Fig. 10 Binders low-temperature cracking resistance

grade. This is due to the high Evotherm content, which tends to dilute and reduce the viscosity of the CMB. Similar finding was reported by [58]. On the contrary, [65] discovered that Evotherm raised the failure temperature of polymer-modified bitumen. A similar trend was observed in the phase angle curves, where Evotherm marginally reduced the elastic response of the CMB. As a result, the Evotherm modification has no significant negative impact on the binders’ high-temperature performance.

### 3.5 Bending beam rheometer

Figure 10 illustrates the findings from the BBR test. *m*-value denotes the ability of bitumen to dissipate applied stress via plastic flow, whereas creep stiffness (*S*) denotes the bitumen’s stiffness at low temperatures. Preferably, the bitumen should be less stiff (lower creep stiffness) and capable of quickly releasing stress for better low-temperature performance (higher *m*-value) [68]. Also, the tested binder must satisfy the AASHTO M320 specification limits of  $S \leq 300$  MPa and  $m\text{-value} \geq 0.3$ . The figure shows that the CMB (0% Evotherm) has a creep stiffness of 104 MPa at – 12 °C, the value increased to 188 MPa and 287 MPa – 18 °C and – 24 °C testing temperatures, respectively. The addition of 0.3 percent Evotherm at – 12 °C results in a 12 percent reduction in CMB stiffness, and the deflection of the beam increases by 40%, which is logical due to the Evotherm’s oily nature, which tends to physically diminish the binder’s hardness. At 0.4 and 0.5 percent Evotherm concentration, the creep stiffness decreased in a similar way. When the temperature was raised to – 18 °C, the Evotherm-modified bitumen showed a steady rise in creep stiffness with a lower *m*-value when compared to the 0% Evotherm bitumen. A similar stable trend was observed at – 24 °C (excluding the 0.6% and 0.75% Evotherm), which indicates that the incorporation





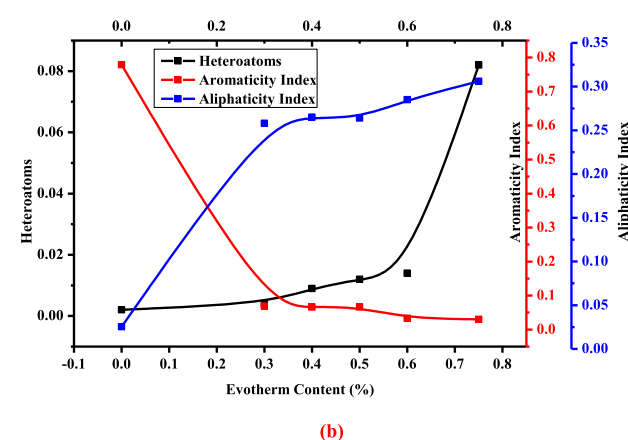
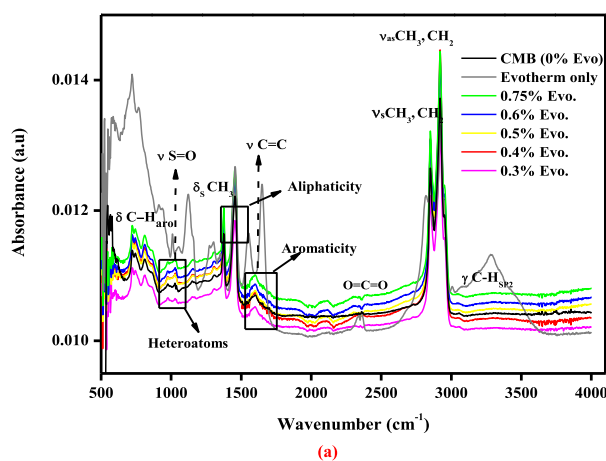
**Fig. 11** High- and low-temperature limits of cup lump rubber-modified bitumen added with Evotherm

of Evotherm to the blend retards the rate of creep stiffness increase at low pavement temperatures. Reduced creep stiffness implies that the binder’s elasticity has improved, and is expected to resist the formation of microcracks at low temperatures [69]. Moreover, the high *m*-value result indicates that Evotherm significantly improved the stress dissipation property of the CMB. At  $-24\text{ }^{\circ}\text{C}$ , the 0.3, 0.4, and 0.5 percent Evotherm-modified binders met the *S* and *m*-value criteria and were classified as PG82-34. However, more testing at a much lower temperature is required until failure occurs. While the bitumen with 0.6% and 0.75% Evotherm did not meet the *S*(60 s) and *m*-value requirements at  $-24\text{ }^{\circ}\text{C}$  testing temperature, they were classified as PG7618. Essentially, modifying the CMB with a little Evotherm lowered the creep stiffness and improved the CMB’s stress release capabilities. Figure 11 summarized the performance grades of the tested bitumen at high and low temperatures.

### 3.6 Fourier transform infrared (FTIR)

Figure 12a depicts the changes in the Evotherm-modified bitumen’s infrared spectrum. In addition, the FTIR of Evotherm alone was presented to show a specific footprint. Visual inspection of the IR shows that the Evotherm-modified binders display similar absorbance to the CMB (0% Evotherm). Meaning that adding Evotherm marginally affect the primary functional set of compounds present in CMB. Furthermore, the lack of a peak at  $1700\text{ cm}^{-1}$  (carbonyl absorption) indicates that no oxidation occurred throughout the modification process.

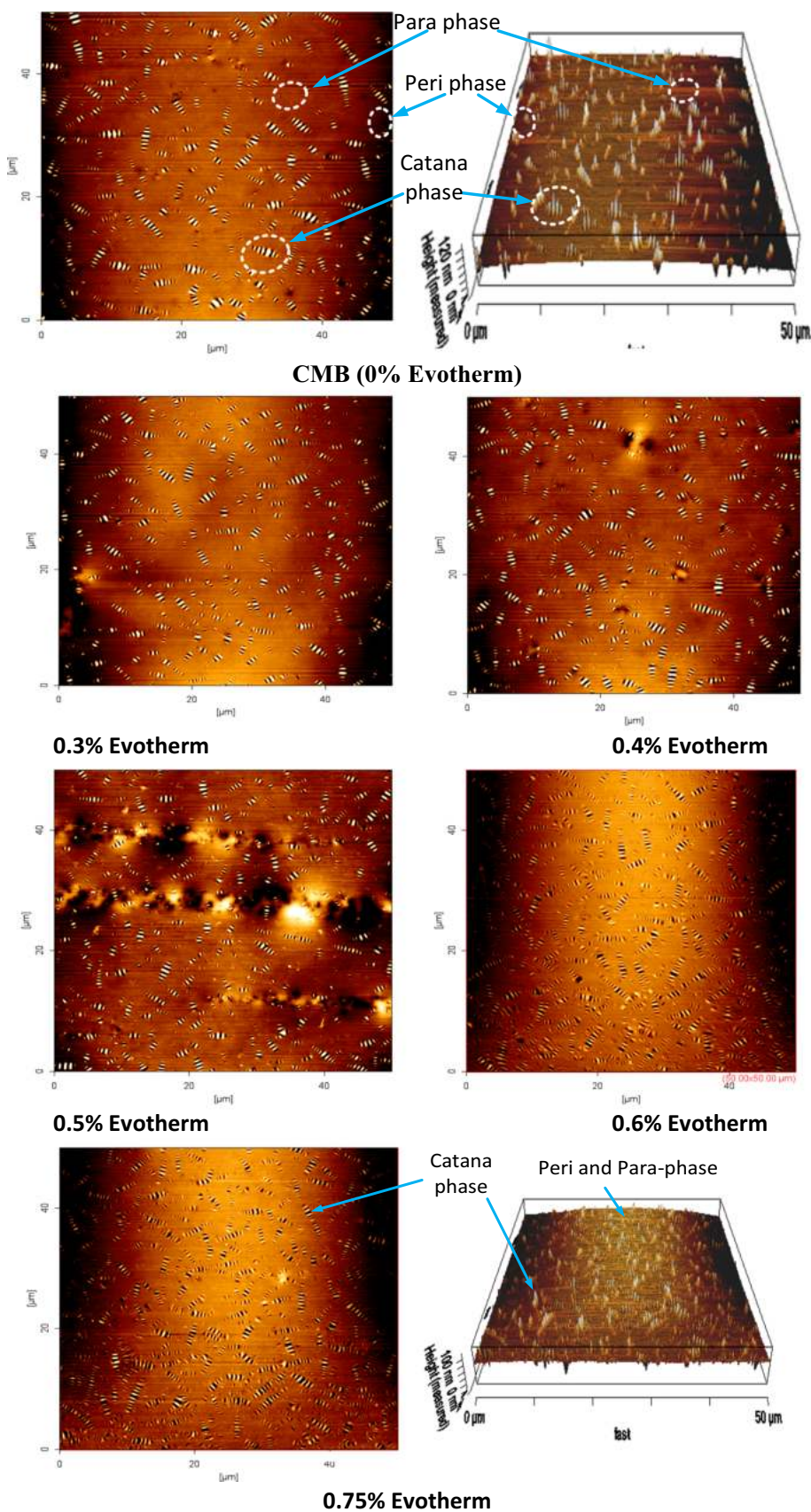
However, significant changes were detected at wavenumbers ranging from  $600\text{ to }2000\text{ cm}^{-1}$ . Cis 1.4-polyisoprene absorption corresponding to  $=\text{C}-\text{H}$  out of plane bending was detected at  $842\text{ cm}^{-1}$ , while a strong out of plane absorption band belonging to the toluene group was detected at



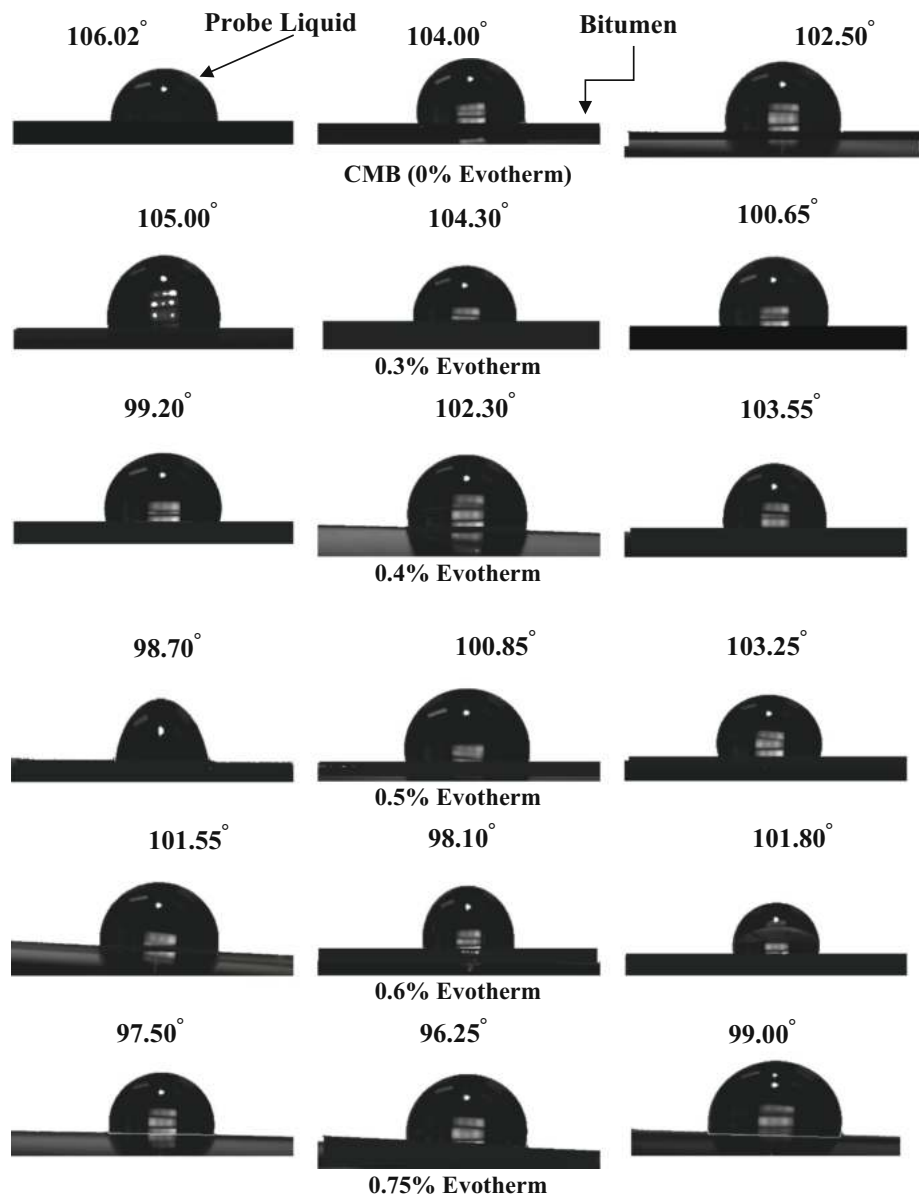
**Fig. 12 a** Evotherm-modified bitumen Fourier transform infrared spectra. **b** Changes in heteroatoms, aromaticity, and aliphaticity index in response to Evotherm modification

$619\text{--}775\text{ cm}^{-1}$ . Figure 12b also displays the quantitative examination of the spectra, it indicates a decrease in absorption at  $1600, 1030, \text{ and } 720\text{ cm}^{-1}$  wavenumbers, which describe the  $\text{C}=\text{C}, \text{C}-\text{O}, \text{ and } \text{C}-\text{H}$  stretch. The decrease in absorption is attributed to the decrease in density of  $\text{C}-\text{H}, \text{C}-\text{O}, \text{ and } \text{C}=\text{C}$  stretch in response to the addition of the Evotherm [65, 70]. The decrease in aromaticity index at  $1600, 900, 810, \text{ and } 720$  wavenumbers decreased with increasing Evotherm content, which confirms the decrease in density reduction. This shows that the Evotherm-modified bitumen has a less compact structure as well as less mobility and elasticity compared to the CMB. The structure becomes less rigid as the Evotherm dosage increases, as evidenced by lower viscosity values of the Evotherm-modified binders and an increase in the aliphaticity index ( $A_{1456} + A_{1378}/A$ ). In conclusion, the FTIR bands of the "Evotherm-modified binders" are comparable to those of CMB without Evotherm.

**Fig. 13** Atomic force microscope scan of Evotherm-modified bitumen



**Fig. 14** Effect of Evotherm modification on contact angle. The average contact angle value decreases with increasing Evotherm content



### 3.7 Atomic force microscopy

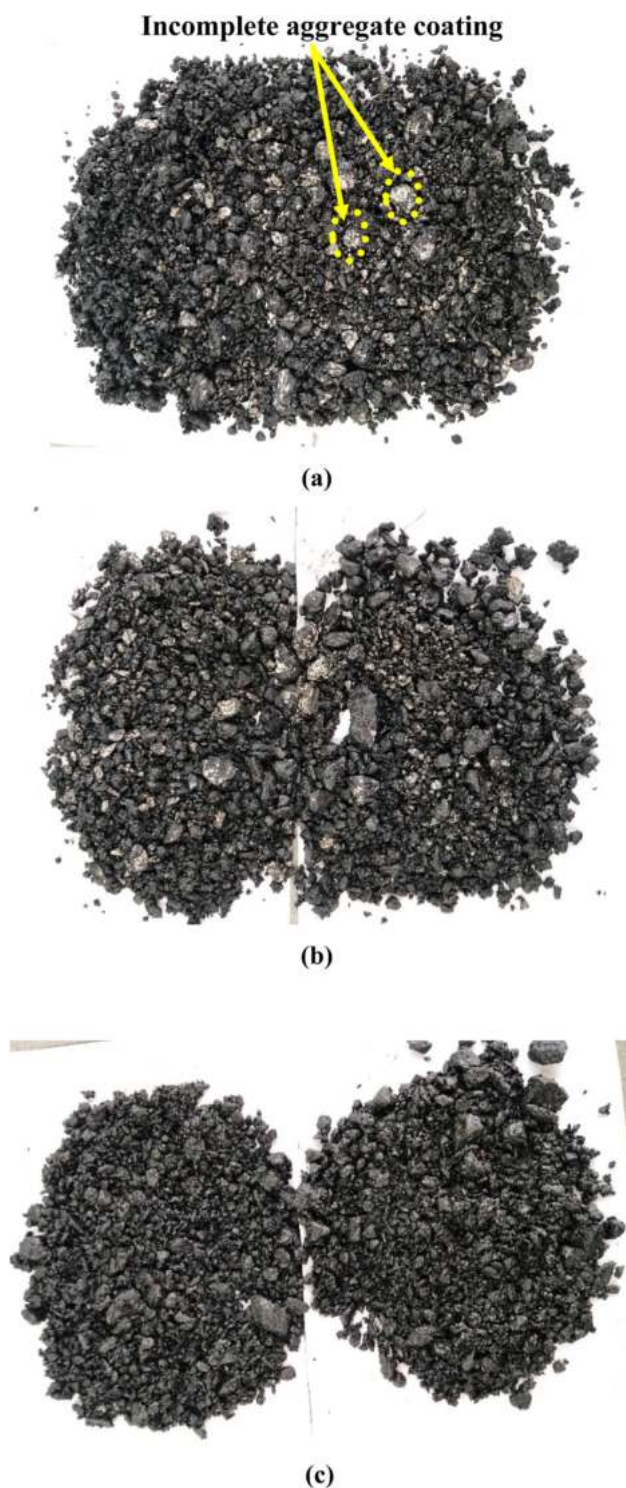
Figure 13 depicts a scanning picture of the modified bitumen at  $50 \times 50$  nm. In AFM test results, a three-phase network structure is commonly observed. They are the Catana phase (bee-structure), the peri-, and the para-phases [71]. The para-phase has a low stiffness and high cohesiveness, whereas the peri- and catana phases have a high stiffness (low cohesion) [72, 73]. A scanning image of modified bitumen shows that the Catana phase is more noticeable, whereas the peri- and para-phases are nearly indistinguishable. This is because the bitumen source and origin of the base binder determine its morphology at the microscale level. At 0.3–0.4% Evotherm modification, no significant changes in the catana, peri- and para-phases were observed. But at 0.6 and 0.75% Evotherm

content, an increase in the number with a decrease in the size of the catana phase was detected, indicating a decrease in binder stiffness. Changes in the shape and size of the catana phase indicate changes in the molecular structure of the bitumen [74]. The lack of clear separation of the peri- and para-phases makes it difficult to examine the changes in bitumen cohesion and adhesion properties; this could open a new line of research into finding another technique for measuring these properties with reliability.

### 3.8 Contact angle

Figure 14 presents the results of the contact angle measurement carried out to examine the affinity of the modified bitumen to water. Bitumen is either hydrophilic (attracts





**Fig. 15** Selection of optimum Evotherm content for reducing the mixing temperature of cup lump-modified asphalt with Evotherm. **a** After mixing at 130 °C with 0.5 percent Evotherm, white dots signify an incomplete aggregate coating. **b** When mixed with 0.5 percent Evotherm at 140 °C, uncoated aggregate is visible. **c** Complete aggregate coating when mixed at 140°C with 0.75% Evotherm

water) or hydrophobic (rejects water and has less susceptibility to moisture-induced failure) [52, 75]. On the CMB (0% Evotherm), the distilled water forms an average angle of 104° and can be termed hydrophobic (contact angle greater than 90°) and tends to repel the dropped water on the bitumen surface. The addition of 0.3–0.75% Evotherm reduced the average contact angle of the CMB from 104° to 103°, 102°, 101°, 100°, and 98°. This implies that Evotherm modification decreased the hydrophobicity of the CMB as Evotherm content increased. Meaning there is an increase in the wettability of the dropped water on the surface of the Evotherm-modified bitumen, which may weaken the bond at the bitumen-aggregate interface. The spreading of water on the bitumen surface leads to fewer points of contact between the aggregate and the bitumen. The results of this test indicate that adding warm mix additives to bitumen reduces its water-repelling performance. Previous laboratory research has linked WMA's moisture sensitivity to insufficient aggregate drying; however, the current findings imply that bitumen's water-repelling power may also play a role in WMA's moisture susceptibility. The biggest issue with WMA's durability is the possibility of moisture-induced bond breakdown, which is caused by the lower mixing temperatures resulting in the formation of a thin water film between the aggregate and the bitumen surface [76]. Thus, the existence of water in the aggregates or outside the bitumen coated aggregate is a leading cause of asphalt moisture damage failure at this critical interface [52].

### 3.9 Selection of optimum Evotherm content and production temperature for asphalt mix production

This section presents the process of selecting the optimum percentage of Evotherm suitable for asphalt mix production using aggregate coating and compactability testing (as recommended by the manufacturer of Evotherm). The chosen approach complies with the AASHTO R35 criteria and is practiced by [14, 77]. It is a trial-and-error method where asphalt mixtures are prepared by varying the Evotherm content (0.3–0.75%) and at various mixing and compaction temperatures. Observations are then made to identify the mixture that has a 100% aggregate coating and sufficient air voids between 3–5%. The aggregate coating test is summarized in Fig. 15a–c. The first trial mix was prepared with the CMB + 0.5% Evotherm content at mixing and compaction temperatures of 130 and 120 °C, respectively. The loose mixture shows less than 100% aggregate coating with more than 5% air void, indicating the need to increase the mixing temperature or the Evotherm content (Fig. 15a). Another mixture was made by raising the mixing and compaction temperature to 140 and 130 °C, respectively, while keeping the Evotherm dosage at 0.5 percent; this resulted in

a minor increase in aggregate coating but not the desired 100% (Fig. 15b). Another sample was made by raising the Evotherm value to 0.75 percent while the mixing temperature was maintained; this resulted in a 100 percent aggregate coating and 3–5% air void (Fig. 15c). As a result, 0.75% Evotherm content was chosen as the best dose for making the CMA mixture in WMA.

## 4 Conclusion

This study looked into the effect of Evotherm modification on the rheology and morphology of cup lump rubber-modified bitumen. It was discovered that adding Evotherm to CMB slightly reduced its viscosity. A lower percentage of Evotherm (0.3–0.5%) has no effect on CMB rutting resistance, whereas a higher percentage (above 0.5%) reduces rutting resistance by one grade at high temperature. The CMB's cracking resistance at low temperatures increased after the addition of 0.3–0.5 percent Evotherm to the blend. Furthermore, the FTIR results show that incorporating Evotherm lowered the absorption of double and single bonds, but the primary functional groups remained unaffected. The be-structure is clearly dispersed in the AFM scan, although the para-phase and peri-phase are not clearly separated. Contact angle measurement revealed that the inclusion of Evotherm marginally lowered the CMB's water-repelling performance. According to the aggregate coating test results, 0.75% Evotherm content is the optimum dosage to be used in lowering the manufacturing temperature of cup lump-modified asphalt. Finally, the rheology and morphology of Evotherm-modified bitumen are comparable to cup lump-modified bitumen, so the binder can be used at the mix design level for environmentally friendly and sustainable road construction. It is worthy of mention that all the tests conducted in this study were at the binder level. As a result, future research will look into the mechanical interfacial bonding performance of the Evotherm-modified binders as well as cost–benefit analyses of the binders at the mix design level.

**Acknowledgements** The authors would like to thank Universiti Teknologi Malaysia for their assistance, and the NEEDS Assessment Intervention from Kebbi State University of Science and Technology in Nigeria.

## References

- Allison, K.: Those amazing rubber roads. *Rubber World*. **78**, 47–52 (1967)
- Shaffie, E.; Ahmad, J.; Arshad, A.K.; Kamarun, D.; Awang, H.: Investigation on rutting performance of nanopolyacrylate and natural rubber latex polymer modified asphalt binder mixes. *J. Teknol.* **78**, 11–15 (2016). <https://doi.org/10.11113/jt.v78.9469>
- Shaffie, E.; Ahmad, J.; Arshad, A.K.; Kamarun, D.; Kamaruddin, F.: Stripping performance and volumetric properties evaluation of hot mix asphalt (HMA) mix design using natural rubber latex polymer modified binder (NRMB). In: *InCIEC 2014*, pp. 873–884. Springer, Singapore (2015)
- Siswanto, H.: The effect of latex on permanent deformation of asphalt concrete wearing course. *Procedia Eng.* **171**, 1390–1394 (2017). <https://doi.org/10.1016/j.proeng.2017.01.452>
- Othman, Z.; Hainin, M.R.; Warid, M.N.M.; Idham, M.K.; Kamarudin, S.N.N.: Cup lump modified asphalt mixture along Jalan Kuala Lumpur-Kuantan, daerah Temerloh. Pahang. *MATEC Web Conf.* **250**, 1–8 (2018). <https://doi.org/10.1051/mateconf/201825002007>
- Tuntiworawit, N.; Lavansiri, D.; Phromsorn, C.: The modification of asphalt with natural rubber latex. *Proc. East. Asia Soc. Transp. Stud.* **5**, 679–694 (2005)
- Bernamea: Malaysia's New Rubberised Road Technique a World-First (2017)
- Cao, W.; Wang, Y.; Wang, C.: Fatigue characterization of bio-modified asphalt binders under various laboratory aging conditions. *Constr. Build. Mater.* **208**, 686–696 (2019). <https://doi.org/10.1016/j.conbuildmat.2019.03.069>
- Mirzaaghaeian, E.; Modarres, A.: Rheological properties of bituminous mastics containing chemical warm additive at medium temperatures and its relationship to warm mix asphalt fatigue behavior. *Constr. Build. Mater.* **225**, 44–54 (2019). <https://doi.org/10.1016/j.conbuildmat.2019.07.236>
- Fakhri, M.; Javadi, S.; Sassani, A.; Torabi-Dizaji, M.: Zinc slag as a partial or total replacement for mineral filler in warm mix asphalt and its effects on self-healing capacity and performance characteristics. *Materials* **15**(3), 736 (2022)
- Abdullah, M.E.; Zamhari, K.A.; Hainin, M.R.; Oluwasola, E.A.; Hassan, N.A.; Yusoff, N.I.M.: Engineering properties of asphalt binders containing nanoclay and chemical warm-mix asphalt additives. *Constr. Build. Mater.* **112**, 232–240 (2016). <https://doi.org/10.1016/j.conbuildmat.2016.02.089>
- Mustafa Kamal, M.; Hadithon, K.A.; Abu Bakar, R.: Warm mix asphalt additive in natural rubber modified bitumen. *MATEC Web Conf.* **266**, 04005 (2019). <https://doi.org/10.1051/mateconf/201926604005>
- Abdulrahman, S.; Hainin, M.R.; Idham Mohd Satar, M.K.; Abdul Hassan, N.; Mohd Warid, M.N.; Yaacob, H.; Mohd, A.; Che Puan, O.: Physical properties of warm cuplump modified bitumen. *IOP Conf. Ser. Mater. Sci. Eng.* (2019). <https://doi.org/10.1088/1757-899X/527/1/012048>
- Abdulrahman, S.; Rosli, M.; Khairul, M.; Mohd, I.: Mechanical performance and global warming potential of unaged warm cup lump modified asphalt. *J. Clean. Prod.* (2021). <https://doi.org/10.1016/j.jclepro.2021.126653>
- Wang, H.; Liu, X.; Zhang, H.; Apostolidis, P.; Scarpas, T.; Erkens, S.: Asphalt–rubber interaction and performance evaluation of rubberised asphalt binders containing non-foaming warm-mix additives. *Road Mater. Pavement Des.* (2018). <https://doi.org/10.1080/14680629.2018.1561380>
- Abdulrahman, S.; Hainin, M.R.; Khairul, M.; Mohd, I.; Hassan, A.; Usman, A.: Rutting and moisture damage evaluation of warm mix asphalt incorporating POFA modified bitumen. *Int. J. Eng. Adv. Technol.* **9**, 90–98 (2019). <https://doi.org/10.35940/ijeat.A1052.109119>
- Almeida-Costa, A.; Benta, A.: Economic and environmental impact study of warm mix asphalt compared to hot mix asphalt. *J. Clean. Prod.* **112**, 2308–2317 (2016). <https://doi.org/10.1016/j.jclepro.2015.10.077>
- Fransesqui, M.A.; Yepes, J.; García-González, C.; Gallego, J.: Sustainable low-temperature asphalt mixtures with marginal porous volcanic aggregates and crumb rubber modified bitumen. *J. Clean. Prod.* **207**, 44–56 (2019). <https://doi.org/10.1016/j.jclepro.2018.09.219>



19. Ghafar, S.A.; Warid, M.N.M.; Hassan, N.A.: Laboratory investigation of cup lump modified bitumen emulsion. *Constr. Build. Mater.* **359**, 129471 (2022). <https://doi.org/10.1016/j.conbuildmat.2022.129471>
20. Abdul Ghafar, S.; Mohd Warid, M.N.; Abdul Hassan, N.: Effect of emulsifier on physical properties of cup lump modified emulsified bitumen residues. *IOP Conf. Ser. Earth Environ. Sci.* (2022). <https://doi.org/10.1088/1755-1315/1022/1/012032>
21. Mustafa Kamal, M.; Ariffin Hadithon, K.; Abd Rahim, R.: Performance of cup lump modified binder (CMB)—HMA containing Sasobit® Wax. *IOP Conf. Ser. Earth Environ. Sci.* (2022). <https://doi.org/10.1088/1755-1315/1022/1/012028>
22. Hazoor Ansari, A.; Jakarni, F.M.; Muniandy, R.; Hassim, S.; Elahi, Z.; Meftah Ben Zair, M.: Effect of cup lump rubber as a sustainable bio-modifier on the properties of bitumen incorporating polyphosphoric acid. *Constr. Build. Mater.* **323**, 12650 (2022). <https://doi.org/10.1016/j.conbuildmat.2022.126505>
23. Azahar, N.M.; Hassan, N.A.; Jaya, R.P.; Hainin, M.R.; Yusoff, N.I.M.; Kamaruddin, N.H.M.; Yunus, N.Z.M.; Hassan, S.A.; Yaacob, H.: Properties of cup lump rubber modified asphalt binder. *Road Mater. Pavement Des.* **220**, 1–21 (2019). <https://doi.org/10.1080/14680629.2019.1687007>
24. Mohd Azahar, N.; Abdul Hassan, N.; Putrajaya, R.; Rosli Hainin, M.; Che Puan, O.; Athma Mohd Shukry, N.; Azril Hezmi, M.: Engineering properties of asphalt binder modified with cup lump rubber. *IOP Conf. Ser. Earth Environ. Sci.* **220**, 1–13 (2019). <https://doi.org/10.1088/1755-1315/220/1/012014>
25. Azahar, N.M.; Hassan, N.A.; Jaya, R.P.; Nor, H.M.; Satar, M.K.I.M.; Mashros, N.; Mohamed, A.: Mechanical performance of asphalt mixture containing cup lump rubber. *J. Teknol.* **81**, 179–185 (2019). <https://doi.org/10.11113/jt.v81.13857>
26. Razali, R.; Hainin, M.R.; Hassan, N.A.; Idham, M.K.; Malek, M.S.C.; Warid, M.N.M.; Yaacob, H.; Mohamed, A.; Ismail, C.R.: Field performance of asphalt pavement maintenance using cup lump rubber modified asphalt (CMA). *Mater. Sci. Eng.* **527**, 1–9 (2019). <https://doi.org/10.1088/1757-899X/527/1/012064>
27. Shaffie, E.; Wan Hanif, W.M.M.; Arshad, A.K.; Hashim, W.: Rutting resistance of asphalt mixture with cup lumps modified binder. *IOP Conf. Ser. Mater. Sci. Eng.* **271**, 1–8 (2017). <https://doi.org/10.1088/1757-899X/271/1/012056>
28. Sani, A.; Mohd Hasan, M.R.; Shariff, K.A.; Jamshidi, A.; Ibrahim, A.H.; Poovaneshvaran, S.: Engineering and microscopic characteristics of natural rubber latex modified binders incorporating silane additive. *Int. J. Pavement Eng.* (2019). <https://doi.org/10.1080/10298436.2019.1573319>
29. Sani, A.; Hasan, M.R.M.; Shariff, K.A.; Poovaneshvaran, S.; Ibrahim, I.: Morphological identification of latex modified asphalt binder prepared with surfactants. In: Mohamed Nazri, F. (ed.) *Proceedings of AWAM International Conference on Civil Engineering*, pp. 1–13. Springer International Publishing, Cham (2020)
30. Abu Bakar, S.K.; Abdulah, M.E.; Mustafa Kamal, M.; Abd Rahman, R.; Hadithon, K.A.; Buhari, R.; Ahmad Tajudin, S.A.: Evaluating the rheological properties of waste natural rubber latex modified binder. *E3S Web Conf.* **34**, 1–5 (2018). <https://doi.org/10.1051/e3sconf/20183401037>
31. Shafii, M.A.; Lai Yew Veng, C.; Mohamad Rais, N.; Ab Latif, A.: Effect of blending temperature and blending time on physical properties of NRL-modified bitumen. *Int. J. Appl. Eng. Res.* **12**, 3844–3849 (2017)
32. Siswanto, H.: Improving of water resistance of asphalt concrete wearing course using latex-bitumen binder. *MATEC Web Conf.* **97**, 1–7 (2017). <https://doi.org/10.1051/mateconf/20179701033>
33. Krishnapriya, M.G.: Performance evaluation of natural rubber modified bituminous mixes. *Int. J. Civ. Struct. Environ. Infrastruct. Eng. Res. Dev.* **5**, 121–134 (2015)
34. Wen, Y.; Wang, Y.; Zhao, K.; Sumalee, A.: The use of natural rubber latex as a renewable and sustainable modifier of asphalt binder. *Int. J. Pavement Eng.* **18**, 547–559 (2015). <https://doi.org/10.1080/10298436.2015.1095913>
35. Tinavallie, A.: Improving the ductility and elastic recovery of bitumen-natural rubber latex blend. Master Thesis (2013)
36. ASTM D5D5M: Standard Test Method for Penetration of Bituminous Materials. In: *Annual Book of ASTM Standards*, pp. 6–8. ASTM International, West Conshohocken, PA (2014)
37. ASTM D 36: Standard Test Method for Softening Point of Bitumen (Ring-and-Ball Apparatus). In: *ASTM Standard*, pp. 8–11. ASTM International, West Conshohocken, PA (2014)
38. Huang, J.; Muhammad, Y.; Li, J.; Li, J.; Yang, C.: Study on the self-healing performance of urea-formaldehyde-dicyclopentadiene (UF-DCPD) microcapsules-incorporated SBS polymer-modified asphalt. *Arab. J. Sci. Eng.* **47**, 5079–5091 (2022). <https://doi.org/10.1007/s13369-021-06416-7>
39. ASTM D6084: Standard test Method for Elastic recovery of Asphalt Materials by Ducltometer. In: *Annual Book of ASTM Standards*, pp. 668–672 (2014)
40. Ullah, S.; Shah, M.I.; Alqurashi, M.; Javed, M.F.; Dawood, O.; Aslam, F.; Atiq, M.; Rehman, U.; Hussain, E.E.: Eco-friendly incorporation of crumb rubber and waste bagasse ash in bituminous concrete mix. *Materials* **15**(7), 2509 (2022)
41. Yu, X.; Liang, X.; Chen, C.; Ding, G.: Towards the low-energy usage of high viscosity asphalt in porous asphalt pavements: a case study of warm-mix asphalt additives. *Case Stud. Constr. Mater.* **16**, e00914 (2022). <https://doi.org/10.1016/j.cscm.2022.e00914>
42. Hunter, R.N.; Self, A.; Read, J.: *The Shell Bitumen Handbook*, 6th edn. Thomas Telford Ltd, London (2015)
43. ASTM D7175: Standard Test Method for Determining the Rheological Properties of Asphalt Binder Using a Dynamic Shear Rheometer. In: *ASTM Standard*, pp. 1–16 (2008)
44. Cavalli, M.C.; Zaumanis, M.; Mazza, E.; Partl, M.N.; Poulidakos, L.D.: Aging effect on rheology and cracking behaviour of reclaimed binder with bio-based rejuvenators. *J. Clean. Prod.* **189**, 88–97 (2018). <https://doi.org/10.1016/j.jclepro.2018.03.305>
45. ASTM D6816: Standard Practice for Determining Low-Temperature Performance Grade (PG) of Asphalt Binders. In: *Annual Book of ASTM Standards*, pp. 1–9. ASTM International, West Conshohocken, USA (2015)
46. Carret, J.C.; Falchetto, A.C.; Marasteanu, M.O.; Di Benedetto, H.; Wistuba, M.P.; Sauzeat, C.: Comparison of rheological parameters of asphalt binders obtained from bending beam rheometer and dynamic shear rheometer at low temperatures. *Road Mater. Pavement Des.* **16**, 211–227 (2015). <https://doi.org/10.1080/14680629.2015.1029696>
47. Aguiar-Moya, J.P.; Salazar-Delgado, J.; Bonilla-Mora, V.; Rodríguez-Castro, E.; Leiva-Villacorta, F.; Loría-Salazar, L.: Morphological analysis of bitumen phases using atomic force microscopy. *Road Mater. Pavement Des.* **16**, 138–152 (2015). <https://doi.org/10.1080/14680629.2015.1029672>
48. Al-Rawashdeh, A.S.; Sargand, S.: Performance assessment of a warm asphalt binder in the presence of water by using surface free energy concepts and nanoscale techniques. *J. Mater. Civ. Eng.* **26**, 803–811 (2013). [https://doi.org/10.1061/\(asce\)jmt.1943-5533.0000866](https://doi.org/10.1061/(asce)jmt.1943-5533.0000866)
49. Pauli, A.T.; Grimes, R.W.; Beemer, A.G.; Turner, T.F.; Branthaver, J.F.: Morphology of asphalts, asphalt fractions and model wax-doped asphalts studied by atomic force microscopy. *Int. J. Pavement Eng.* **12**, 291–309 (2011). <https://doi.org/10.1080/10298436.2011.575942>
50. Maia, M.M.A.S.; Dinis-almeida, M.; Martinho, F.C.G.: The influence of the affinity between aggregate and bitumen on the mechanical performance properties of asphalt Mixtures 1–13 (2021)

51. Bhasin, A.; Little, D.N.: Application of microcalorimeter to characterize adhesion between asphalt binders and aggregates. *J. Mater. Civ. Eng.* **21**, 235–243 (2009). [https://doi.org/10.1061/\(asce\)0899-1561\(2009\)21:6\(235\)](https://doi.org/10.1061/(asce)0899-1561(2009)21:6(235))
52. Kakar, M.R.; Hamzah, M.O.; Akhtar, M.N.; Woodward, D.: Surface free energy and moisture susceptibility evaluation of asphalt binders modified with surfactant-based chemical additive. *J. Clean. Prod.* **112**, 2342–2353 (2016). <https://doi.org/10.1016/j.jclepro.2015.10.101>
53. Hefer, A.: Bitumen-aggregate systems and quantification of the effects of water on the adhesive bond. Ph.D Thesis (2005)
54. NCHRP Report RRD 316: Using surface energy measurements to select materials for asphalt pavements. The National Academies Press, Washington, D.C. (2007)
55. Turbay, E.; Martínez-Arguelles, G.; Navarro-Donado, T.; Sánchez-Cotte, E.; Polo-Mendoza, R.; Covilla-Valera, E.: Rheological behaviour of WMA-modified asphalt binders with crumb rubber. *Polymers* (2022). <https://doi.org/10.3390/polym14194148>
56. Li, D.; Leng, Z.; Zou, F.; Yu, H.: Effects of rubber absorption on the aging resistance of hot and warm asphalt rubber binders prepared with waste tire rubber. *J. Clean. Prod.* **303**, 127082 (2021). <https://doi.org/10.1016/j.jclepro.2021.127082>
57. Yu, H.; Leng, Z.; Gao, Z.: Thermal analysis on the component interaction of asphalt binders modified with crumb rubber and warm mix additives. *Constr. Build. Mater.* **125**, 168–174 (2016). <https://doi.org/10.1016/j.conbuildmat.2016.08.032>
58. Li, R.; Shao, N.; Yue, J.; Liang, B.: Research on the influence of different warm-mix modifiers on pavement performance of bitumen and its mixture. *Appl. Sci.* **13**, 955 (2023)
59. Yousefi, A.A.: Rubber-modified bitumens. *Iran. Polym. J.* **11**, 303–309 (2002)
60. Al-Shamsi, K.; Hassan, H.F.; Mohammed, L.N.: Effect of low VMA in hot mix asphalt on load-related cracking resistance. *Constr. Build. Mater.* **149**, 386–394 (2017). <https://doi.org/10.1016/j.conbuildmat.2017.05.120>
61. Golchin, B.; Hamzah, M.O.; Hasan, M.R.M.: Optimization in producing warm mix asphalt with polymer modified binder and surfactant-wax additive. *Constr. Build. Mater.* **141**, 578–588 (2017). <https://doi.org/10.1016/j.conbuildmat.2017.02.123>
62. Oliveira, J.R.M.; Silva, H.M.R.D.; Abreu, L.P.F.; Fernandes, S.R.M.: Use of a warm mix asphalt additive to reduce the production temperatures and to improve the performance of asphalt rubber mixtures. *J. Clean. Prod.* **41**, 15–22 (2013). <https://doi.org/10.1016/j.jclepro.2012.09.047>
63. Kataware, A.V.; Singh, D.: Evaluating effectiveness of WMA additives for SBS modified binder based on viscosity, superpave PG, rutting and fatigue performance. *Constr. Build. Mater.* **146**, 436–444 (2017). <https://doi.org/10.1016/j.conbuildmat.2017.04.043>
64. Bocoum, A.; Hosseinnezhad, S.; Fini, E.: Investigating effect of amine based additives on asphalt rubber rheological properties. In: *Asphalt Pavements*, pp. 921–931. Taylor & Francis Group, London (2014)
65. Xiao, F.; Punith, V.S.; Amirkhanian, S.N.: Effects of non-foaming WMA additives on asphalt binders at high performance temperatures. *Fuel* **94**, 144–155 (2012). <https://doi.org/10.1016/j.fuel.2011.09.017>
66. Yu, H.; Leng, Z.; Xiao, F.; Gao, Z.: Rheological and chemical characteristics of rubberized binders with non-foaming warm mix additives. *Constr. Build. Mater.* **111**, 671–678 (2016). <https://doi.org/10.1016/j.conbuildmat.2016.02.066>
67. Zhang, W.; Zou, L.; Wang, Y.; Liu, J.; Yang, C.; Di, J.; Hu, H.; Yang, Z.: Influence of high viscosity petroleum resin (HV-PR) on the intermediate and high temperature performances of styrene-butadiene-styrene block copolymer (SBS) modified bitumen. *Arab. J. Sci. Eng.* **47**, 12521–12533 (2022). <https://doi.org/10.1007/s13369-021-06550-2>
68. ASTM D 6648-08: Standard Test Method for Determining the Flexural Creep Stiffness of Asphalt Binder Using the Bending Beam Rheometer (BBR). In: *Annual Book of ASTM Standards*, pp. 1–14. ASTM International, West Conshohocken, USA (2014)
69. Zhou, Y.; Xu, G.; Leng, Z.; Kong, P.; Wang, H.; Yang, J.; Qiu, W.; Xu, Z.; Chen, X.: Investigation on storage stability and rheological properties of environment-friendly devulcanized rubber modified asphalt. *Phys. Chem. Earth.* **129**, 103332 (2023). <https://doi.org/10.1016/j.pce.2022.103332>
70. Jiang, H.; Liu, Y.; Muhammad, Y.; Pei, R.; Guo, R.; Li, J.: Preparation and evaluation of performance and mechanism of gallic acid-rubber powder-microalgae bio-oil/styrene block copolymers composite modified asphalt. *Arab. J. Sci. Eng.* (2022). <https://doi.org/10.1007/s13369-022-07366-4>
71. De Moraes, M.B.; Pereira, R.B.; Simão, R.A.; Leite, L.F.M.: High temperature AFM study of CAP 30/45 pen grade bitumen. *J. Microsc.* **239**, 46–53 (2010). <https://doi.org/10.1111/j.1365-2818.2009.03354.x>
72. Wan Nur Aifa, W.A.; Jaya, R.P.; Hainin, M.R.: Binder characterization and performance of asphaltic concrete modified with waste cooking oil. Ph.D Thesis (2016)
73. Samsudin, M.S.; Arshad, A.K.; Ahmad, J.; Masri, K.A.: Microstructure of nanosilica modified binder by atomic force microscopy. *J. Teknol.* **78**, 33–44 (2016). <https://doi.org/10.11113/jt.v78.9480>
74. Magonov, S.; Alexander, J.; Surtchev, M.; Hung, A.M.; Fini, E.H.: Compositional mapping of bitumen using local electrostatic force interactions in atomic force microscopy Sergei Magonov. *J. Microsc.* **265**, 196–206 (2017). <https://doi.org/10.1111/jmi.12475>
75. Tarrer, A.; Wagh, V.: The effect of the physical and chemical characteristics of the aggregate on bonding. Washington, DC. Strategic Highway Research Program National Research Council (1991)
76. materials-14-06248-v3.pdf
77. Sheth, N.; Sebaaly, P.E.; Hajj, E.Y.; Pirathepan, M.: Evaluation of selected warm mix asphalt technologies. *Road Mater. Pavement Des.* **16**, 475–486 (2015). <https://doi.org/10.1080/14680629.2015.1030825>

Springer Nature or its licensor (e.g. a society or other partner) holds exclusive rights to this article under a publishing agreement with the author(s) or other rightsholder(s); author self-archiving of the accepted manuscript version of this article is solely governed by the terms of such publishing agreement and applicable law.